# DRILLING WASTE DISCHARGES IN THE MARINE ENVIRONMENT: A RISK BASED DECISION METHODOLOGY

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### DRILLING WASTE DISCHARGES IN THE MARINE ENVIRONMENT: A RISK BASED DECISION METHODOLOGY

by

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#### ABSTRACT

Offshore drilling operations generate rock cuttings and spent drilling fluids wastes during the well drilling phase. The wastes contain toxic substances that are harmful to the marine ecosystem. Discharge limitations and restrictive guidelines in different jurisdictions of the world are under development. Pollution prevention by encouraging synthetic based fluids (SBFs) in place of traditional oil based fluids has been supported in recent years. Despite having environmentally benign characteristics. SBFs associated wastes still have a certain amount of pollutants due to barite and contamination with formation oil.

The main aim of this study was to develop a risk management framework for determining the best diffiling waste fischarge scenario for disposal in the marine environment. The specific objectives of this research were: (1) development of probabilistic contaminant fate modeling methodology using fugacity and aquivalence based approaches; (2) development of an ecological risk assessment methodology using probabilistic concepts; (3) development of human health cancer and non-cancer risk assessment methodologies using probabilistic concepts; (4) development of a fuzzy composite programming framework for risk management by integrating environmental risk. cost estimates and technical feasibility for various treatment options; and (5) an application of the developed risk management framework to a hypothetical case study.

Fate modeling in this research was performed using fugacity and aquivalence based concepts. A chemical specific approach was employed for contaminant fate modeling. A steady state nonequilibrium water and sediment interaction model with probabilistic inputs was used to determine the contaminant concentrations in the water column and pore water. The uncertainty and variability in the model inputs were expressed by the statistical distributions. The concentrations in the water column and pore water were estimated using Latin Hypercube sampling (LHS) based Monte Carlo (MC) simulations. The concentrations in the water column and pore water followed lognormal distributions. The estimated parameters of lognormal distribution for known discharge conditions were used for performing multiple regression analyses. The highest 95<sup>th</sup> percentile was used as the predicted environmental concentration (*PEC*). The uncertainties in the *PEC* were expressed by the coefficients of regression models.

The PEC values were converted into exposure concentrations (EC) by adjusting for bioavailability and probability of exposure. The whole ecological community was defined as assessment endpoints. The toxicity assessment analyses were based on the lognormally distributed predicted no effect concentrations (PVEC). The lowes It( $^{\circ}$  percentile on PVEC distributions was used as a safety level or PVEC criteria value. Bootstrapping was performed on original PVEC data to determine the uncertainty in the PVEC criteria values. The hazard or risk quotients (HQIRQ) were calculated by dividing EC with PVEC criteria values. The LCHARM model's approach was used to covert HQ/RQ into risk estimates for each contaminant. The composite ecological risk for drilling waste was determined by integrating the individual risk estimates assuming statistically independent events.

The human health risk methodology was based on the consumption of contaminated seafood. A probabilistic framework for human health risk assessment was developed for cancer and noncancer risk estimates. The chronic daily intake rate (CDI) was established based on fish ingestion rates, lipid content, bioconcentration factors, exposure duration, exposure frequency, and averaging time. The LHS based MC simulations were performed to estimate the CDI. Arsenic was the only proven human carcinogen in the drilling waste stream. The composite hazard index for non-cancer risks was calculated by simple addition for a given exposure scenario. A risk management methodology using fuzzy composite programming (CP) was developed. The costs for trearment, editing full dots due to discharge and ecological and human health damages were estimated. The technical feasibility of various solid control devices was also studied from a performance viewpoint. The environmental risk reductions, cost suving and technical feasibility indices were grouped using CP methodology. A double weighting scheme was employed in CP. The final utility and centroidal values of the system improvement indices were calculated through Chen and Yager fuzzy ranking methods. respectively to determine the best management alternative.

The risk management framework was applied to a hypothetical case study on the East Coast of Canada. Five discharge scenarios, or management alternatives including 10.0%, 8.5%, 7.0%, 5.5% and 4.0% attached base fluids to wet cuttings, were selected for the analysis. The 4.7¢ attached base fluid option was found to be the best management option for the first trial when risk and cost were given equal weight and technical feasibility was allotted one-third of the weight. The 7.0% discharge option was the second best management option and that was followed by the 5.5%, 8.5% and 10.0%, respectively. The sensitivity analysis was performed using four different weighting schemes to account for human subjectivity. It was concluded that 4.0% was the best management alternative for the discharge options of 7.0% and/or 5.5% attached base fluid to the wet cuttings (approximately 5.6% of div cuttings).

This study has introduced a new concept of integrating probabilistic fate modeling with ecological and human health risk assessment methodologies within a risk management framework to determine the best management alternative under conflicting objectives. This study has provided a framework for a decision support system for the selection of the best drilling waste marine discharge option under any known regulatory and technical constraints.

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### LIST OF SYMBOLS

| 2                 | Representing Water Media in Water-Sediment interaction Model (subscript)    |  |
|-------------------|---|--|
| 4                 | Representing Sediment Media in Water-Sediment interaction Model (subscript) |  |
| 5                 | Representing SS in Water-Sediment Interaction Model (subscript)             |  |
| 6                 | Representing Biota in Water-Sediment Interaction Model (subscript)          |  |
| α                 | Regression Coefficients   |  |
| A                 | Aquivalence or Coefficient for Deposition                                   |  |
| А,                | Management alternatives   |  |
| Α.                | Impact Area   |  |
| В                 | Bias Correction Factor or number of bootstrap samples                       |  |
| С                 | Concentration   |  |
| $C_{\mathcal{E}}$ | Effluent Concentration  |  |
| CPW               | Concentration in Pore Water   |  |
| Cw                | Concentration in Water Column   |  |
| δ                 | Grain/Particle Diameter   |  |
| D                 | Transportation or Transformation Parameter                                  |  |
| Δ                 | Sediment Mixing Depth   |  |
| Dsw               | Diffusion Coefficient   |  |
| d.,               | Depth of Water Column   |  |
| Е                 | Pollutant Loading Rate  |  |
| Е                 | Exponential Distribution  |  |
|                   |   |  |

| ε                          | Error Term or Resuspension Rate (mg/m2-hr)      |
|----------------------------|---|
| EC:0                       | Concentration at 50% Adverse Effects            |
| $f_i$                      | Fugacity  |
| $\boldsymbol{\varphi}_{i}$ | Organic Fraction                                |
| Ø <sup>-1</sup>            | Standard Normal Function                        |
| Pmas                       | Scalar Corresponding to the Maximum Eigen Value |
| G                          | Gibbs's Free Energy                             |
| G                          | Gravity Force                                   |
| <i>G</i> ,                 | Flow Rate                                       |
| 8                          | Gibbs's Free Energy/mole                        |
| h                          | Enthalpy or membership value                    |
| Н                          | Henry's Law Constant (Pa. m3/mol)               |
| HIC                        | Half Life                                       |
| i                          | Discount Rate                                   |
| k                          | Rate Constant                                   |
| KAW                        | Air-Water Partitioning Coefficient              |
| K <sub>B</sub>             | Bioconcentration Factor (BCF)                   |
| Koc                        | Portioning Coefficient for Organic Carbon       |
| Kow                        | Octanol Water Partitioning Coefficient          |
| KP                         | Portioning Coefficient (L/kg)                   |
| K <sub>n</sub>             | Sediment Partitioning Coefficient               |
| Ka                         | SS Partitioning Coefficient                     |
| Ksw                        | Water-Sediment Partitioning Coefficient         |
| L,                         | Lipid Content                                   |
| LC:0                       | Lethal Concentration to Kill 50% of Organisms   |
| $L_{th}(x)$                | Index Value at Membership Function of h         |
| LN                         | Lognormal Distribution                          |
| m                          | Mass  |
| μ                          | Mean  |
| $\mu_{h}$                  | Membership Function                             |
| n                          | Molar Concentration                             |

| N              | Normal Distribution                             |
|----------------|---|
| Р              | Pressure  |
| Р              | Probability                                     |
| P2             | Pollution Prevention                            |
| <i>p</i> ,     | Balancing Factor                                |
| PW             | Pore Water (Subscript)                          |
| R              | Impact Radius or Universal Gas Constant         |
| ρ              | Density   |
| R <sup>2</sup> | Coefficient of Determination                    |
| ρ,             | Rank Correlation Coefficient                    |
| 5              | Entropy   |
| S              | Solubility                                      |
| σ              | Standard Deviation                              |
| 1              | Time  |
| Т              | Temperature                                     |
| т              | Triangular Distribution                         |
| TN             | Truncated Normal Distribution                   |
| r              | Shear Stress                                    |
| U              | Uniform Distribution                            |
| u              | Advective Velocity or Current Speed             |
| uß             | Burial Rate                                     |
| U,             | Intermedia Transport Velocities                 |
| $U_{t}(x)$     | Utility Value                                   |
| v              | Volume Fraction                                 |
| V              | Volume  |
| w              | Settling Velocity                               |
| Way            | Weight Reflecting Importance of Basic Indicator |
| W              | Water (Subscript)                               |
| 5              | Resuspension Coefficient                        |
| z              | Fugacity Capacity                               |
| Ζ'             | Aquivalence Capacity                            |
|                |   |

### LIST OF ACRONYMS

| Analytical Hierarchy Process                        |
|---|
| American Petroleum Institute                        |
| American Standard for Testing and Materials         |
| Averaging Time                                      |
| Australia-New Zealand Water Quality Guide Lines     |
| Best Available (economically achievable) Technology |
| Barrels (160.4 L)                                   |
| Benthic Boundary Layer Transport                    |
| Bioconcentration Factor                             |
| Best Conventional Pollutant Control Technology      |
| Bioavailable Fraction                               |
| Bootstrap Interval                                  |
| Best Practicable Control Technology                 |
| Body Weight   |
| Canadian Council of Ministers for Environment       |
| Cumulative Distribution Function                    |
| Chronic Daily Intake (mg/kg-day)                    |
| Conversion Factor                                   |
| Chemical Hazard Assessment and Risk Management      |
| Confidence Interval                                 |
| Control Measures                                    |
| Canada-Newfoundland Offshore Petroleum Board        |
| Composite Programming                               |
|   |

| Consumer Price Index                               |
|--|
| Clean Water Act                                    |
| Dry Cuttings                                       |
| Dilution Factors                                   |
| Exposure Concentration                             |
| Exposure duration                                  |
| Empirical Distribution Function                    |
| Exposure Frequency                                 |
| Environmental Impact Assessment                    |
| Enhanced Mineral Oil                               |
| Ease of Operation                                  |
| Equilibrium Partitioning Approach                  |
| Fish Intake Rate                                   |
| Floating Production and Offloading System          |
| Fine Removal Unit                                  |
| Gravity Based Structure                            |
| Geometric Mean                                     |
| Goodness of Fit                                    |
| Gulf of Mexico                                     |
| Global Warming Impact                              |
| Health and Safety                                  |
| Human Health Risk Assessment                       |
| Hazard Index                                       |
| Composite Hazard Index                             |
| Hazard Quotient                                    |
| Iso-Alkane   |
| Internal Olefins                                   |
| Intake Rate  |
| Integrated Risk Information System                 |
| Joint Uncertainty and Inter-individual Variability |
| Linear Alkyl Benzene                               |
|  |

| LAO   | Linear Alpha Olefins                         |
|-------|--|
| LCA   | Life Cycle Analysis                          |
| LF    | Leaching Factor                              |
| LHS   | Latin Hypercube Sampling                     |
| LOEC  | Lowest Observed Effect Concentration         |
| MADM  | Multiple Attribute Decision-Making           |
| MAUF  | Multiple Attribute Utility Function          |
| MATC  | Maximum Acceptable Toxicant Concentration    |
| MC    | Monte Carlo (simulations)                    |
| MCDM  | Multiple Criteria Decision-Making            |
| MLV   | Most Likely Value                            |
| MMS   | Minerals Management Service                  |
| MODM  | Multiple Objective Decision-Making           |
| MSDS  | Material Safety Data Sheet                   |
| MT    | Mass Transfer Coefficient or Metric Tones    |
| MW    | Molecular Weight                             |
| NGL   | Natural Gas Liquid                           |
| NOEC  | No Observed Effect Concentration             |
| NORM  | Naturally Occurring Radioactive Materials    |
| OBF   | Oil Based Drilling Fluid                     |
| ODP   | Ozone Depletion Potential                    |
| OOC   | Offshore Operator's Committee                |
| OSPAR | Oslo and Paris                               |
| PAHs  | Polynuclear Aromatic Hydrocarbons            |
| PAO   | Polyalpha Olefins                            |
| PCBs  | Polychlorinated Biphenyls                    |
| PDF   | Probability Density Function                 |
| PEC   | Predicted Environmental Concentration        |
| PNEC  | Predicted No Effect Concentration            |
| PSS   | Primary Shale Shaker                         |
| QSAR  | Quantitative Structure Activity Relationship |

| QWASI    | Quantitative Water Air Sediment Interaction   |
|----------|---|
| RfD      | Reference Dose                                |
| RMS      | Root Mean Square                              |
| RMSE     | Root Mean Square Error                        |
| ROC      | Retention on Cuttings                         |
| RQ       | Risk Quotient                                 |
| SA       | Sensitivity Analysis                          |
| SAW      | Simple Additive Weighting                     |
| SBF      | Synthetic Based Drilling Fluid                |
| SE       | Standard Error                                |
| SF       | Slope Factor                                  |
| SS       | Suspended Solids                              |
| SS,      | Sum of Squares                                |
| SSS      | Secondary Shale Shaker                        |
| ST       | Status of Technology                          |
| TW       | Total Weight                                  |
| U.S. DOE | U.S. Department of Energy                     |
| U.S. EPA | United States Environmental Protection Agency |
| UF       | Uncertainty Factors                           |
| WBF      | Water Based Drilling Fluid                    |

#### Chapter 1

#### INTRODUCTION

This research was planned to integrate and apply various components of nsk-based decision methodology for synthetic based drilling waste discharges in the marine environment. The components of this risk-based approach are contaminant fate modeling. ecological and human health risk assessment and risk management. This thesis focuses on all these components of risk-based decision-making approach with relevance to drilling waste discharges in the marine environment. A hypothetical case study is also discussed in this thesis for an application of the developed holistic methodology.

#### 1.1. BACKGROUND OF THE RESEARCH

Demand for energy sources is increasing throughout the world at a rapid rate due to industrial development and population growth. This increasing demand has accelerated the search for new sources of oil and gas on land and offshore. Oil production involves two main phases of drilling operations. namely exploration and development. The exploration phase operations determine the potential hydrocarbon reserves and include drilling of exploratory wells, whereas the development phase operations include drilling of production wells, once a hydrocarbon reserve has already been discovered and delineated. The rigs used in exploration and development sometimes differ but the drilling process is generally the same for both types of operations (U.S. EPA, 1999a).

Drilling operations use drilling muds to lubricate the drill bit. control borehole pressure, and flush rock cuttings out of the well. Disposal of rock cuttings and used mud constitutes one of the most significant waste discharges associated with offshore drilling. The drilling fluids or circulating muds, are broadly classified into three groups (U.S. EPA, 1999a).

- Water based drilling fluid (WBF): conventional drilling mud with water used as its continuous phase.
- Oil based drilling fluid (OBF): diesel, mineral, or some other oil used as its continuous phase.
- Synthetic based drilling fluid (SBF): synthetic based material like polyesters and vegetable esters used as its continuous phase.

SBF consists of a synthetic base fluid, weighting agent (bante) and some other additives. Since 1990, the oil and gas extraction industry has developed many synthetic and nonsynthetic materials as base fluids to provide drilling performance characteristics comparable to traditional oil based fluids (OBFs) with the lower environmental impacts and greater worker safety of water based fluids (WBFs). These characteristics have been achieved through lower toxicity, elimination of polynuclear aromatic hydrocarbons (PAHs), faster biodegradation rates and lower bioaccumulation potential of pollutants (U.S. EPA, 1999a).

Prohibition of the discharge of rock cuttings into the North Sea has been implemented (U.S. EPA, 2000a). As a result, all cuttings must now be either re-injected downhole, or shipped to the shore for treatment and disposal. The regulatory authorities in Nova Scotia have effectively followed suit, moving from a relatively lax 15% (by dry weight) drilling muds on cuttings allowed at discharge to 1%. As the best available economically achievable technology (BAT) can reduce contaminants to the neighbourhood of 4% (U.S. EPA, 2000a), the 1% allowable limit is in practice a prescription of a total ban on drilling waste discharges.

The United States Environmental Protection Agency (U.S. EPA) has identified different options to reduce the discharge of drilling waste into the marine environment. Current industry practice for managing and treating SBF-cuttings before discharge is to process the cuttings through solids separation equipment, which consists of primary and secondary shale shakers and occasionally a centrifuge. Based on current industry data, the efficiency of solids separation equipment results in a long-term average of 10.2% (by wet weight) retention on cuttings. By using new treatment technology, the retention of fluids can be reduced to approximately 4% (U.S. EPA, 2000a, b).

The disposal of drilling fluids into the ocean environment is of major concern for two main reasons: economical loss associated with discharged expensive synthetic drilling

3

fluids with rock cuttings and probable adverse ecological impacts. To determine the fate of contaminants associated with drilling waste, several studies have been conducted in the past. The U.S. EPA (2000a) developed a methodology for assessing surface water and pore water quality impacts by using the Offshore Operator's Committee (OOC) model as described by Brandsma (1996). In this analysis it was assumed that discharged contaminants immediately leached into the water column or into the pore water. Instead of SBFs waste as a whole, constituent organic priority pollutants and heavy metals were studied in this analysis. Some other modeling attempts have been conducted based on particle tracking techniques (e.g., Seaconsult, 2000). A set of models for the dispersion and drift of drilling wastes and suspended sediment in the benthic boundary layer is being developed by Hannah et al. (1997) who applied these models to the Georges Bank, Canada.

Thibodeaux et al. (1986) developed a model for the fate and transport of chemical contaminants originating from offshore drilling bottom deposits. This model was limited to the transport of the soluble constituents from the cuttings and mud deposits. This model does not take into account particulate transport and chemical and biochemical transformations that degrade contaminants within the sediment zone or boundary layer. In the North Sea OSPAR (Oslo and Paris), the Chemical Hazard Assessment and Risk Management (CHARM) model was developed to help stakeholders, including regulators, operators and chemical suppliers. The CHARM model includes a risk management module (Thatcher et al., 1999). This model categorises wastes into different application groups, such as production waters and WBFs. The OBFs are not addressed in this model. due to limited information on input parameters such as biodegradation and bioaccumulation characteristics.

Mackay developed a methodology for estimating chemical fate in multimedia based on the complexity of transport and transformation processes. Mackay and co-workers (1983, 1989 and 1994) applied a fugacity/aquivalence model called QWASI (Quantitative Water Air Sediment Interaction) to rivers, lakes and Hamilton harbour, to study the distribution of detergent chemicals. PCBs (Polychlorinated biphenyls) and heavy metals. In principle, the concept can be applied to any well-mixed body of water for which hydraulic and particulate flow characteristics are defined. QWASI is a steady state non-equilibrium model, which is generally used in the deterministic mode so uncertainties associated with the input variables are not addressed.

Synthetics based fluids (SBFs) are hydrophobic in nature and tend to sink to the bottom with little dispersion. Therefore, the main research focus has been on determining toxicity in the sedimentary phase as opposed to the aqueous phase. Many studies including Candler and Leuterman (1997) and Rabke and Candler (1998) have reported the toxicity response of SBFs for different organisms. In addition to the toxicity of the base fluid, the drilling waste contains organic priority pollutants and heavy metals, which adversely affect the ecological community (U.S. EPA, 1999a). The CHARM model calculates ecological risk by taking the ratio of two parameters: the predicted environmental concentration to the predicted no effect concentration (*PEC/PNEC*). This method is similar to the U.S. EPA approach where predicted concentration is compared with Federal Water Quality Criteria (U.S. EPA, 1999b).

Another aspect of environmental risk assessment of drilling waste is human exposure through consumption of contaminated commercial fish. Marine organisms are exposed to pollutants through direct uptake (bioconcentration) and consumption of lower trophic level organisms (biomagnification). Bioaccumulation of chemicals in aquatic food chains, which is a combined effect of the above two processes, is an important phenomenon in aquatic organisms and affects their predators, especially humans and fisheating wildlife (Campfens and Mackay, 1997). The consumption of contaminated organisms may pose a threat to human health. A deterministic analysis for human exposure and related non-carcinogenic and carcinogenic health risk through consumption of finfish and shrimps is given by the U.S. EPA (1999b, 2000a).

Cost comparisons of different discharge options for water quality, and in the case of zero discharge the non-water quality environmental impacts, are provided in the U.S. EPA study (2000a). The risk management module of the CHARM model is not well accepted by stakeholders, although it compares various alternatives for risk reducing measures. The basis of risk management in the CHARM model is to combine the risk of individual substances into a single risk estimate. The several management alternatives can be compared on the basis of their cost and risk reduction strategy. Some other researchers (e.g. Stansbury et al., 1989) used composite programming for management of dredged material, which accommodated conflicting objectives of environmental risk and cost in their analysis.

To fill in the gap in the scientific literature for managing disposal of drilling waste in the marine environment, a risk-based approach was used. This research integrated contaminant fate modeling, and ecological and human health risk assessment based on risk management methodologies (Sadiq, 2000).

#### 1.2. SCOPE AND PURPOSE OF THE RESEARCH

The study had the following major objectives: determining the fate of contaminants in the water column and the sediment pore water: developing methodologies for ecological and human health risk assessments; and incorporating those in a risk management framework that could also evaluate various treatment technologies using a case study. The pollutant discharged was characterised by considering regulations proposed by different regulatory agencies including the Canada-Newfoundland Offshore Petroleum Board (CNOPB), U.S. EPA and Norwegian Pollution Control Authority (SFT).

The steady state probabilistic model for partitioning of contaminants in multimedia was developed using fugacity/aquivalence approaches. The multimedia-partitioning model was integrated with an Environmental Risk Assessment methodology, which helped to decide the best management strategy for the discharge of drilling wastes. Probabilistic and fuzzy arithmetic concepts were incorporated throughout the modeling process to account for uncertainties. In this research, a methodology for probabilistic contaminant fate modeling was proposed, which was integrated with environmental risk assessment and risk based multiple criteria fuzzy composite programming. To accomplish the above objectives, the proposed research covered the following tasks:

- Development of a methodology for fate modeling using probabilistic concepts:
- Development of a methodology for ecological risk assessment:
- · Development of a methodology for human health risk assessment;
- Development of a risk based decision-making approach by incorporating human health and ecological risks, technical feasibility of various treatment options, and environmental damage and treatment costs; and
- · Application of developed methodologies for a hypothetical case study.

The following are some environmental issues related to offshore drilling waste discharges, which were not studied in this research.

- Non-water quality environmental impacts, which include greenhouse gases and other air pollution problems;
- The drilling fluid exposure to workers during handling and treatment and their related health risk: and
- · The ecological damages caused by smothering and burial by drill cuttings.
### 1.3. ORGANIZATION OF THE THESIS

This thesis is comprised of eight chapters. The first two chapters provide an introduction to the problem and establish the theoretical background of the problem. Chapters 3-6 are mainly methodology-based chapters, which discuss relevant literature during the course of developing models. Chapter 7 is an application-based chapter and uses models and methodologies developed in previous chapters for a hypothetical case study. Conclusions and recommendations of this research are presented in Chapter 8. Figure 1.1 explains the organization of this research (Sadiq, 2000).

Specifically. Chapter 1 explains the scope and objectives of the research and the organisation of the thesis. Chapter 2 covers the theoretical background of the research and provides information about characteristics of drilling waste, and different available approaches and models for contaminant fate modeling. The main focus of Chapter 2 is to discuss the background literature related with contaminant fate modeling.

Chapter 3 develops the proposed methodology for probabilistic fate modeling. Physicochemical characteristics of contaminants and media (water column and sediments) are discussed in detail in this chapter. Based on probabilistic fate modeling results. regression models are developed in this chapter.

Chapter 4 develops a methodology for ecological risk assessment due to toxicological effects of chemicals present in the drilling waste. This analysis was performed based on U.S. EPA (1998) and Canadian Council of Ministers for Environment (CCME, 1997)

ecological risk assessment frameworks. The components of an ecological risk assessment framework include problem formulation, an analysis phase and risk characterization, which are discussed in detail in this chapter with reference to drilling waste discharges. A probabilistic approach using Monte Carlo (MC) simulations was employed in this chapter.

Chapter 5 develops a methodology for human health risk assessment due to consumption of contaminated commercial fish. The input parameters for cancer risk and non-cancer risk models and the associated uncertainties are discussed in detail. A probabilistic approach using MC simulations was used to estimate the human health risk.

Chapter 6 develops a nsk management methodology based on fuzzy composite programming. Literature related to risk management using multiple criteria decisionmaking methodologies is discussed. Various solid control treatment technology options available offshore are explored in this chapter. Cost estimations of treatment options, drilling fluids and environmental damage costs are also discussed. A risk management methodology was proposed by integrating risk estimates from chapters 4 and 5, and technical feasibility and cost estimates from Chapter 6.

Chapter 7 is a hypothetical case study on the East Cost of Canada. The models developed in the methodology-based chapters are applied in this case study. In this chapter various discharge scenarios are identified and pollutant loading rates are estimated based on available information.



Figure 1.1. Organization of the thesis

Based on the estimated pollutant loading rates, contaminant concentrations are calculated in the water column and pore water using the regression models developed in Chapter 3. Based on exposure concentrations, ecological risks are characterized using the methodology developed in Chapter 4. The concentration in fish tissues are estimated based on contaminant exposure concentrations. Human health risks are estimated based on consumption of contaminated fish as discussed in Chapter 5. Cost estimates of treatment options for various discharge scenarios are made based on the methodology developed in Chapter 6. Assumptions are made for the missing information and data where necessary for a hypothetical case study. Comparisons of various discharge scenarios are discussed in detail in Chapter 7.

Chapter 8 discusses the conclusions from this research. Recommendations for the directions of future research are established based on the results obtained.

# Chapter 2

# POTENTIAL CONTAMINANT FATE MODELING APPROACHES

#### 2.1. INTRODUCTION TO OFFSHORE OIL OPERATIONS

Various types of processing systems, equipment and operating techniques are used by the oil industry to explore, develop, and produce oil and gas from an offshore field. Operations during both the exploratory and development drilling stages also determine the design needs for offshore environmental considerations. Rotary drilling uses a system of circulating drilling fluid primarily to remove drill cuttings from the borehole. The drilling fluid cools and lubricates the drill string and drill bit; builds an impermeable cake on the well bore; controls subsurface pressure; supports part of the weight of the drill string; reduces the damage to the formation and conducts geological information from the formation to the surface (Meinhold, 1998).

The drilling fluid (mud) is a mixture of water, base fluids, special clays, and minerals and chemicals. The drilling fluid is pumped downhole through the drill string and is ejected through nozzles in the drill bit at high pressure. The jet of fluid lifts the cuttings off the bottom of the hole and away from the bit. The drilling fluid is circulated to the surface through the annulus between the drill string and casing. At the surface, the drill cuttings, silt, and sand are removed from the drilling fluid before it is returned downhole through the drill string. Figure 2.1 presents a schematic flow diagram of a drilling fluid circulation system.



Figure 2.1. Typical drilling fluids circulation systems (U.S. EPA, 1999a)

The cuttings, sand, and silt are separated from the drilling fluid by a solid separation process. Some of the drilling fluid remains attached to the cuttings after treatment. After solid separation, the cuttings are disposed of in a manner that depends on the type of drilling fluid used, the oil content of the cuttings, and the regulatory regime. The disposal methods include transport to shore for land-based disposal, ocean discharge, and subsurface injection. A typical solids control system consists of some of the following equipment, depending on the drilling program: primary and secondary shale shakers to perform the initial separation of drill cuttings and fluid, and a centrifuge to remove drilling fluid from the waste stream. Figure 2.2 illustrates the arrangement of such a system.



Figure 2.2. A layout of solid control system

# 2.2. DRILLING FLUIDS AND ASSOCIATED DRILLING WASTE

An OBF is a drilling fluid that consists of brine (seawater) contained as an emulsion in oil. This is in contrast to water based fluid (WBF), which consists of small quantities of oils present as lubricants as an emulsion within brine. Oil based fluids have been formulated with diesel and mineral oil. When shale is drilled with an OBF, the oil is in contact with the shale and the fine nature of the shales prevents entry of oil and the integrity of the shale is maintained. Therefore, reduction in shale water content occurs due to osmotic forces which leads to a strengthening of the shale in the near wellbore region. In contrast, when WBF is used to drill shale, water transmits from the fluid into the water-wet shale. The support of the shale is thus lost and the shale can washout of the wellbore (Hall, 2000).

Another characteristic of mineral oils is their high degree of natural lubricity. This is of great importance when drilling deviated and extended reach wells because drag factors are reduced and consequently so is the risk of the drill pipe becoming stuck. In contrast to WBF, the lack of polarity of the continuous phase of an OBF means that the fluids do not react with other potentially troublesome formations, such as salt, gypsum, and anhydrite. Mineral oils also provide stable mud properties over a wide range of temperatures and hole conditions and have corrosion inhibiting properties (Meinhold, 1998).

OBF shows poor biodegradability in aerobic conditions and the degradation of mineral oils is extremely slow in anaerobic conditions, which are the typical conditions found within a drill cutting pile. The U.S. EPA's initial legislation limiting discharge of oil or diesel based drilling fluid attached set a maximum of 15% of the total wet weight of cuttings discharged to the seafloor. This has led to a legacy of drill cutting piles, whose

physico-chemical properties have not appreciably changed since they were deposited 15 to 20 years ago, due to slow biodegradation rates (Hall, 2000).

The discharge of oil-contaminated cuttings during drilling activity is one of the major contributors to oil industry impacts upon the marine environment. The early focus of efforts to reduce the environmental impacts was centered on reducing the volumes of drilling fluids discharged with the cuttings, as well as the generic toxicity of the base oil itself. By 1985 it became clear that regardless of the inherent levels of toxicity of the base oils, the cuttings piles persisted and continued to pollute for many years due to leaching of chemicals into the ambient environment (Hall, 2000).

The U.S. EPA (1999b) suggested product substitution (e.g., SBFs instead of OBFs) as the best way of reducing offshore oil environmental impacts. The oil and gas extraction industry has developed many oleaginous (like oil) base materials such as vegetable esters, poly-alpha olefins (PAO), internal olefins (IO), linear alpha olefins (LAO), iso-alkane (IA), ethers, linear alkyl benzene (LAB), and others to increase the efficiency of drilling operations. These base fluids were developed to obtain the drilling performance of traditional OBFs in addition to lower environmental impact potential. The wastes generated from SBFs are less toxic, have lower bioaccumulation and faster biodegradation potential, and are free of polyaromatic hydrocarbons (PAHs). Table 2.1 gives a comparison of the PAH content of SBFs and OBFs. A lower PAH implies lower toxicity to an ecological community.

| Base Fluid                 | PAH         |      |
|----------------------------|-------------|------|
|                            | (%)         |      |
| Diesel                     | 25          | 2    |
| Conventional mineral oil   | 1 - 7       | OPE  |
| Paraffin oil               | <1          | OBrs |
| Enhanced mineral oil (EMO) | 0.001 - 0.2 |      |
| Esters                     | = 0         |      |
| Acetal                     | ≈ 0         |      |
| Poly-alpha olefins (PAO)   | ≈ 0         | SBFs |
| Linear alpha olefins (LAO) | ≈0          |      |
| Iso-alkane (IA)            | ≈0          |      |
| Internal olefins (IO)      | ≈0          | ,    |

Table 2.1. PAH content comparison of SBFs and OBFs (modified after Meinhold, 1998)

With these improved characteristics, the U.S. EPA (1999a) is still proposing a controlled discharge of the cuttings associated with SBFs. SBFs are less dispersible in nature and sink to the seafloor, and may be a potential environmental concern to the benthic community. It is believed that environmental impacts include smothering by the drill cuttings, changes in grain size and composition, and anoxia caused by the decomposition of organic matter (U.S. EPA, 1999a). The environmental impacts associated with the zero discharge of OBFs can be more harmful than the discharge of SBFs due to non-water quality environmental impacts, like air pollution and ground water pollution in the case of incineration and land based disposal, respectively (U.S. EPA, 1999a). A qualitative comparison of OBFs, WBFs and SBFs for various offshore/onshore activities is given in Table 2.2. Various environmental factors are ranked from scale 0 to 3. A bigger value represents the higher risk potential involved in that activity (or factor).

| Activity                      | Risk Type           | Factors                       | OBFs | WBFs | SBFs |
|-------------------------------|---------------------|-------------------------------|------|------|------|
|                               | Occupational        | Accidents                     | 2    | 3    | 2    |
|                               |                     | Chemical exposure             | 2    | 1    | 1    |
|                               | Public              | Air emissions                 | 1    | 2    | 1    |
| Drilling                      |                     | Air emissions                 | 1    | 2    | 1    |
|                               | Environmental       | Spills                        | 2    | 1    | 1    |
|                               | Energy use          |                               | 1    | 2    | 1    |
|                               |                     | Accidents                     | 2    | 1    | 1    |
|                               | Occupational        | Chemical exposure             | 2    | 1    | 1    |
| Onsite                        | Public              | Bioaccumulation and ingestion | 0    | 1    | 0    |
| discharge/                    |                     | Water column effects          | 0    | 1    | 0    |
| solids control                | Environmental       | Bioaccumulation and effect    | 0    | 1    | 0    |
|                               |                     | Benthic effects               | 1    | 1    | 1    |
|                               | Energy use          | 1                             | 1    | 1    |      |
|                               | Occupational        | Accidents                     | 3    | 0    | 1    |
|                               |                     | Chemical exposure             | 2    | 0    | 1    |
|                               | Public              | Air emissions                 | 1    | 0    | 0    |
| Loading and<br>transportation |                     | Accidents                     | 1    | 0    | 1    |
|                               | Environmental       | Spills                        | 3    | 0    | 1    |
|                               |                     | Water emissions               | 1    | 0    | 0    |
|                               |                     | Air emissions                 | 2    | 0    | 0    |
|                               | Energy use          |                               | 2    | 0    | 0    |
|                               | Quantization        | Accidents                     | 3    | 0    | 0    |
|                               | Occupational        | Chemical exposure             | 2    | 0    | 0    |
|                               | Dublic              | Air emissions                 | 2    | 0    | 0    |
| Onshore                       | Public              | Groundwater contamination     | 1    | 0    | 0    |
| disposal                      | Facilitation        | Air emissions                 | 2    | 0    | 0    |
|                               | Environmental       | Groundwater contamination     | 1    | 0    | 0    |
|                               | Energy use          | 2                             | 0    | 0    |      |
| Resource impa                 | cts (Landfill space | /injection capacity)          | 2    | 1    | 0    |
| Economic imp                  | acts                |                               | 3    | 1    | 2    |
| Liabilities                   |                     |                               | 3    | 1    | 1    |

# Table 2.2. A qualitative comparative assessment for drilling fluids in different activities (modified after Meinhold, 1998)

ole value of risk

1: Low value of risk 2: Medium value of risk 3: High value of risk

The volume of drill cuttings generated depends on the dimensions of the well and percent washout, the type of formation being drilled, the type of drill bit, and the type of drilling fluid used (Meinhold, 1998). The discharge of cuttings with adhering drilling fluids and additional minor quantities of attached fluids is not constant, as cuttings are discharged only during drilling periods. The amount of drilling fluids adhering to discharged cuttings and subsequently released from the waste treatment process is difficult to estimate in general, but the quantity of drilling fluid used is directly proportional to the depth of the well.

The Minerals Management Service (MMS, 1989) classifies wells based on the depth of the water column. The wells in water deeper than 300 m are called deep water wells. Wells in less than 300 m are shallow water wells. The U.S. EPA (2000a) studied three specific offshore areas: the Gulf of Mexico. California. and Alaska's Cook Inlet. The analysis was further categorised on the basis of current discharge practice (10.2% attachment of SBF to the rock cuttings) and two proposed discharge options (approximately 4%). The details of the quantities of attached drilling mud and expected drill cuttings for different model wells are summarised in Table 2.3 (U.S. EPA, 1999a, b).

| Model Well          | Hole<br>Diameter<br>(cm) | Depth<br>Interval<br>(m) | Volume of Cuttings<br>(including 7.5% washout)<br>(m <sup>3</sup> ) | Volume of 10.2%<br>attached base fluid<br>(m <sup>3</sup> ) |
|---------------------|--------------------------|--------------------------|---|---|
| Shallow Development | 22                       | 2300                     | 95  | 38.4  |
| Shallow Exploratory | 15-31                    | 450-1825                 | 190   | 80.4  |
| Deep Development    | 22-31                    | 600-1375                 | 137   | 58.1  |
| Deep Exploratory    | 22-45                    | 600-1375                 | 305   | 129.1   |

Table 2.3. Summary for model wells (modified after U.S. EPA, 2000a)

Research into alternative biodegradable base fluids started with common vegetable oils including oil from peanuts, rapeseeds and soya beans. Esters were found to be the most suitable naturally derived base fluids in terms of potential for use in drilling fluids. Esters are exceptional lubricants, show low toxicity and a high degree of biodegradation both aerobically and anaerobically. There are various fatty acids and alcohols from which esters can be synthesised. Following the success of esters, other drilling fluids were formulated that were classified as synthetics. These new products have not matched the environmental performance of ester based drilling fluids (Hall. 2000). A comparison of SBFs based on toxicity ( $LC_{i0}$  - lethal concentration to kill 50% of organisms), potential for bioaccumulation ( $K_{OW}$ , octanol-water partitioning coefficient) and biodegradation under aerobic and anaerobic conditions is shown in Table 2.4. Higher values of  $LC_{i0}$ show lower toxicity and smaller values of  $K_{OW}$  represent lower bioaccumulation potential. Some of the olefin fluids are still in use, but in many areas of the world their discharge on cuttings is being restricted because they do not show sufficiently good performance in terms of biodegradation and toxicity (Hall, 2000).

Another aspect of ester comparison with other synthetic based drilling fluids is the human contact with the drilling fluids during treatment and drilling operations. Skin contact occurs on the drilling floor, and it can also be inhaled through vapors in the shale shaker room. People working on the drill floor may have extensive contact with the drilling fluid and there is a potential danger of splash in the eyes. SBFs have higher boiling points and flash points and reduced vapor pressure than OBFs. SBFs vapors concentrations reported in the North Sea are less than 10 mg/m<sup>3</sup> (Meinhold, 1998). Table 2.5 presents occupational health data reported by drilling fluid manufacturers.

|                 |  |                       |                        | L   | ethal Concen                 | tration (LC)               | o in mg/kg dr                   | y weight of    | sediment)1       |                  |         |                   |    |  |
|-----------------|--|-----------------------|------------------------|---|------------------------------|----------------------------|---------------------------------|----------------|------------------|------------------|---------|-------------------|----|--|
| Fluid           | Fluid Year Source Carbon<br>Type Atoms | Source                | Source Carbon          | rce Carbon 10-d <i>LP</i> <sup>*</sup> 96-hr <i>LP</i> <sup>b</sup> | scd                          | sed are                    | K=                              | $K = (\%)^{3}$ | (%) <sup>4</sup> | Status           |         |                   |    |  |
| Type            |  | U.S. EPA              | Baroid<br>(Hall, 2000) | U.S. EPA  | Baroid<br>(Hall, 2000)       | 10-d CV <sup>e</sup>       | mg/L                            | mg/L           | LOGKow           | Biodeg.          | Biodeg. |                   |    |  |
| Esters          | 1989                                   | Palm<br>Kernel<br>Oil | 8 - 14                 | = 5700-<br>10200  | = 9900-<br>10100             | = 14300-<br>22000          | ≈ 20,000                        | >10000         | 60000            | 50000            | 1.69    | 82                | 80 | Usage<br>increasing<br>globally          |
| PAO             | 1991                                   | Natural<br>Gas        | 20 - 30                | >10000  |                              |                            |                                 | >10000         | 3900             | >50000           | 15.4    | 52                | 12 | Poor<br>Biodegradation                   |
| LAO             | 1992                                   | Natural<br>Gas        | 14 - 18                | = 550-900   | = 150-400                    | = 1000                     | ≈ 150-<br>1000                  | 1270           | >10000           | >10000           |         | 72                | 20 | Being<br>increasingly                    |
| ю               | 1994                                   | Natural<br>Gas        | 15 - 18                | ≈ 1650-<br>2500   | = 350-650                    | ≈ 1100-<br>2900            | = 350-<br>2000                  |                | 2050             | >10000           | >9.0    | 68                | 34 | regulated<br>against                     |
| 1A <sup>6</sup> | Petro<br>PUR<br>I                      | EDRILL<br>A-35        | 11-21                  | >400,0<br>Rainbe<br>(90   | 00 mg/L<br>ow troad<br>5-hr) | >1,000,0<br>Mysidop<br>(90 | 000 mg/L<br>osis bahia<br>5-hr) | 1644<br>(mg/L) | 196.3<br>(72-hr) | >2000<br>(48-hr) | 1.1     | 68.1 <sup>5</sup> |    | Being used on<br>East Coast of<br>Canada |

# Table 2.4. Properties of synthetic based drilling fluids

<sup>1</sup>Lethal concentration (LC<sub>50</sub> in mg/kg dry weight of sediment unless otherwise stated)

\*10-d static toxicity test with the marine amphipod Leptocheirus plumulosus,

<sup>b</sup>96-hr static toxicity test with the marine amphipod Leptocheirus plumulosus,

<sup>6</sup>10-d static toxicity test with the sediment worker Corophium volutor,

<sup>4</sup>Toxicity test with the marine algae Skeletonema costatum, and

"Toxicity test with the marine copepods Acartia tonsa

<sup>2</sup> Bioaccumulation (Octanol-water partitioning coefficient)

<sup>3</sup> OECD 306 protocol for base fluid, conditions are untypical of cutting piles

<sup>4</sup> Protocol ISO/DIS 11734, conditions are similar to cutting piles

<sup>3</sup> OECD 301-B (after 28 days)

<sup>6</sup> Iso-alkane, Material Safety Data Sheet (Terra Nova, 2000)

Table 2.5. Material safety data sheet (MSDS) of synthetic based drilling fluids

| Base<br>Fluid | Flash point<br>(C) | Boiling point<br>(C) | Vapor pressure<br>(mm of Hg @<br>20C) | Human health hazard  |
|---------------|--------------------|----------------------|---------------------------------------|--|
| Esters        | 179                | 2341                 | ≈ 0.066 <sup>1</sup>                  | <ul> <li>Slight eye and skin irritation</li> <li>Low acute inhalation toxicity</li> <li>Ingestion causes irritation,<br/>nausea, vomiting and diarrhea</li> </ul>  |
| PAO           | 81                 | 218-251              | <0.1                                  | <ul> <li>Repeated and prolonged<br/>exposures cause skin irritation<br/>and dermatitis</li> </ul>  |
| LAO           | 113                | 245-279              | 0.1                                   | <ul> <li>Aspiration hazard</li> <li>Causes drowsiness</li> </ul>   |
| Ю             | 118                | 240                  | 0.05<br>(at 40C)                      | Aspiration hazard     Nuisance particulates     Repeated and prolonged     exposures cause eye and skin     irritation and dermatitis     Ingestion cause irritation,     nausea, vomiting and cough   |
| ²IA           | > 135              | 256-295              | 0.001                                 | Repeated and prolonged<br>exposures cause eye and skin<br>irritation     Repeated inhalation of<br>excessive amount may cause<br>irritation of respiratory tract<br>Oil deposits in lungs may cause<br>fibrosis and pulmonary<br>dysfunction |

(modified after Meinhold, 1998)

<sup>1</sup>Calculated by Quantitative Structure Activity Relationships (QSARs) (Lyman et al., 1990)

2 Material Safety Data Sheet (Terra Nova, 2000)

The waste stream discharged from drilling operations that use SBFs or other non-aqueous drilling fluids consists of three main components: drilling fluid, drill cuttings, and formation oil. The U.S. EPA (1999a) has analyzed pollutant reduction options, compliance costs, and non-water quality environmental impacts, which are based on the drilling waste characteristics data given in Table 2.6. Formation oil is the only possible source of organic priority pollutants in the SBF cutings waste stream. Formation oil contamination is an indicator pollutant of PAHs (U.S. EPA, 2000a). It includes naphthalene, fluorene, phenantherene and phenol. The estimated average adhering drilling fluid on the SBF-cuttings waste stream contains 0.2% formation oil by volume of SBF. Since the composition of formation (crude) oil varies widely, diesel oil is used to estimate the organic pollutant concentrations associated with formation oil (U.S. EPA, 2000a).

| Priority Polla | atant Organics  | mg Pollutant/ml Formation Oil |                 |  |  |  |
|----------------|-----------------|-------------------------------|-----------------|--|--|--|
| Naphthalene    |                 | 1.43                          |                 |  |  |  |
| Fluorene       |                 | 0.78                          |                 |  |  |  |
| Phenantheren   | ne              |                               | 1.85            |  |  |  |
| Phenol         |                 | 6.0                           |                 |  |  |  |
| Metals         | mg/kg of Barite | Metals                        | mg/kg of Barite |  |  |  |
| Cadmium        | 1.1             | Lead                          | 35.1            |  |  |  |
| Mercury        | 0.1             | Nickel                        | 13.5            |  |  |  |
| Antimony       | 5.7             | Selenium                      | 1.1             |  |  |  |
| Arsenic        | 7.1             | Silver                        | 0.7             |  |  |  |
| Beryllium      | 0.7             | Thallium                      | 1.2             |  |  |  |
| Chromium       | 240.0           | Zinc                          | 200.5           |  |  |  |
| Copper         | 18.7            |                               |                 |  |  |  |

Table 2.6. Waste characteristics of SBF-cuttings

Barite is used as a weighting agent and is the primary source of heavy metals in the drilling waste. The trace metal concentrations depend on the source of barite. The U.S. EPA and U.S. Department of Energy (U.S. DOE) have reported averaged trace metals concentrations based on data from more than 500 wells as shown in Table 2.6 (U.S. EPA, 1995, 2000a; Meinhold, 1998). Naturally occurring radioactive materials (NORM), such as potassium, strontium, uranium, thorium and radon can also be present in the formation oil, but are not reported by the U.S. EPA.

# 2.3. CONTAMINANTS FATE MODELING

Various processes, concepts and approaches have been used in determining the fate of drilling waste discharges in receiving water bodies. The present literature search indicates that no fully validated drilling waste discharge model exists which describes all the principles and processes in an appropriate framework. The transport and fate of drilling waste is a complex process, that is still only partially understood (Khondaker, 2000). Generally physical processes are employed in the fate modeling and other processes (and aspects) including chemical, biological and ecological are not incorporated. The transport of drilling waste involves (1) advection. (2) dispersion, (3) flocculation, (4) deposition. (5) consolidation, (6) erosion, and (7) resuspension. The impacts of these processes on the fate of drilling waste depend on the characteristics of the waste and the hydrodynamics of the receiving water bodies. The salient features and modular structure of such models are summarized in Figure 2.3. This structure is valid for those models, which consider only obvisical transport processes.

The other type of modeling approach is to consider the fate of contaminants based on their physico-chemical characteristics. The exposure concentrations of drilling waste chemicals using dilution factors and further partitioning of chemicals in the water column and sediments are estimated based on their bioaccumulation potential ( $K_{ow}$  and bioconcentration factors). These types of modeling approaches are popular in ecological risk assessment of wastes generated from offshore drilling operations. In the following sections a discussion of a few of the present state of the art modeling approaches is presented.



Figure 2.3. Important features of drilling waste transport models (modified after Khondaker, 2000)

#### 2.3.1. U. S. EPA APPROACH

In the U.S. EPA study (1999a, b; 2000a, b), the SBF-cuttings discharge impact and water quality assessments rely on OOC (Offshore Operator's Committee) modeling data. Brandsma (1996) presented a model for the transport behavior of oil and solids from cuttings contaminated with oil-based fluids. Due to the similar hydrophobic (lower solubility in water) and physical properties (like vapor pressure and density) of SBFs and OBFs, the dispersion behavior of SBF-cuttings was assumed to be the same as those of OBF-cuttings (U.S. EPA, 1999a; Sadiq et al., 2000). In the water column, total organic pollutant discharge concentrations were assumed to represent the soluble concentration. Metals were assumed to leach or dissolve immediately into the water column with pollutant specific amounts determined for mean seawater pH.

Brandsma's study (1996) in the North Sea presented oil concentration as a function of transport time (Figure 2.4). Brandsma (1996) used 5.5% (by wet weight) oil content as the effluent to calculate the extent of dilutions available at various distances from the disposal location. Based on the mean OBF-cuttings density and the estimated initial oil concentration, available dilutions were determined at various distances from the disposal point. The water column pollutant concentrations at the edge of a mixing zone (at 100 m from the discharge) were calculated by dividing the drilling waste pollutant concentration by the dilution factor. The effluent concentrations for metals were further adjusted by a leaching factor (*LF*) to account for the portion of the total pollutant concentration that was dissolved and therefore available in the water column.



Figure 2.4. Oil concentrations as function of transport time (after Brandsma, 1996)

The US EPA (1999b) pore water quality analysis characterized sediment pollutants through a number of field surveys and the SBF itself was used as the marker for the pollutant presence due to its less dispersive behavior. In order to determine SBF-cuttings pollutant concentrations, it was assumed that the relative proportional concentrations between the base fluid and other pollutants were maintained after discharge and transport. The pollutant concentrations were converted into pore water concentrations using an equilibrium partitioning approach (EqP). For metals, the mean seawater leach factors of trace metals in barite were employed and for organic pollutants, partition coefficients were used to calculate the pore water concentrations. The partitioning coefficients estimate the ratio of sediment concentration to pore water concentration. The sediment concentration multiplied by the pollutant specific leach factor, or the inverse of the partitioning coefficient, gave the amount of pollutant available in the pore water. The analysis was conservative due to the assumption that discharged pollutants immediately leach into the water column or into pore water (interstitial water). Sadiq et al. (2000; 2001a, b) also used results of the OOC model for determining the fate of drilling waste for hypothetical case studies.

#### 2.3.2. DEPOSITION MODELING

Seaconsult (2000) conducted a study for the Canadian offshore for the deposition modeling of synthetic base fluids attached to drill cuttings. The model predicts the SBF concentration (mg/kg of dry sediment) and thickness of cuttings on the seabed. The basic input for the model is the dry weight of discharged material. This model solves a regular Cartesian grid with a mesh size of 100 m. The model predicts the position of the contaminant based on the drill cutting size and density. The governing transport equation is solved using a particle-tracking technique. A set of discrete particles is released over time, and each particle and associated mass is defined. The procedure involves Monte Carlo (MC) simulations for simulating diffusive effects. The deposition model tracked the discharge of a single sediment class with a known characteristic grain size ( $\delta$  in mm). The settling velocity (w) for particles more

than 100  $\mu$ m was defined by  $4.2 \times \delta^3$ , and for smaller size particles a constant value of 0.0012 m/s was assumed. The basic equations used for calculating base fluid concentration in the sediments and thickness of deposition on the seabed are given in Table 2.7.

The deposition model assumes that oil will remain attached to the cuttings during transport. The U.S. EPA (1999a) has made a similar assumption. This model determines

the fate of SBF without considering the partitioning of base fluid within the multimedia. The resuspension, biodegradation, adsorption and other processes are not addressed in this model.

# Table 2.7. Deposition model for determining synthetic oil concentration in the sediments (Seaconsult, 2000)

| $O_{jk} = \frac{W_{jk} \times 1000}{A}$  | (2.1)         |
|--|---------------|
| where<br><i>j</i> and <i>k</i> are <i>x</i> and <i>y</i> coordinates of the model cell/mesh, respectively. |               |
| $W_{j,k} = dry weight (kg) in cell j and k$  |               |
| A = Area of a cell (1 ha or 10,000 m <sup>2</sup> )  |               |
| $O_{j,k}$ = Deposition density or dry weight of sediment (g/m <sup>2</sup> )                               |               |
|  |               |
| $h_{j,k} = \frac{O_{j,k}}{\gamma}$   | (2.2)         |
| where  |               |
| $\gamma = \text{ in situ bulk density (1850 kg/m3)}$   |               |
| $h_{j,k}$ = deposition thickness (mm) in cell j and k  |               |
| $C_{j,k} = \frac{C_{al} \times W_{j,k}}{A \times h_a \times (I - n) \times \gamma_x}$                      | (2.3)         |
| where  |               |
| $\gamma_{\rm s}$ = specific weight of mineral sediment (2600 kg/m <sup>3</sup> )                           | 김 씨에서 동양을     |
| n = seabed porosity (0.4)  |               |
| $h_m$ = sediment mixing depth (0.08m)  | 그는 바람이 잘 수 있는 |
| $C_{oil}$ = initial oil loading (10 <sup>5</sup> mg/kg)  |               |
| $C_{j,k} = \text{SBF}$ concentration (mg/kg of dry sediment of sample)                                     |               |

## 2.3.3. bblt MODEL

Hannah et al. (1997) formulated and tested a basic bblt (benthic boundary layer transport) model. The key feature of the model is that it includes the process of vertical shear dispersion. The basic local bblt model assumes no horizontal variations in the ocean environment and allows continuous and bulk releases of sediment. The latest version, a "spatially-variable bblt" model, allows full spatial variability in water depth, currents, and bottom stress (boundary layer). These models partition the sediment load (drilling waste assumed on the seabed) into discrete pseudo-particles with mass *m* and velocity *w*. Shuffling of the packets represents vertical mixing which is defined by tidal currents. The bblt model output is the concentration profile throughout the depth based on the position of packet *N*. These models assume that drilling particles act as separate particles and are distributed based on particle size distribution. Another shortcoming of these models is that they do not consider the partitioning of contaminants within the water column and sediments.

#### 2.3.4. PROTEUS (2001)

This model has been developed to simulate the dispersion of drilling waste discharges and subsequently disturbed cutting piles. Proteus (2001) considers the characteristics of discharged particulates and their size, and settling velocity during simulations. The settling velocities data from laboratory experiments have been used for the validation of algorithms used in Proteus to predict settling velocity. This model is used for water based drilling fluids in which desegregation of particulates after discharge happens.

This model simulates a 3-dimensional physical mixing process, which includes the initial dynamic discharge, the effects of density differences in the plume and surrounding fluid, and the transport by the current. This model also considers dynamic partitioning between particles and seawater, degradation and volatilization.

#### 2.3.5. CHARM MODEL

The CHARM (Thatcher et al., 1999) model addresses only water based drilling chemicals. The *PEC*<sub>water</sub> (Predicted Environmental Concentration) is calculated for continuous and batch discharges. The *PEC*<sub>water</sub> values for both systems are calculated by simple mass balance equations by considering refreshment rate and dilution factors around the point of discharge. The refreshment rate is defined by dividing the residual current speed by the radius around the platform. Table 2.8 presents the summary of calculations for determining concentrations in the water column and sediments. The *PEC*<sub>watherer</sub> is defined by using the water-sediment partitioning coefficient (*K*<sub>10</sub>). *PEC*<sub>watherer</sub> is further adjusted by biodegradation, which is arbitrarily selected for one-year time tore duce the predicted environmental concentration. The CHARM model uses a simplified approach for calculating exposure concentrations and does not address the fate of SBFs and OBFs in the marine system.

#### 2.3.6. MASS TRANSPORT MODEL

Thibodeaux et al. (1986) have developed a mass transport model for the chemical contaminants originating from offshore drilling deposits. This model was limited to the transport of the soluble constituents of the cuttings and mud deposits. This model did not take into account particulate transport and chemical and biochemical transformations that degrade contaminants within the sediment zone or boundary layer.

#### Table 2.8. Concentration calculations for CHARM model (modified after Thatcher et al., 1999)

Dosage expressed as weight percentage  $M = W_t \times V_* \times \rho_*$ (2.4) where amount (mass) of pollutant discharged (kg) м Wr weight percentage of the pollutant in the mud = V., = volume of mud discharged for the specific section (m') = density of the discharged mud (kg/m') ρ., Volume of ambient water available for dilution 1 (2.5) V . = ---platform density where  $V_p$ volume of ambient water per platform (m') Plat form density number of platforms per square kilometre (1/km) = Water depth average water depth around the platform (m) Refreshment rate of the ambient water r = 24 × 3600 (26) 2×Y/. where r fraction of sea water refreshed in the area around the platform (1/day) Y = radius around platform (m) U = residual current speed (m/s) The volume of water passing the platform during the period of drilling a section  $V = V, \times r$ 1271 where V. volume of water passing the platform (m<sup>3</sup>/d) = v, volume of ambient water per platform (m1) r = fraction of sea water refreshed in the area around the platform (1/day) PEC\_\_\_\_\_ for continuous discharges can now be calculated using  $PEC_{max} = \frac{M}{T \times V} \times 10^{10}$ (2.8)where PEC water, com PEC not for continuous discharges (mg/L) = м amount (mass) of pollutant discharged (kg) -T time needed to drill a section (d) V. = volume of water passing the platform (m'/d) =  $PEC_{water cont} \times K_{1w} \times (1 - d_{1.05})$ PEC (2.9) where PEC PEC in the sediment around the platform (mg/kg) PEC mater, com PEC\_mer for continuous discharges (mg/L) = Knw = sediment-water partition coefficient (L/kg) dus fraction of a substance in sediment that is degraded in one year -

#### 2.3.7. FUGACITY BASED MODELS

Environmental thermodynamics generally applies to a narrow temperature range between 0° to 25C° and the pressures are generally atmospheric except for additional hydrostatic pressures underwater. These special conditions in environmental fate modeling help in simplifying the complex thermodynamic models dealing with the fate of contaminants (Mackay, 1991). Generally two approaches are used to develop equations relating equilibrium concentrations in two phases. The most common approach is Nemest's Distribution Law, which states that the concentration ratio (C<sub>1</sub>/C<sub>2</sub>) is relatively constant and equal to partition/distribution coefficient K<sub>12</sub>. Linear and non-linear equations can be fitted to the data to calculate the rate constants. The second approach needs an intermediate parameter, a criterion of equilibrium, which can be related to C<sub>1</sub> and C<sub>2</sub>, separately. Chemical potential, fugacity, and activity are common parameters. Fugacity is the most common criterion for organic substances (Mackay, 1991).

In the environment, chemicals migrate from one phase to another. The fraction of a compound between the different phases at equilibrium is determined by minimization of Gibbs's free energy (G) (Moran and Shapiro, 1988). G is a function of enthalpy (h) and entropy (S).

$$\frac{G}{n} = g = h - TS \tag{2.10}$$

where

| G = Gibbs's free energy (Joules)            |
|---|
| h = Enthalpy (Joules/mole)                  |
| $T = \text{Temperature} (\text{K}^{\circ})$ |
| S =Entropy (Joule/mole·K°)                  |
| n = Number of moles                         |
| g = Gibbs's free energy/mole (Joules/mole)  |
|   |

In differential form above equation reduces to

| dg = dh - d (TS)                | (2.11) |
|---------------------------------|--------|
| where                           |        |
| dh = v dP + T dS                | (2.12) |
| and                             |        |
| d(TS) = T  dS + S  dT           | (2.13) |
|                                 |        |
| Therefore equation 2.11 becomes |        |

| dg = (v dP + T dS) - (T dS + S dT) | (2.14) |
|------------------------------------|--------|

| dg = v dP - S dT | (2.15) |
|------------------|--------|

For an ideal gas

$$P_{V} = RT$$
 (2.16)

where

v =Volume per unit mole (V/n), and R =Universal gas constant

Therefore equation 2.15 becomes

$$dg = \frac{RT}{P}dP - S dT \tag{2.17}$$

If temperature (T) is constant

$$dg = RT d(LnP)_{\rm T} \tag{2.18}$$

The ideal gas behavior is applicable over a limited range of pressure and temperature, therefore to account for non-ideal behavior a property called fugacity (f) is introduced. Therefore equation 2.18 becomes

$$dg = RT d(Ln f) \tag{2.19}$$

Fugacity has units of pressure and reduces to pressure as ideal gas conditions are approached (i.e., pressure  $P \rightarrow 0$ ).

$$\lim_{p \to 0} \frac{f}{p} = l \qquad (2.20)$$

Therefore final relation becomes

$$RT d(Ln f) = v(d P)_{\gamma}$$

$$(2.21)$$

Fugacity may now be determined by using equations for pressure or volume, which account for non-ideal behavior. Therefore the molar concentration and fugacity relationship can be defined from an ideal gas law.

$$C = \frac{n}{V} = \frac{P}{RT} = \frac{f}{RT} = Zf$$
(2.22)

where Z is the fugacity capacity and establishes values for each chemical in each phase. This process starts from the air phase  $(Z_i)$  in which it can be shown that

$$Z_A = \frac{l}{RT}$$
(2.23)

If an air-water partition coefficient  $K_{sw}$  is available, the value of Z for a chemical in water can be calculated by taking  $f_i$  (air fugacity) equal to  $f_w$  (water fugacity).

$$C_{u} = \frac{C_{A}}{K_{AW}} \therefore \text{ Generally, } C_{2} = C_{1}K_{21}$$
(2.24)

It is thus possible to calculate Z, values for chemicals in soil, fish, and sediments if the partitioning coefficients are known. The relationships have been developed for partitioning coefficients of several organic chemicals as a function of chemical properties, such as solubility in water, K<sub>ow</sub> and vapor pressure. The expressions of these relationships are given in Table 2.9.

| Compartment                                       | Compartment Definition of Z (mol/m <sup>3</sup> -Pa)   |  |  |  |  |
|---|--|--|--|--|--|
| Air   | I/RT<br>R = 8.314 mol/m <sup>3</sup> Pa and<br>T = Temperature (K)   |  |  |  |  |
| Water   | <i>1/H</i> or $C^3/P^3$<br>$C^5 = Aqueous solubility (mol/m3)$<br>$P^5 = Vapor pressure (Pa)$<br>H = Henry's law constant (Pa. m3/mol) |  |  |  |  |
| Solid sorbent (e.g. soil, sediment and particles) | $K_P \rho_5 /H$<br>$K_P = Partition coefficient (L/kg)$<br>$\rho_5 = Density (kg/L)$   |  |  |  |  |
| Biota (fish)                                      | $K_{g} \rho_{g} / H$<br>$K_{g} = \text{Bioconcentration factor (L/kg)}$<br>$\rho_{g} = \text{Density (kg/L)}$                          |  |  |  |  |
|   |  |  |  |  |  |

Table 2.9. Definition of fugacity capacities (Z)

Fugacity concepts have been successfully applied in pharmacokinetics, food uptake of plants, sewage treatment plants, and regional distribution of pollutants (Mackay, 1991; Mackay et al., 1992; Mackay and Paterson, 1990). The fugacity approach is convenient for complex ecological systems and has been extensively used in modeling the distribution of contaminants in multimedia environments. The use of fugacity instead of concentration defines the equilibrium status of phases and the direction of diffusive transfer.

Mackay (1991) developed a methodology for estimating chemical distributions in multimedia based on the level of complexity of the systems, namely

Level I: Equilibrium, Steady state, and No-flow system, Level II: Equilibrium, Steady state, and Flow system, Level III: Non-equilibrium, Steady state, and Flow system and Level IV: Non-equilibrium, Unsteady state, and Flow system.

In Level L the equilibrium partitioning of a fixed amount of non-reacting compound is calculated using fugacity capacities that are derived from physico-chemical data. The equilibrium ratios of concentrations and the relative amounts in each phase can be calculated. In Level II, the steady state equilibrium concentrations are calculated for fixed emissions that are balanced by reactions of various types in each phase. This provides the information on the compound's overall persistence and about the proportion that reacts in each phase. The critical limitations of Levels I and II are the assumptions of achieving equilibrium between all phases. In practice, there is a transfer resistance that limits the transfer from one phase to another and then tends to contain the chemical within that phase. Interphase transport resistance defines Level III in which each phase may have a

different fugacity. Level IV introduces the concept of inflows (emissions) that change with time, i.e. non-equilibrium and unsteady state condition. If emissions are constant, the Level IV will approach Level III (Mackay, 1991).

#### 2.4. POTENTIAL FUGACITY/AQUIVALENCE BASED APPROACH

A level III version of the fugacity model called the QWASI model was developed by Mackay et al. (1983) and applied to rivers and lakes, specifically for detergent chemicals, PCBs and heavy metals. In the OWASI model, the steady state solution describes conditions that will be reached after prolonged exposure of the system to constant input conditions. The use of fugacity as an equilibrium criterion is suitable for chemicals that can establish measurable concentrations in the vapor phase. It is not applicable to some metals, organometals, ionic compounds, nor some organics such as polymers whose vapor pressure is negligible. To model the behavior of these chemicals, another equilibrium criterion analogous to fugacity is used. This criterion is known as the aqueous equivalent concentration (aquivalence) criterion, which is suitable for both volatile and non volatile chemicals (Mackay and Diamond, 1989). A model developed in terms of aquivalence is ultimately similar to models written in terms of concentration or fugacity. The aquivalence model has been applied to the multi-phase distribution of inorganic chemicals assuming they behave as single species, such as total lead. The fugacity/aquivalence approach has been extensively applied to Lake Ontario and Hamilton Harbor for various heavy metals and organic chemicals (Mackay and Diamond, 1989). The OWASI model takes into account advective flow, volatilization, sediment

deposition, resuspension, and burial, sediment-water diffusion, and transformation reaction processes (Mackay et al., 1983).

In levels higher than Level I, the rates of loss through reaction and advection are considered. The degradation rates for chemicals are defined by a first order rate constant (k) or calculated from a half-life ( $H_{1/2}$ ) in each medium. The transformation and transportation processes are calculated in terms of *D*-values. The rate constant k ( $hr^{-1}$ ) is used to calculate a reaction *D*-value  $D_R$  in each medium. The  $D_R$  is defined as

$$D_8 = V_1 \times Z_1 \times k_1$$
 (2.25)

where

- $V_t = \text{Volume}(\text{m}^3)$
- $Z_i = Fugacity capacity (mol/m<sup>3</sup>-Pa)$
- $k_t = \text{Reaction rate constant (hr')}$
- $i = i^{th}$  medium

The rate of reactive loss ( $E_R$ , mol/hr) is calculated by multiplying  $D_R$  with fugacity, f. Similarly, for calculating advection losses, flow rates (Gi), or detention times are defined. The D-value for advection ( $D_A$ ) is defined as  $Gi \times Zi$  and the advection rate of loss ( $E_A$ , mol/hr) as the product of  $D_A$  and f. At Level III additional diffusive and non-diffusive processes of intermedia transport are taken into account. In conventional 4 media systems, Mackay and co-workers have defined 12 intermedia transport velocity parameters ( $U_1$  to  $U_{12}$ ). These parameters are used to calculate the intermedia transport *D*-values. Mackay (1991) defined four primary compartments namely, air, water, soil and sediments. The air is defined as a mixture of aerosols; water as mixture of suspended particles and fish; soil as solids, water and air; and sediments as solids and pore-water. Different media are assigned the following numerals:

| Media            | Numeral |
|------------------|---------|
| Air              | 1       |
| Water            | 2       |
| Soil             | 3       |
| Sediment         | 4       |
| Suspended solids | 5       |
| Fish             | 6       |

Air and soil compartments are not relevant to the present research, which will deal mainly with two primary compartments, i.e., water and sediments. The intermedia mass transport means water-to-sediment diffusion and advection through interface and vice versa. Different mechanisms of transport and transformation in the two-compartment model are shown in Figure 2.5. Intermedia transport velocities  $U_8$ ,  $U_9$  and  $U_{10}$  represent sedimentwater mass transfer through diffusion, sediment deposition and sediment resuspension, respectively. The intermedia *D*-values for water to sediment ( $D_{22}$ ) and sediment to water ( $D_{c2}$ ) are calculated by relations given in Table 2.10.  $D_{24}$  accounts for mass transfer from water to sediment and deposition of suspended solids. Similarly  $D_{42}$  accounts for mass transfer from sediment to water and resuspension of sediments to the water column. The kinetics of resuspension and deposition of suspended solids (SS) are discussed in the next chapter using Huang's (1992) experimental results.



Figure 2.5. Water sediment interaction model

The advection and reaction *D*-values in water and sediments are calculated based on flow rate and half-life of pollutants. First order kinetics is applied to determine the reaction rate constant and *D*-values ( $D_{R2}$  and  $D_{R4}$ ). Similarly, knowing the burial rate of sediments and the detention time in the water column,  $D_{A2}$  and  $D_{A4}$  can be calculated, respectively. The calculations of fugacity (*f*) are based on Mackay et al.'s (1992) work, but have been modified to suit a two-compartment system of water and sediment. A systematic procedure for calculating the concentration in water and sediment is summarized in Table 2.10.

| Table 2.10. Procedure for | r calculating concentrations | in water column and | pore water |
|---------------------------|------------------------------|---------------------|------------|
|---------------------------|------------------------------|---------------------|------------|

| No. | Comments and instructions  | Parameters and Equations  |
|-----|--|---|
| 1   | Define physico-chemical properties of pollutant.   | Henry's coefficient (H), solubility (S), $K_{m}$ , etc.   |
| 2   | Define media (sediment and water) physical properties.   | Volume (V) and surface area $(A_W)$ , depths of water column $(d_w)$ and sediment $(\Delta)$ , volume fractions $(v)$ , densities $(\rho)$ etc.   |
| 3   | Define flow rates or advection velocities in<br>water-sediment system. Similarly reaction<br>rates can be determine from half lives ( $H_{1/2}$ ) or<br>kinetic coefficients (k) in water and sediments.                   | $G_i$ (flow rate) = volume/detention time<br>k = 0.693/H <sub>1/2</sub> for first order kinetics  |
| 4   | Calculate the fugacity capacity (Z) of all<br>components.<br>$\rho = \text{organic fraction}$<br>$\rho = \text{density}$<br>$K_{0\sigma} = Portioning coefficient for organiccarbon content$                               | $ \begin{array}{l} Z_{z} = I/H & (H: \operatorname{Henry's coefficient}) \\ Z_{z} = Z_{z}, p, \phi, K_{x}, \\ Z_{z} = Z_{z}, \rho, \phi, K_{x},  \log(K_{x}) = 0.544 \times Log(K_{xx}) + 1.377 \\ Z_{x} = Z_{z}, \rho, \phi, K_{x}, \qquad \qquad$  |
| 5   | Calculate the bulk Z values $(Z_{\theta})$ for primary<br>compartments, i.e., water and sediment<br>v = volume fraction  | $Z_{02} = Z_2 + v_x Z_x + v_y Z_y and$<br>$Z_{04} = Z_2 \times (pore water) + Z_4 \times (solid particles)$   |
| 6   | Calculate process D-values (mol/Pa-hr)<br>Reaction<br>Advection  | $D_{R2} = Z_{R2} V_2 k_2$ and $D_{R4} = Z_{R4} V_4 k_4$<br>$D_{k2} = Z_{R2} G_2$ and $D_{k4} = Z_{R4} G_4$  |
| 7   | Define pollutant loads by direct emissions in water $(E_2)$ and sediments $(E_4)$  |   |
| 8   | Calculate intermedia transport <i>D</i> -values<br>Where $Z_2$ , $Z_4$ , $Z_7$ are fugacity capacities of<br>water, sediment and suspended solids,<br>respectively, and $A_W$ is the area of water-<br>sediment interface. | $\begin{array}{l} D_2 = A_W U_1 Z_2 + A_W U_2 Z_2 \\ D_{L2} = A_W U_1 Z_2 + A_W U_{10} Z_4 \\ U_4 = sediment-water mass transfer coefficient (MT) \\ U_6 = SS deposition \\ U_{10} = sediment resuspension \\ U_{10} = sediment resuspension \\ \end{array}$  |
| 9   | Calculate total D-values   | Water $D_{T_{1}} = D_{R_{2}} + D_{A_{2}} + D_{24}$<br>Sediment $D_{T_{2}} = D_{R_{2}} + D_{24} + D_{24}$  |
| 10  | Making mass balance equations under steady<br>state conditions<br>Water: $E_1 + f_1 D_{i1} = f_1 D_{i2}$ and<br>Sediment: $E_1 + f_1 D_{i2} = f_2 D_{i3}$  | $f_{z} = \frac{E_{z} + \frac{E_{D_{zr}}}{D_{r}}}{D_{r}}  \text{and} \ f_{z} = \frac{(E_{z} + f_{z}D_{yz})}{(D_{rz})}$   |
| 11  | Calculate concentrations (mg/L)<br>MW = Molecular weight   | $ \begin{array}{l} C_w = MW \times Z_2 \times f_2  (water \ column) \\ C_{Fw} = MW \times Z_2 \times f_4  (pore \ water) \\ C_{gw} = MW \times Z_g \times f_2  (bulk \ water \ column) \\ C_{ex} = MW \times Z_g \times f_4  (bulk \ sediments) \end{array} $   |
| 12  | Calculate the emissions through each process<br>in different media   | $ \begin{split} E_{R2} &= D_{R2} f_{2} \\ E_{A2} &= D_{A2} f_{2} \\ E_{A2} &= D_{A2} f_{2} \\ E_{T1} &= D_{A2} f_{1} \\ E_{T2} &= D_{A2} f_{1} \\ E_{T2} &= D_{A2} f_{1} \\ E_{T2} &= D_{A2} f_{2} \\ E_{T2} &= D_{A2}$ |
| 13  | Check total emissions ( $E$ )<br>Calculate total mass ( $M$ )<br>Calculate overall residence time ( $t_0$ )  | $E = E_{1} + E_{4} \qquad \text{and} \\ M = \sum (C \times V) \qquad \text{and} \\ t_{0} = M/E$   |
As mentioned earlier, the use of fugacity as an equilibrium criterion is suitable only for chemicals that can establish measurable concentrations in the vapor phase, but is not applicable to metals, organometals, nor ionic compounds that have negligible or zero vapor pressure. For these types of chemicals an alternative equilibrium criterion. "aquivalent concentration" or "aquivalence" (A) is recommended (Mackay and Diamond, 1989). The (A) criterion has the dimensions of concentration but the Z' values become dimensionless. The simpler way to look at this criterion is the "equivalent equilibrium aqueous phase" concentration, and so the dimensionless Z; value becomes equal to 1. In case of the water phase, (A) is the actual concentration. The Z' for other phases becomes the dimensionless partition coefficients, e.g.

$$Z_{s}^{\prime} = I$$

$$Z_{s}^{\prime} = \rho_{s} K_{\pi}$$

$$Z_{s}^{\prime} = \rho_{s} K_{\pi}$$

$$Z_{s}^{\prime} = \rho_{s} K_{\pi}$$

$$Z_{s}^{\prime} = \rho_{s} K_{\pi}$$
(2.26)

where  $K_{\infty} K_{\pi}$  and  $K_{\theta}$  are the partitioning coefficients for sediment, particulate matter and bioconcentration factor for biota. respectively. The procedure of calculating concentrations as described in Table 2.10 will remain the same but parameters of Z and f will be replaced by Z' and A, respectively. This chapter has discussed main research issues: firstly, various types of drilling fluids and their characteristics were discussed from ecological and human health risk viewpoints: secondly, various state of the art modeling approaches used in determining the fate of drilling waste in the marine environment were discussed. In the end, a step by step methodology was presented for a proposed water-sediment interaction model that will be used in the next chapter for applying a probabilistic approach for determining the fate of drilling waste.

## Chapter 3

# METHODOLOGY FOR PROBABILISTIC FATE MODELING

This chapter deals with various steps involved in the development of a methodology for fate modeling. Dnilling waste is a mixture of various types of pollutants. The physicochemical characteristics of the pollutants and the multimedia environment in which they disperse, govern the ultimate fate of the pollutants. The variability and uncertainty in input variables and the model's structure determine the uncertainty in the model's outputs. A parametric based probabilistic approach is adopted for a fugacity/aquivalence based water sediment interaction model in which the model inputs are defined by statistical distributions.

Monte Carlo (MC) simulations using Latin Hypercube Sampling (LHS) were performed to determine the model outputs: water column concentration ( $C_w$ ), pore water concentration ( $C_{rw}$ ), and their associated uncertainties. The sensitivity analysis (SA) was performed using the rank correlation coefficient ( $\rho_c$ ) and by estimating the percent contribution of input variables to the variance of the model's output. The key input variables of the water sediment interaction model were identified. In the end, multiple linear regression models were developed to predict environmental concentration in multimedia as a function of pollutant loading rate (*E*) and radius of impacted area (*R*). A chemical specific approach was employed in the fate modeling. The pollutants under consideration were arsenic (As), cadmium (Cd), copper (Cu), chromium (Cr), mercury (Hg), nickel (Ni), lead (Pb), zinc (Zn), base fluid (ester), and naphthalene.

In adopting a parametric probabilistic approach using MC simulations for fate modeling, a four-step process was followed. In the first step, a detailed literature search was conducted to determine the values and/or ranges of various inputs to the model, which included physico-chemical characteristics of the pollutants and multimedia properties. The statistical distributions of model inputs were selected subjectively in the second step. Then in the next step. MC simulations were performed to estimate concentrations in the water column and pore water and their associated uncertainties. The key input variables were identified using sensitivity studies in MC simulations results. Finally, empirical regression models were developed to determine the representative environmental concentration of the pollutants in terms of the pollutant loading rates and impact area. This four-step fate modeling process is shown in Figure 3.1. The details of these steps are discussed in the following sections.

## 3.1. PARAMETER SELECTION

The quantities of drill cuttings produced are proportional to the rate of advancement of a drill bit downhole. The U.S. EPA (2000a) estimated the pollutant weights and volumes in the drilling waste streams for various types of model wells from the data provided by the American Petroleum Institute (API). The drill cuttings are brought to the surface and drilling fluids are separated from the cuttings by a solid separation system that may include shale shakers and centrifuges. The drilling fluid is reused provided its rheological characteristics meet the technical requirements of drilling. The drill cuttings, which also carry residual drilling fluid, are typically discharged continuously during drilling operations (Meinhold, 1998).



Figure 3.1. A four-step process of fate modeling methodology

The drill cuttings and the attached drilling fluid are high-density suspended solids, which may cause benthic smothering and sediment grain size alteration upon discharge in the marine environment. In addition to the physical impacts of drilling wastes, the SBFs are comprised of conventional and non-conventional pollutants, which may be toxic in nature and have bioaccumulation potential. The drilling waste may cause hypoxia (reduction in oxygen) in the immediate sediments depending on bottom currents, temperature and rate of biodegradation.

Barite has a specific gravity of 4.6 and is used as a weighting agent in drilling fluid. It accounts for approximately 33% of total SBF weight. Barite consists mainly of barium sulphate, but also contains traces of heavy metals, which include arsenic (As), cadmium (Cd), copper (Cu), chromium (Cr), mercury (Hg), nickel (Ni), lead (Pb), and zinc (Zn). Generally the trace heavy metals present in barite are reported in terms of mg/kg of barite (U.S. EPA, 1999b). The sole source of poly aromatic hydrocarbon (PAH) in the synthetic based drilling waste stream is contamination by formation oil. On average, the formation oil is 0.2% (by volume) of SBFs. Due to the variable composition of formation oils. diesel oil is recommended for estimation of quantities of organic pollutants (U. S. EPA, 2000b).

The following sections will deal with specific physico-chemical characteristics of fugacity/aquivalence-based water sediment interaction model inputs, which are pollutants and media properties.

#### 3.1.1. PHYSICO-CHEMICAL PROPERTIES OF THE POLLUTANTS

The whole toxicity and the chemical specific approaches are generally employed for fate modeling and exposure assessment in the risk assessment framework. In the chemical specific approach, each pollutant present in the waste is treated individually based on its own physico-chemical characteristics. A similar approach is employed by the U.S. EPA (2000a) and in the CHARM model (Thatcher et al., 1999) for drilling wastes fate modeling. The drilling waste as a single entity can be considered in the fate model provided that the physico-chemical characteristics are known. In the absence of the substantial data required for fate modeling, the whole toxicity approach is not possible to implement. Therefore the chemical specific approach was employed in this research.

The behaviour and fate of a pollutant in the environment depends on its physico-chemical characteristics. The major characteristics include Henry's coefficient (H), bioaccumulation potential ( $K_{ow}$ ), heavy metals partitioning coefficients for sediments ( $K_o$ ) and suspended solids and particulates ( $K_o$ ), half lives in water and sediment ( $H_{sc}$  and  $H_{cop}$ ), and mass transfer coefficient (MT or  $U_0$ ). Table 3.1 summarises these physico-chemical characteristics of pollutants.

Mackay and co-workers (e.g., Mackay et al., 1992) have recommended a fugacity based approach for those organic compounds that can maintain vapour pressure in air. For ester and naphthalene a fugacity-based approach will be used in a water sediment interaction model. The base fluid (ester) present in SBF is considered to be one of the pollutants in a drilling wastestream. Ester is used here as the base fluid, as it is the most popular base fluid among the synthetic based fluid family. Esters are obtained by crushing palm oil kernels. The oil is split into fatty acids and glycerol, which after fractionation is mixed with 2-ethyl hexanol in the presence of a catalyst to give a vegetable ester. Generally 8 to 12 carbon atoms are present in the ester molecules. A general formulation of an ester molecule is given in Figure 3.2 (Hall, 2000). The molecular weight (MW) for ester is 200, based on the molecular formula given in Figure 3.2.



Figure 3.2. Synthesis of esters from fatty acids (palm oil) and alcohol

Henry's coefficient (H) is the most important parameter for organic pollutants in the fugacity based models. It is used to estimate the fugacity capacity in water ( $Z_2$ ). The H provides a relationship for pollutant concentration in air and water. The H is the ratio of vapour pressure (P) and pollutant solubility in water (S). The experimental H value for ester is not reported in the literature. In the absence of experimental values, the QSARs (Quantitative Structure Activity Relationship) are used to determine an H value (Lyman et al., 1990). The quantitative structure activity relationships are based on laboratory data that correlate different physico-chemical characteristics of the pollutant. The QSAR for solubility and values of vapour pressure are used to estimate the H for ester (Lyman et al.,

1990). The estimated value of H is in the range of 0.10 to 0.15 m<sup>3</sup>-Pa/mol (Table 3.1). The H values for naphthalene are extensively reported in the literature. The reported experimental values for H are in the range from 42 to 56 m<sup>3</sup>-Pa/mol (Howard, 1989). Diamond et al., 1994). The H is not relevant parameter for heavy metals and an aquivalence based approach is used.

The mass transfer through diffusion is estimated with the help of a diffusion coefficient  $(D_{gw})$ . It is also calculated using QSAR because experimental values are not reported. The molecular formula of ester is used to estimate the diffusion coefficient  $(0.19 \times 10^{-9} \text{ m}^2/\text{hr})$  (Lyman et al., 1990). The mass transfer coefficient (*MT or U<sub>t</sub>*) can be calculated by dividing  $D_{gw}$  with the path length or sediment mixing depth (*A*). Seaconsult (2000) used an 8 cm sediment mixing depth to determine oil concentration on the seabed. Brandsma (1996) proposed a 4 to 9 cm thick sediment layer. For this range of thickness, the estimated *MT* for ester may vary from (2 to 5) × 10<sup>-5</sup> m/hr. For naphthalene and heavy metals. *MT* values are reported in the range of (1 to 4) × 10<sup>-4</sup> m/hr (Table 3.1).

The bioaccumulation potential of an organic compound is determined by the partitioning coefficients for organic carbon ( $K_{ac}$ ). It is used to estimate fugacity capacity in sediment ( $Z_d$ ) and suspended solids ( $Z_3$ ). The log of the octanol-water partitioning coefficient ( $K_{ow}$ ) is reported to be 1.69 (U.S. EPA, 1999b; Hall, 2000). The  $K_{ac}$  values are related to  $K_{ow}$  by a QSAR as given in Table 3.1 (Lyman et al., 1990). The log of octanol-water partitioning coefficient ( $K_{ow}$ ) for naphthalene is reported to be 3.30 (U.S. EPA, 1999a) but Howard (1989) has also reported experimental values of  $K_{ac}$  in the range of 400 to 1000.

| Pollutant   | Approach    | MV/MIV | Henry's Coefficient (11)<br>m <sup>1</sup> pa/mol   | Kim of Ka& Ka<br>LAR  | H100<br>days             | 1110<br>duys                        | MT or Us<br>m/hr  |
|---|-------------|--------|---|---|--------------------------|-------------------------------------|---|
| Esters<br>C <sub>11</sub> H <sub>1</sub> O <sub>1</sub> | Fugucity    | ,200   | "Lage 1/5) = 1 013×Lage(K) - 0 52<br>5 × 64 mol/m"<br>1' = 8 - 9 Pa<br>11 = 0.1 - 0.15                    | 2 "Lagkins = 1.69<br>"Lagkins = 0.544 × Lag(kins) + 1.377<br>K = urgane carbon partitioning coefficient | 16 10 22                 | 12 to 130                           | $^{1}D_{BW} = 0.191 \times 10^{3} m^{2} fm$<br>$^{1}A = 4 \cdot 9 cm$<br>$(I_{8} = D_{BW}/A$<br>$II_{8} = (2 \cdot 5) \times 10^{3} m/hr$ |
| Naphthalcne   |             | 128.2  | 1 2tr<br>95:6t.   | $0001 \text{ of } 0005, \text{ pure } \text{ w/y} \times 16.0 = \text{ w/y},$                           | 852 01 52.               | 02 01 5 0.                          |   |
| Lead<br>(Pb)  |             | 207.2  | $e^{h_{1}K_{a}} = 6.68 \times 10^{5}$ (SSVwater)<br>$e^{h_{1}K_{a}} = 3.33 \times 10^{5}$ (sediment/water | , "(LugK, range = 3.4 · 6.5)<br>() : "(LugK, range = 2.0 · 7.0)   | $_{ee}01 \times l_{q,s}$ | $_{\eta_{1}}01 \times 1_{\eta_{1}}$ |   |
| Mercury<br>(Hg)   |             | 272    | $K_{a} = 0.5 \cdot 2 \times 10^{4}$<br>$K_{a} = 0.5 \cdot 2 \times 10^{4}$                                | ; "HargA, range = 4 2 · 6 9)<br>; "HargA, range = 3 8 · 6 0)  | '25 - 2000               | '64 years                           |   |
| Arsenic<br>(As)   |             | 6.44   | "K <sub>4</sub> = 1.0 × 10"   | : "U.A.g.K., range = 2 0 · 6 0)<br>. "(Lag.K., range = 1 6 · 4 3)                                       | ÷                        | ×                                   | $_{1}01 \times 0.1 = {}^{9}/1$  |
| Copper<br>(Cu)  | Aquivalence | 61.6   | <sup>4</sup> K <sub>a</sub> = 2.0 × 10 <sup>4</sup>   | "'ILogK, range = 3.1 - 6.1)<br>"ILogK, range = 0.7 - 6.2)   | ÷                        | ×                                   | $1/a = 2.0 \times 10^{4}$   |
| Chromium<br>(Cr)  |             | 52.0   |   | "llagK, tange = 3.9 · 6.0)<br>"flagK, range = 1.9 · 5.9)  |                          |                                     | $t^{h} U_{s} = 4.0 \times 10^{4}$   |
| Zinc<br>(Zn)  |             | 654    | *K <sub>*</sub> = 1.0 × 10 <sup>5</sup><br>*K <sub>*</sub> = 5.2 × 10 <sup>5</sup>                        | "ItagK, range = 3.5 · 6.9)<br>"UtagK, range = 1.5 · 6.2)  | a,01 × 1,                | n01 × 1,                            |   |
| Nickel<br>(Ni)  |             | 58.7   |   | "HargK, range = 3.5 · 5.7)<br>"HargK, range = 0.3 · 4.0)  |                          |                                     |   |
| Cadmium<br>(Cd)   |             | 112.4  | ${}^{0}K_{s} = 3.2 \times 10^{5}$   | . "HangK <sub>a</sub> range = 2 8 · 6 3)<br>"(LugK <sub>a</sub> range = 0 5 · 7 3)                      | 4                        | ÷                                   |   |
|   |             |        | a 11.011.2000)  | g Lang et al (1991).<br>A Michael Annovation (1986).  |                          |                                     |   |
|   |             |        | · Huward (1969)   | I Ihanswad (1979)   |                          |                                     |   |
|   |             |        | d Diamond et al (1994)  | 1 Standary (1991)   |                          |                                     |   |
|   |             |        | COMPANY AND A STATE   | A Brandonia (1996)  |                          |                                     |   |
|   |             |        | I Markay et al (1992)   | 1 Searconstit (2000)  |                          |                                     |   |
|   |             |        |   | 11 11 S 11 V (1658)   |                          |                                     |   |

Table 3.1. Physico-chemical properties of selected pollutants in drilling waste stream

3

The reaction rate coefficient (k) determines the biodegradation potential of organic pollutants. It can be determined by half-lives of organic pollutants using first order kinetics. The organic pollutants degrade at different rates in water and sediments. Based on aqueous and sediment phase anaerobic biodegradation tests, the range of ester halflives in water ( $H_{ic2}$ ) and sediments ( $H_{ic20}$ ) are reported to be 16 to 22 days and 12 to 130 days, respectively (Table 3.1). The details of experimental results are summarised by the U.S. EPA (1999b). The half-lives of naphthalene in sediment ( $H_{ic20}$ ) and water ( $H_{ic2}$ ) under anaerobic conditions may range from 25 to 258 days and 0.5 to 20 days, respectively (Howard, 1989). Heavy metals are not biodegradable and the half-lives of 1 × 10<sup>26</sup> years or more are generally assumed in the aquivalence-based fate modeling (e.g., Ling et al., 1993).

In the aquivalence based approach, the aquivalence capacity (Z) is determined using the partitioning coefficient from water to  $SS(K_a)$  and water to sediment ( $K_a$ ). The values of partitioning coefficients may vary with a change in hardness and pH. Some equilibrium speciation computer models including. *MINEQL* + and *MINTEQA2* are also used to estimate the equilibrium partitioning of the metals (U.S. EPA, 1999c). Diamond et al. (1994), Diamond (1999) and Ling et al. (1993) have reported the partitioning coefficient values for various heavy metals. The U.S. EPA (1999c) has summarised the data for a range of partitioning values in various media including soil/water. sediment/water and SS/water. The range of  $K_a$  and  $K_a$  values are summarised in Table 3.1.

#### 3.1.2. MEDIA CHARACTERISTICS

The next step in the fate modeling is to define the physico-chemical nature of the environment in which pollutants are discharged. The marine environment consists of two basic compartments: sediment and water. The sediment layer consists of water and solid particles and the water column contains SS and biota. The media (water and sediments) characteristic parameters include density ( $\rho$ ), volume fraction (v), organic content ( $\phi$ ) and various coefficients used in determining the deposition of SS. resuspension and burial rates of sediments. Table 3.2 summarises the media characteristic values.

The layer of sediments just above the seabed is very loose and contains high water content. The volume fraction (v<sub>4</sub>) of the pore water in sediments may range as high as 0.7 to 0.92 in the lake system (Mackay and co-workers). Seaconsult (2000) and Brandsma (1996) used a volume fraction of 0.4 in drilling waste fate modeling, which represents medium to coarse mixed sand porosity. Sleath (1984) has suggested a range of 0.35 to 0.5 (mixed sand to silt) for the seabed. Similarly, the SS volume fraction (v<sub>5</sub>) is reported in the range of 0.2 to 50 ppmv (parts per million by volume) (Mackay and co-workers). The Terra Nova (1996) Environmental Impact Assessment (EIA) reported v<sub>5</sub> in the range of 10 to 2770 ppmv on the Grand Banks.

The densities of SS ( $\rho_2$ ) and dry sediments ( $\rho_2$ ) are reported in the range of 1500 to 2400 kg/m<sup>3</sup> and 2000 to 2640 kg/m<sup>3</sup>, respectively (Mackay and co-workers). Seaconsult (2000) has reported insitu bulk density ( $\rho_2$ ) of 1850 kg/m<sup>3</sup> and specific weight of mineral sediment ( $\rho_2$ ) of 2596 kg/m<sup>3</sup> for the Georges Bank. Brandsma (1996) used a  $\rho_4$  of 2430

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kg/m<sup>3</sup> in the OOC model for the prediction of oil content in sediments. The organic fractions in SS ( $\phi_1$ ) and sediments ( $\phi_2$ ) determine the fugacity capacities (2). Kranck et al. (1992 and 1996) have suggested that individual clay and silt particle (<100 µm) may flocculate in the marine system due to the biologically facilitated environment. The organic fraction in SS is reported in the range of 0.12 to 0.32. The organic fraction in sediments ( $\phi_2$ ) is reported in the range of 0.035 to 0.045 (Mackay and co-workers).

Physical processes including sediment burial and resuspension and *SS* deposition rates establish the pollutant concentration in water and sediments. The advection/burial rate  $(u_{\theta})$  for sediments is reported in the range of 0.19 to 4.36 gm/m<sup>2</sup>/day (Diamond et al., 1994; Diamond, 1999; and Ling et al., 1993). For the density of dry sediment of 2500 kg/m<sup>3</sup>, the burial rates are estimated to be in the range of 0.003 to 0.064 cm/yr. Mackay et al. (1992) have also reported burial rates as high as 1.75 cm/yr in a lake system.

The resuspension rate  $(U_{10})$  reported in the lake system varies from 0.5 to 13.5 gm/m<sup>2</sup>/day. The  $U_{10}$  becomes equal to 0.007 to 0.2 cm/yr for a sediment density of 2500 kg/m<sup>3</sup> (Table 3.2). Sleath (1984) has summarised more than 25 different relationships for initial movement of sediment bedloads. Huang (1992) has also performed laboratory experiments for resuspension of drilling waste and developed an equation for mass flux as a function of advection velocity (u), and densities of water ( $\rho_2$ ) and sediment ( $\rho_4$ ). For  $\rho_2 = 1000 \text{ kg/m}^3$ ,  $\rho_4 = 2500 \text{ kg/m}^3$ , resuspension rate ( $U_{10}$ ) may vary from 0.0085 to 0.021 cm/yr The details of these models are given in Table 3.2.

| Deposition<br>(1/s)                  | 5.0 a (1) about (1) 444 (1) (4)<br>(2) (2) (2) (2) (2) (2) (2)<br>(2) (2) (2) (2) (2) (2) (2) (2) (2) (2)   | and the second secon   | 1944)<br>3.<br>2.<br>1.<br>1.<br>1.<br>1.   |
|--------------------------------------|---|---|---|
| Resuspension<br>(U <sub>10</sub> )   | 2 0 10 1018 (2017)<br>1.1.3 5 (2017/day)<br>1.1.3 5 (2017/day)<br>1.0.5 (2017/day)<br>1.0.12 (2017/day)<br>1.0.10 2 (2017)<br>0.000 10.0 2 (2017)<br>1.0.10 | Assumption of the set | 1 Icra Nura 1<br>1 Ibang 1995<br>2 Second 1<br>2 Second 1 908<br>1 Second 1 908<br>1 Second 1 908 |
| Depth<br>(m)                         | 95m<br>Vitand<br>Banks  | 01 0'<br>20 0'<br>200 0 0'<br>20 04 0 09<br>20 04   | 1994)<br>1989)<br>1989)   |
| Advection<br>rate/Burial rate<br>(u) | <sup>11</sup> a = 1 8 cmb.  | 0.19.0.7 (g/m/day)<br>0.1 (g/m/day)<br>1.4.3.6 (g/m/day)<br>1.4.3.6 (g/m /day)<br>1.4.3.6 (g/m /day)<br>1.4.3.6 (g/m /day)<br>1.1 % (g/m)<br>1.1 % (g/m)  | c Damond et al.<br>Damond (1994)<br>g Mackey and Di<br>g Wet i terry at                           |
| Organic<br>fraction<br>(Ø)           | "0 24-0 32<br>"0 12<br>"0 12<br>"0 12<br>"0 12<br>"0 12   | 004<br>00159<br>00159   |   |
| Density<br>(kg/m')<br>(p)            | 0001.<br>1001<br>0007.<br>0007.<br>0007.<br>0007.<br>0007.<br>10001.  | 001<br>001<br>0002<br>0002<br>0002<br>0002<br>0002<br>0002<br>0   |   |
| Volume<br>fraction<br>( v)           | "5 × 10 <sup>4</sup><br>"5 × 10 <sup>4</sup><br>"5 × 10 <sup>5</sup><br>0 5.2 × 10 <sup>3</sup><br>2 08 × 10 <sup>7</sup><br>"4.63 × 10 <sup>7</sup><br>"9.10 × 10 <sup>4</sup>   | 07<br>0015<br>1015<br>104<br>103405   | Lay et al (1992)<br>EFA (1993)<br>absua (1996)<br>q et al (2000)                                  |
| Components                           | (S (S)<br>Mater (2)<br>Tish (6)   | hure water (2)<br>fediment<br>olids (4)   | a Mar<br>b 11 S<br>fitan<br>d Sada  |
| Primary (<br>Medium                  | Column<br>(2)<br>F  | Sediment F  |   |

Table 3.2. Physical properties of primary compartments (media) and their components

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Mackay and co-workers have also reported the deposition rates in the range of 1.5 to 15 gm/m<sup>2</sup>/day for a lake system. The estimated thickness of the material deposited may vary from 0.03 to 0.31 cm/yr for a SS density of 1800 kg/m<sup>2</sup> (Table 3.2). Mackay et al. (1992) have also reported a high deposition rate of 0.44 cm/yr. Seaconsult (2000) used a particle tracking technique to determine the fate of synthetic based fluids. They have recommended that particles of size ( $\delta$ ) smaller than 100 µm, will form flocs due to the presence of phytoplankton and other organic substances and will settle with a constant fall velocity of 100 m/day (or 0.00761 cm·s<sup>-1</sup>). Huang (1992) has performed experiments to determine the fall velocity of discrete fall velocity of suspended particles of diameter ( $\delta$ ) less than 100 µm. He recommended that the particles of size bigger than 100 µm will settle with a fall velocity of 4.2( $\delta$ )<sup>43</sup>. Sleath (1984) has proposed the multiplication coefficient (A) in the range of 2.8 to 6.5 instead of 4.2 depending on the shape of a settling particle. The details of various deposition models are given in Table 3.2.

For determining the advection rate in the water column and the resuspension rate on the seabed, current velocity (u) is used. Brandsma (1996) used a constant value, which is the root mean square (RMS) of velocities over the cross-sectional area. He employed this assumption for the OOC model for predicting the fate of oil-based drill cuttings in the North Sea. The current speed is highly variable in intensity and direction. It is a sitespecific characteristic and the focus of this research is the East Coast of Canada. Terra Nova (1996) has reported a mean current speed of 3.6 cm·s<sup>-1</sup> at the surface and 1.8 cm·s<sup>-1</sup> at mid depth on the East Coast of Canada (Newfoundiand). Sadiq et al. (2001a-c) have also used a mean current speed (u) of 1.8 cm·s<sup>-1</sup> for hypothetical case studies in the Newfoundland offshore.

## 3.2. MODEL CHARACTERISATION

After defining the pollutants and media characteristics, the water sediment interaction model formulation can be applied to determine the fate of pollutants in the water column and sediment pore water. To use a parametric approach in probabilistic analysis, the input variables are expressed in terms of statistical distributions. The statistical distributions are defined subjectively based on available data as discussed in the previous section. Monte Carlo (MC) simulation using Latin Hypercube Sampling (LHS) is proposed for performing uncertainty analysis. To perform the MC simulations in the fate modeling process some assumptions are made. A bnef description of the selection of distributions and MC simulation methods is also presented in this section.

## 3.2.1 MODEL ASSUMPTIONS

A marine system is a complex environment in which various processes are occurring simultaneously. A water sediment interaction model considers some of the important physico-chemical processes to determine the fate of pollutants in the water column and sediments. Therefore, the following assumptions are made to simplify the complex processes occurring in the marine environment:

 The water sediment interaction model is a steady state model and therefore continuous pollutant loading rates are assumed (Meinhold, 1998).

- In the fate modeling, the impact area (A<sub>w</sub>) is predefined to estimate the pollutant concentrations in the water column and pore water. The impact area (A<sub>w</sub>) is defined by a radius (R), which is the radius of a circle whose area equals to the impact area Brandsma (1996).
- The water column depth (d<sub>w</sub>) is fixed to determine the mixing control volume for pollutants (Figure 3.3). In this case the value is taken at 95 m (Terra Nova, 1996).



Figure 3.3. Control volume and uniform velocity profile (not to the scale)

4. Mid depth velocity (u) is assumed to be a representative velocity, which has a uniform profile over the cross section (boundary layer effects are not considered). The volume flow rate (G) can be determined by multiplying cross-sectional area with the advective velocity (u). The projected cross-sectional area of the control volume is the product of the depth of water column (d<sub>w</sub>) and the diameter (2R) of the impacted area.

- The pollutants entering the control volume are assumed to be completely mixed under steady state conditions and physico-chemical processes are taking place uniformly over the impact area (A<sub>w</sub>).
- 6. Depending on the general characteristics of the drilling waste, part of the pollutant loads is assumed to go directly into the sediments (*E<sub>i</sub>*) and the remainder disperses in the water column (*E<sub>2</sub>*) and settles according to the particles' settling velocities.
- 7. Floc formation at the concentrations of fine cuttings is quick and assumed to be an instantaneous process with floc diameters of 100 µm or more while settling. The larger cuttings of = 1 mm may take 8 to 18 minutes to settle for a depth of 95 m water column (Sleath, 1984). Therefore drill cuttings larger than 1 mm are assumed to settle immediately, which account for the pollutant loading going directly into the sediments (*E<sub>A</sub>*). In this analysis a conservative approach is adopted: 20% of the load is assumed to be attached to bigger particles that go directly to sediments, while the remainder settles at their corresponding settling velocities.

The following section will discuss various types of uncertainties involved in fate modeling and MC simulation methods used in estimating uncertainties. A detailed description of the selection of statistical distributions used for input variables to the water sediment interaction model is also presented.

#### 3.2.2. UNCERTAINTIES - ESTIMATION METHODS

Uncertainties are inherent in modeling processes, and include natural randomness, uncertainty associated with the model parameters, and the model's structure. Natural randomness/heterogeneity includes spatial and temporal fluctuations inherent in natural processes. The structure of mathematical models that are employed to represent scenarios and processes of interest is often a key source of uncertainty, due to the fact that models are only a simplified representation of a real-world system (Cullen and Frey. 1999). Model uncertainties are not significant in a well-tested physical model like fugacity and aquivalence based models (CCME, 1997).

Quantities that are inherently variable over time, space or some population of individuals are referred to as variability or Type A uncertainty. This variability cannot be reduced, as it is the inherent quality of the system. Another type of uncertainty is called lack of knowledge uncertainty, subjective or Type B uncertainty. The variability is the property of the system being studied and uncertainty is the property of the analyst or instrument (Cullen and Frey, 1999).

To estimate the uncertainty in the model's parameters and structure, various simulation techniques are used (Ang and Tang, 1984). The methods for simulating the propagation of probability distribution through inputs in a model are based upon random sampling. The most well known method is MC simulations using Latin Hypercube Sampling (LHS). Some transformation techniques, which are not based on MC simulations, are also used in propagating distribution. The taxonomy of commonly used methods for propagating moments and distributions through models are summarized in Table 3.3.

| Group                                 | Methods   |  |  |  |  |
|---------------------------------------|---|--|--|--|--|
| Analytical Solutions for Moments      | <ul> <li>Central limit theorems</li> <li>Properties of mean and variance</li> </ul>   |  |  |  |  |
| Analytical Solutions of Distributions | Transformation of variables   |  |  |  |  |
| Approximation Methods for Moments     | <ul><li>First-order methods</li><li>Taylor series expansions</li></ul>  |  |  |  |  |
| Numerical Methods                     | Simple MC simulation     Latin Hypercube sampling     Importance sampling     Fourier amplitude sensitivity test     Others |  |  |  |  |

Table 3.3. Common probabilistic methods for propagating moments and distributions (Cullen and Frey, 1999)

Monte Carlo refers to the traditional method of sampling random variables in simulation modeling. Samples are chosen completely randomly across the range of the distribution, thus necessitating large numbers of samples for convergence for highly skewed or long tailed distributions (Palisade, 1997). Figure 3.4 describes the general procedure of MC simulation technique in which samples from two independent input variable distributions  $x_1$  and  $x_2$  are selected randomly. The output distribution z is calculated using a relationship of input variables e.g., in Figure 3.4 output (z) is a product of variables  $x_1$  and  $x_2$ .



Figure 3.4. A random sampling process of simple or ordinary MC simulations

Many variations of the MC simulation technique are aimed at reducing the computational effort and improving statistical techniques. A review of the literature on the use of MC simulation in environmental engineering shows that the LHS method is the only variance reduction technique that is widely used (Cullen and Frey, 1999). The use of MC methods in the uncertainty analysis especially in the area of risk assessment and fate modeling has received growing attention in recent times. The MC simulation method has been proposed as the method of choice due to the robustness of the method and its ability to handle non-linearity of the problem (U.S. EPA, 1996a). As a result, MC simulation is the most widely used methodology to account for uncertainties in contaminant fate modeling.

The input parameters are not necessarily independent of each other. The correlation coefficients can also be defined in MC simulations to account for the inter dependence of input parameters. The estimation of correlation coefficients is data intensive but subjective judgments may also be used. This is one of the major drawbacks of using MC simulations. In the absence of enough evidence, independent input parameters are assumed. Trial and error methods are also used in which different values of correlation coefficients are assumed to perform MC simulations (Hammonds et al., 1994).

The U.S. EPA (1996a) has recommended LHS over simpler random sampling for complex models due to better performance. The LHS is a relatively new sampling technique used in simulation modeling (Palisade, 1997). It is designed to accurately recreate the input distribution through sampling in fewer iterations when compared with the simple MC simulations. The key to LHS is stratification of the input probability distributions. Stratification divides the cumulative curve into equal intervals on the cumulative probability scale (0 to 1). A sampling is then randomly taken from each interval or stratum of the input distribution. Sampling is then forced to take values from each interval or recreate the input probability distribution. It offers great benefits in terms of increased sampling efficiency and faster run times (Morgan and Henrion, 1990).

#### 3.2.3 SELECTION OF STATISTICAL DISTRIBUTION

The distributions may result directly from data obtained from proper experimental design, but in the absence of data usually subjective judgement must be used to show the degree of belief that the unknown value of a parameter lies within a specified range (Hammonds et al., 1994). When data are limited but uncertainty is relatively low (less than a factor of 10), a range may be used to specify a uniform distribution. If there is knowledge about the most likely value (MLV) or mode, in addition to range, a triangular distribution may be assigned. When the uncertainty range exceeds a factor of 10, it is often prudent to assume a probability distribution of the logarithms of parameter values; therefore, when the range of uncertainty is very large, a log-uniform, log-triangular distribution may be more appropriate than the uniform or triangular distribution (Hammonds et al., 1994). The other important commonly used distributions are normal, lognormal, exponential and/or empirical distributions. These are usually dependent on the availability of relevant data. The most commonly used probability density functions (PDF) of some statistical distributions are shown in Figure 3.5.



Figure 3.5. PDFs of some commonly used statistical distributions

The statistical distributions like uniform (U), triangular (T), exponential (E), normal (N), and lognormal (LN) distributions are defined for various input variables in this research. The truncated forms of normal and lognormal distributions are also defined where minimum and maximum bounds are known. Table 3.4 provides a summary of proposed statistical distributions for physical properties of media and their characteristic parameters.

| Inputs   | Statistical distribution | Min.  | μ or MLV | σ   | Max.  | Comments  |  |
|--|--------------------------|-------|----------|-----|-------|---|--|
| $\Delta or d$ , (m)<br>sediment depth                          | Triangular               | 0.04  | 0.08     |     | 0.09  | Sleath (1984);  |  |
| v <sub>2</sub><br>pore water fraction                          | Triangular               | 0.35  | 0.4      |     | 0.5   | (2000)  |  |
| SS volume fraction   | Triangular               | 0.2   | 20       |     | 50    | Various sources<br>(see in the text)                              |  |
| Ø,<br>Sediments organic<br>fraction                            | Uniform                  | 0.035 |          |     | 0.045 | Timothy and<br>David (2000)                                       |  |
| Ø:<br>SS organic fraction                                      | Uniform                  | 0.12  |          |     | 0.32  |   |  |
| ρ <sub>4</sub> (kg/m <sup>3</sup> )<br>density of<br>sediments | Triangular               | 2000  | 2500     |     | 2640  | Various sources<br>(see in the text)<br>Sadiq et al.<br>(2000a-d) |  |
| $\rho_s (kg/m^3)$<br>density of SS                             | Triangular               | 1500  | 1800     |     | 2400  |   |  |
| u (cm/sec)<br>advective velocity                               | Exponential              |       | 1.8      |     |       |   |  |
| ug (g/m²/day)<br>burial rate                                   | Uniform                  | 0.19  |          |     | 0.70  | Diamond<br>(1999)   |  |
| ç<br>coefficient of<br>resuspension                            | Uniform                  | 0.002 |          |     | 0.005 | Huang (1992)  |  |
| δ(μm)<br>SS diameter   | Truncated<br>Lognormal   | 100   | 200      | 150 | 1000  | Sleath (1984);  |  |
| A<br>coefficient for<br>deposition                             | Triangular               | 2.8   | 4.2      |     | 6.5   | Seaconsult<br>(2000)  |  |

Table 3.4. Proposed statistical distribution for multimedia properties

ppmv: parts per million by volume

#### Statistical Distributions for Media Properties

Brandsma (1996) has suggested that the mixing depth of sediment (.d) on the seabed may vary from 4 to 9 cm. but Seaconsult (2000) has recommended a depth of 8 cm for determining the base oil concentration. A triangular distribution (T) is assumed with a minimum value of 4. most likely value (MLV) of 8 and maximum value of 9 cm. It can be designated by T (4.0, 8.0, 9.0).

The volume fraction of SS (v<sub>2</sub>) is mostly reported in the range of 0.2 to 50 ppmv (Mackay and co-workers) although a value as high as 2770 ppmv is also reported (Terra Nova, 1996). This high value is neglected due to lack of evidence. A triangular distribution is assumed for v<sub>e</sub> which has a minimum value of 0.2, mode (MLV) of 20 and a maximum value of 50 ppmv. The pore volume fraction (v<sub>2</sub>), or porosity ranges from 0.35 to 0.5 in seabed sediments (Sleath, 1984). Seaconsult (2000) assumed a constant value of 0.4. A triangular distribution is assumed with minimum and maximum values of 0.35 to 0.5. respectively. An MLV (or mode) of 0.4 is recommended. The proposed triangular distribution can be designated by T (0.35, 0.4, 0.50).

The organic fractions in SS ( $\phi_2$ ) and sediments ( $\phi_4$ ) are assumed to be uniformly distributed (U) using minimum and maximum values available in the literature. Timothy and David (2000) have recommended a uniform distribution for defining the organic fraction in the food web modeling. The  $\phi_1$  is defined by U (0.12, 0.32), where 0.12 and 0.32 are minimum and maximum values, respectively. Similarly, the organic fraction in sediments ( $\phi_4$ ) is defined by U (0.035, 0.045). The densities of SS  $(\rho_2)$  and sediments  $(\rho_4)$  are assumed to be triangularly distributed. The  $\rho_3$  is defined by T (1500, 1800, 2400). The density of sediment  $(\rho_4)$  is defined by T (2000, 2500, 2640). The minimum and maximum values are defined based on recommendations by Seaconsult (2000) and U.S. EPA (1999b). The most likely value (MLV) is calculated by taking the average of two intermediate values (Table 3.2).

The advective velocity ( $\omega$ ) is defined by exponential distribution (E) with a mean current speed of 1.8 cm/sec (Sadiq et al., 2001a-d). The burial rate ( $\omega_{\mu}$ ) of sediment varies from 0.003 cm/yr (0.19 g/m<sup>2</sup>/day) to 0.064 cm/yr (4.36 g/m<sup>2</sup>/day). A value as high as 1.75 cm/yr is also reported in the literature, but ignored due to lack of evidence. A uniform distribution with a range of 0.19 to 0.7 g/m<sup>2</sup>/day is used (Diamond, 1999) for a conservative assessment of pollutant concentration in sediments (Table 3.4).

To estimate the resuspension rate  $(U_{10})$ , the resuspension coefficient ( $\xi$ ) is defined by uniform distribution U (0.002, 0.005), as proposed by Huang (1992). To determine the deposition rates of drill cuttings with SS flocs. a methodology proposed by Seaconsult (2000) is adopted, which states that the fine particles make flocs of 100 µm or more before settling. The multiplication coefficient (A) of the settling equation (Table 3.2) is also defined by a triangular distribution, T (2.8, 4.2,6.5) (Sleath, 1984).

The particle size distribution of drill cuttings shows that approximately 45% are fines (< 250 µm), 5% medium sand (250 - 500 µm), 20% coarse sand (500 µm - 1 mm) and 30% are of bigger size pebbles (Seaconsult, 2000; Sleath, 1984). Particle size ( $\delta$ ) is generally defined by lognormal distribution in the soil and fluid mechanics literature. A lognormal distribution (LN) is assumed with a mean and standard deviation of 200  $\mu$ m and 150  $\mu$ m, respectively. The lognormal distribution is truncated at the upper and lower bounds of 1000  $\mu$ m and 100  $\mu$ m, respectively. The cuttings of larger than 1 mm are assumed to settle immediately, which accounts for an assumed 20% of total pollutant loadings going directly into the sediments (*E*<sub>0</sub>) (Sadig et al., 2001d).

## Statistical Distributions for Physico-chemical Characteristics of Pollutants

The statistical distributions for physico-chemical characteristics of pollutants are summarized in Table 3.5. In the case of organic pollutants, statistical distributions of Henry's coefficient (*H*) and half-lives in water (*H<sub>icc</sub>*) and sediments (*H<sub>icc</sub>*) are defined. The half-lives are derived based on anaerobic biodegradation rates in water and sediments. Henry's coefficient (*H*) for ester is calculated based on QSAR (Lyman et al., 1990). A uniform distribution is assumed, which is designated by U (0.1, 0.15). Henry's coefficient (*H*) for naphthalene ranges from 42 to 56 and a uniform distribution is assumed. The *K*<sub>oc</sub> for naphthalene is assumed to be uniformly distributed and designated by U (400, 1000). The half-lives of both organic pollutants are assumed to be normally distributed with lower and upper bounds truncated at reported minimum and maximum values, respectively. Ester mass transfer (*U*<sub>0</sub>) through diffusion is calculated by QSAR (Lyman et al., 1990). The diffusion coefficient (*D<sub>mr</sub>*) is divided by path length or mixing depth ( $\Delta$ ) to calculate *U<sub>b</sub>*. For all other pollutants under consideration, *U<sub>i</sub>* is assumed to be triangularly distributed and designated by T (1, 2, 4) × 10<sup>-4</sup>.

| Organic<br>pollutants | H<br>(m <sup>3</sup> ·pa/ mol) | K <sub>ow</sub> or K <sub>or</sub>             | MT (U <sub>a</sub> )<br>(m/hr) | Hu2<br>(days)           | H1/20<br>(days)             |
|-----------------------|--------------------------------|--|--------------------------------|-------------------------|-----------------------------|
| Esters                | U<br>(0.1, 0.15)               | $K_{m} = 49$<br>point estimate                 | D <sub>sw</sub> /A             | TN<br>(50, 20, 12, 130) | TN<br>(20, 2, 16, 22)       |
| Naph.                 | U<br>(42, 56)                  | For <i>K</i> <sub>a</sub> ,<br>U<br>(400, 600) |                                | TN<br>(10, 5, 0.5, 20)  | TN<br>(150, 50, 25, 258)    |
| Heavy<br>Metals       | LogKa<br>(L/kg)                | LogK <sub>x</sub><br>(L/kg)                    |                                | H1/2<br>(days)          | Η <sub>1/2σ</sub><br>(days) |
| Рb                    | TN<br>(4.6. 1.9. 2.0. 7.0)     | TN<br>(5.7. 0.4, 3.4, 6.5)                     |                                |                         |                             |
| Hg                    | TN<br>(4.9, 0.6, 3.8, 6.0)     | TN<br>( 5.3. 0.4, 4.2, 6.9)                    |                                | T<br>(25, 1000, 2000)   | 64 years<br>point estimate  |
| As                    | TN<br>(2.4, 0.7, 1.6, 4.3)     | TN<br>( 3.9, 0.5, 2.0, 6.0 )                   | $T^{3}$                        |                         |                             |
| Cu                    | TN<br>(3.5, 1.7, 0.7, 6.2)     | TN<br>(4.7, 0.4, 3.1, 6.1)                     |                                |                         |                             |
| Zn                    | TN<br>(4.1, 1.6, 1.5, 6.2 )    | TN<br>( 5.0, 0.5, 3.5, 6.9)                    |                                |                         |                             |
| Cr                    | TN<br>(4.9, 1.5, 1.9, 5.9 )    | TN<br>( 5.1, 0.4, 3.9, 6.0)                    |                                |                         |                             |
| Cd                    | TN<br>(3.3. 1.8. 0.5. 7.3 )    | TN<br>(4.9, 0.6, 2.8, 6.3)                     |                                |                         |                             |
| Ni                    | TN<br>(3.9, 1.8, 0.3, 4.0)     | TN<br>(4.4, 0.4, 3.5, 5.7)                     |                                |                         |                             |

Table 3.5. Proposed statistical distribution for physico-chemical properties of selected pollutants

'U.S. EPA (1999c) has defined the distribution and parameters

TN (mean, sidey ., man., max.)

Trmm. MLV. mux.)

For heavy metals half-lives are not important except for Hg, which exists in various forms. The half-life of Hg in water is designated by a triangular distribution T (25.1000,2000). In aquivalence based models, the most important parameters are  $K_*$  and  $K_*$  which define the metals partitioning with SS and sediments, respectively. The U.S. EPA (1999c) collected information from more than 250 sources to define the ranges and statistical distributions of heavy metal partitioning coefficients. The LogK, and LogK, are normally distributed and are truncated at minimum and maximum bounds. The details of the statistical distributions and their characteristic parameters are given in Table 3.5.

## 3.3 PROBABILISTIC ANALYSIS

After defining the statistical distributions for model inputs the immediate objective of this research was to perform MC simulations to determine the model outputs. The outputs of the model were pollutant concentrations in the water column ( $C_{*}$ ) and sediment pore water ( $C_{rei}$ ). The uncertainty/variability in the model inputs contributed to the uncertainty in model outputs.

The uncertainty in the model structure due to simplification of the processes was not addressed in this research (CCME, 1997). The fugacity/aquivalence model structure developed in Table 2.10 (Chapter 2) was converted into the spreadsheet format as shown in Figure 3.6. The @Risk (Palisade, 1997) software was used for MC simulations. The software is an add-on to Microsoft Excel. The input parameters were defined by probability density functions (PDF).

The MC simulations were performed for different predefined impact area  $(A_v)$  and operational pollutant loading rate  $(E_v)$  conditions. The concentrations of each pollutant in the water column and pore water were determined under a given discharge condition. The simulations were then repeated for another set of conditions defined by a different impact area and/or pollutant loading rate. The model outputs - water column  $(C_w)$  and sediment pore water  $(C_{rw})$  concentrations - were obtained in the form of an empirical distribution

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function (EDF). The EDFs of model outputs were fitted to various candidate distributions. The parameters of the best-fitted distribution were determined for each discharge condition. The predicted environmental concentration (*PEC*) was determined by defining a higher percentile level of a concentration which is generally 90<sup>th</sup> or 95<sup>th</sup> percentile value on fitted distributions (Lenwood et al., 1998). Further, multiple linear regression analysis was performed to develop empirical regression models for parameters of the fitted distribution (e.g., mean and standard deviation) for each pollutant separately as function of their pollutant loading rates (*E*<sub>i</sub>) and radius of impacted area (*R*).



Figure 3.6. The MC simulations for fate modeling in the spreadsheet format

## 3.3.1. OPERATIONAL DISCHARGE CONDITIONS

The range of pollutant loading rates can be defined based on their emissions in various jurisdictions, which include the Gulf of Mexico (GOM), California, Alaska and the East Coast of Canada. The base fluid (ester) constitutes approximately 47% of the SBF's weight. The average weight of cuttings produced from a model deep development well is approximately 350 metric tones (MT). For the case of 10% attached fluids. 45 MT (57 m<sup>3</sup>) of ester as a base fluid goes into the ocean system from a single well. The ester quantity is reduced to 16 MT for 4% attached base fluid (U.S. EPA, 1999b).

The loading rates were selected such that all practical discharge conditions would be covered, e.g., for 15% attached base fluid, the discharge rate of ester will be 108 kg/hr (in the case of a one month drilling time). The discharge rate decreases to 26.4 kg/hr for the 5% attached base fluid case (Sadiq et al., 2001b). The discharge rates of ester in the ocean environment were selected in the range of 5 to 150 kg/hr.

Similarly, heavy metals and naphthalene loads can be calculated based on the composition of stock barite used in the SBFs and formation oil contamination. respectively. The heavy metals As, Cd, Cu, Hg, Ni and Pb are found in low concentration in barite (< 35 mg/kg of barite). Sadiq et al. (2001c) calculated As and Cu pollutant loadings rates which were below 1.2 g/hr for 15% attached base fluid. The naphthalene is an indicator for the formation oil contamination and only traces are expected in synthetic base drilling waste. The loading rates for these pollutants were selected within the range of 0.025 to 3 g/hr to cover all practical ranges. The Cr and Zn are generally found in higher concentration in stock barite (> 200 mg/kg of barite). The loading rates for these pollutants were selected within the range of 0.10 to 30 g/hr. The mass balance analysis for calculating the specific pollutant loading rates for our discharge scenarios is provided in Chapter 7 of this thesis.

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The concentrations in the water column and pore water also depend upon the total impact area (A<sub>4</sub>) which is defined by an equivalent circular area of radius *R*. Information about impacts of SBFs from field investigations is limited in comparison to WBFs. The U.S. EPA (1999b) has summarized a few field studies of SBFs' impact. The studies related to SBFs in which ester was used as a base fluid. They are summarized in Table 3.6. For various locations in the North Sea the SBFs were detected only in the upper 10 cm of sediment to a distance of 200 m from the point of discharge.

Daan et al. (1996) have reported an increase in ester concentration with increase in distance. The measured concentration increased from 393 to 834 mg/kg at distances of 75 m to 125 m from the point of discharge. The authors have surmised that this happened due to resuspension, and contaminated heterogeneous sediment near the outlet was spread out and well mixed over a larger area. This study also noted that the effects at 500 m and 3000 m were recovered after 11 months. In another study by Schaanning (1995), the sediment ester concentration decreased logarithmically with distance from discharge point. Benthic organisms were severely affected within a 100 m radius.

The U. S. EPA (1999b) has concluded that there is a little evidence of biological impacts at over 500 m from the point of discharge. The impact on sediment is expected to be within 100 m to 200 m from the point of discharge. The ester based SBFs are readily biodegradable in comparison to other fluids. The U.S. EPA (1999b) also concluded that the recovery of impacted benthic life starts within a one-year time.

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| Study source and location       | <i>dw</i><br>(m) | Cuttings/<br>SBFs discharged | *R<br>(m) | <sup>b</sup> C<br>(mg/kg) | <sup>c</sup> Time<br>(months) |
|---------------------------------|------------------|------------------------------|-----------|---------------------------|-------------------------------|
|                                 |                  | 361 m <sup>3</sup> SBEs:     | 75        | 706                       | 1                             |
|                                 |                  |                              | 75        | 393                       | 4                             |
|                                 |                  |                              | 75        | 84                        | 11                            |
| Daan et al (1996)               |                  |                              | 125       | 300                       | 1                             |
| North Sea/ Dutch                | 30               | 180 Metric tones             | 125       | 834                       | 4                             |
| continental shelf               |                  | (MT) ester                   | 125       | 10                        | 11                            |
|                                 |                  |                              | 200       | 54                        | 1                             |
|                                 |                  |                              | 200       | 161                       | 4                             |
|                                 |                  |                              | 200       | 55                        | 11                            |
|                                 |                  |                              | 50        | 85.300                    | 1                             |
| Smith and May (1001)            |                  |                              | 100       | 46,400                    | 1                             |
| Schaanning (1995)               |                  | 749 MT cutting;              | 200       | 208                       | 1                             |
| North Sea/ Norwegian            | 67               | 96.5 MT ester                | 50        | 0.21                      |                               |
| continental shelf               |                  |                              | 100       | 0.22                      |                               |
|                                 |                  |                              | 200       | 1.34                      |                               |
|                                 |                  |                              | 250       | 93                        | 1                             |
| Vik et al. (1996)<br>Loke       |                  |                              | 500       | 5                         | 1                             |
|                                 | 76-81            | 180 MT SBFs                  | 250       | 10                        | 12                            |
|                                 |                  |                              | 500       | 5                         | 12                            |
|                                 |                  |                              | 250       | 50                        | 1                             |
| Vik et al. (1996)<br>Sleipner A | 76-81            | 399 MT SBFs                  | 500       | 20                        | 1                             |
|                                 |                  |                              | 250       | 25                        | 12                            |
|                                 |                  |                              | 500       | <5                        | 12                            |
| Vik et al. (1996)               |                  |                              | 250       | 400                       |                               |
| Sleipner Ø                      |                  | 380 MT SBFs                  | 500       | 30                        |                               |
| U.S. EPA (1999b)                | )                | 6263 bbl<br>(ester + LAO)    | 75        | 165.051                   | (prior to<br>1997)            |
| GOM continental slope           | 365              | 1486 bbl<br>additional fluid | 75        | 198.320                   | (prior to<br>1998)            |

Table 3.6. Seabed filed studies for Ester based drilling fluids

"distance from point of discharge." concentration of base fluid in dry sediments . "time of sampling after discharge

Based on the available information for each pollutant, the loading rates were defined at three levels corresponding to five different levels of impact radius (*R*). Therefore MC simulations were performed for 15 different discharge scenarios for each pollutant. For example for ester, the first simulation trial was performed at a loading rate of 150 kg/hr assuming that the area impacted was within a radius of 50 m. In the second trial, an impact radius of 100 m was defined. The impact radius is increased in steps to 5 km for the 5<sup>th</sup> trial at the same loading rate. Then the loading rate was changed from 150 to 75 kg/hr for the 6<sup>th</sup> trial. For this loading rate the same procedure was repeated. Similarly for other pollutants, three pollutant loading rates were defined for five impact radii values. Table 3.7 summarizes the loading rates of different pollutants ( $E_i$ ) and impact radius (R) for performing MC simulations on the fugacity/aquivalence based water sediment interaction model.

| Simulation | R    | Ester      | As, Cd, Cu, Ni, Hg, Pb<br>and naphthalene | Cr and Zn |  |
|------------|------|------------|---|-----------|--|
| Trials     | (km) | E, (kg/hr) | E, (mg/hr)                                |           |  |
| 1          | 0.05 | 150        | 3000                                      | 30000     |  |
| 2          | 0.10 | 150        | 3000                                      | 30000     |  |
| 3          | 0.50 | 150        | 3000                                      | 30000     |  |
| 4          | 1.00 | 150        | 3000                                      | 30000     |  |
| 5          | 5.00 | 150        | 3000                                      | 30000     |  |
| 6          | 0.05 | 75         | 1000                                      | 10000     |  |
| 7          | 0.10 | 75         | 1000                                      | 10000     |  |
| 8          | 0.50 | 75         | 1000                                      | 10000     |  |
| 9          | 1.00 | 75         | 1000                                      | 10000     |  |
| 10         | 5.00 | 75         | 1000                                      | 10000     |  |
| 11         | 0.05 | 5          | 25  | 100       |  |
| 12         | 0.10 | 5          | 25  | 100       |  |
| 13         | 0.50 | 5          | 25  | 100       |  |
| 14         | 1.00 | 5          | 25  | 100       |  |
| 15         | 5.00 | 5          | 25  | 100       |  |

Table 3.7. Various discharge scenarios and trials for MC simulations

## 3.3.2. MC SIMULATIONS

The MC simulations using the LHS method were performed for the fugacity/aquivalence models. For each simulation, 10.000 iterations were performed to estimate concentrations in multimedia environment. The inputs of the models were defined by the statistical distributions as described in section 3.2.3. The results of MC simulations for fugacity and aquivalence models are discussed separately in this section.

## Fugacity Models

The MC simulations for ester and naphthalene were performed using a fugacity model. Simulation results for ester are discussed in detail here. In the first trial for ester, the loading rate and impact radius were 150 kg/hr and 50 m, respectively. The outputs of interest in the model simulations were concentrations in multimedia. The model outputs water column concentration ( $C_w$ ) and pore water concentrations ( $C_{PW}$ ) - were obtained in the form of an empirical distribution function (EDF). For first trial, the distribution of Cw is shown in Figure 3.7. The ester water column concentration is provided in the form of a histogram. The 95th percentile of concentration is also shown with the help of a solid line. The ester water column concentration may vary from 0.2 to 4 µg/L for the first trial. The mean and median values of Cw are 0.94 and 0.84 µg/L. respectively. This shows a slight positive skew in the distribution. The standard deviation of water column concentration is 0.41 µg/L. The maximum value of water column concentration is approximately 20 times more than the minimum value. The 95th percentile of the Cw is 1.78 µg/L, which is less than half of the maximum value (4  $\mu$ g/L). It implies that only 5% of the values lie within the range of 1.78 to 4 µg/L.



Figure 3.7. Histogram of water column ester concentration (µg/L) for Trial 1

The second output of the simulations was the pore water concentration ( $C_{rw}$ ). The histogram of pore water concentration is shown in Figure 3.8. The  $C_{rw}$  varies from 6.62 to 23.9 g/L. The mean and median values are 11.8 and 11.3 g/L, respectively. The standard deviation of water column concentration is 2.52 g/L. The minimum value of pore water concentration is approximately 4 times less than its corresponding maximum value. The  $C_{rw}$  distribution is also positively skewed. The 95<sup>th</sup> of pore water concentration is 16.8 g/L.


Figure 3.8. Histogram of pore water ester concentration (µg/L) for Trial 1

Figure 3.9 is a plot showing a comparison of water column and pore water concentration at various percentiles. The concentrations are compared at 5<sup>th</sup>, mean, 50<sup>th</sup> (median), 95<sup>th</sup> and their maximum values. The pore water concentration is higher than the water column concentration by various order of magnitudes. The 95<sup>th</sup> percentile of  $C_{rw}$  is more than  $C_w$ by approximately 7 orders of magnitude. Higher uncertainty is observed in the  $C_w$ values, which may be due to the higher settling rate of drill cuttings and higher uncertainty in half-life of ester in water (12 to 130 days).



Figure 3.9. Comparison of water column and pore water ester concentrations in (Trial 1)

The higher pore water concentration implies higher fugacity ( $f_2$ ) in comparison to water column fugacity ( $f_2$ ). Base fluids are hydrophobic in nature and may try to attach themselves with sediments. Due to the density and particle sizes of drill cuttings, a major portion of the drilling waste settles to the seabed sediments. Under steady state conditions the contaminants start leaching into the pore water and maintain higher levels of concentration in comparison to water column concentration. The MC simulations are also performed for naphthalene in the fugacity model. The MC simulation results for all trials are summarized in section 3.4.2.

### Aquivalence Models

The aquivalence based model is used for predicting heavy metals (As, Cd. Cu, Cr, Pb, Hg, Ni, and Zn) concentration. The MC simulations were performed for each metal separately. The MC simulation results from the first trial for Cd are presented and discussed in detail. The summary of simulations is given for each pollutant in the section 3.4.2.

The first MC simulation trial for Cd was performed at a loading rate of 3g/hr and an impact radius of 50 m. The water column concentration of Cd varies from 4.45 × 10<sup>-4</sup> to 7.61 × 10<sup>-3</sup> µg/L. The mean and median (50<sup>th</sup> percentile)  $C_w$  are 6.77 × 10<sup>-4</sup> and 1.16 × 10<sup>-5</sup> µg/L. respectively. This shows a highly positive skew in the distribution. The standard deviation of the water column concentration is 5.01 × 10<sup>-5</sup> µg/L. The 95<sup>th</sup> percentile of the  $C_w$  is 3.32 × 10<sup>-4</sup> µg/L, which is approximately 20 times less than the maximum value. The minimum value of the water column concentration is approximately 5 orders of magnitude less than the maximum value. The histogram of  $C_w$  is shown in Figure 3.10.

The results for the pore water concentration  $(C_{rw})$  are shown in Figure 3.11. The  $C_{rw}$  may vary from 1.77 × 10<sup>3</sup> to  $3.82 \times 10^3 \ \mu g/L$ . The mean and median values of  $C_{rw}$  are 96.3 × 10<sup>4</sup> and 7.62 × 10<sup>4</sup>  $\ \mu g/L$ , respectively. The  $C_{rw}$  is also a highly positive skewed distribution. The standard deviation of the water column concentration is 2.64 × 10<sup>6</sup>  $\ \mu g/L$ . The 95<sup>th</sup> percentile value of  $C_{rw}$  distribution is 5.51 × 10<sup>6</sup>  $\ \mu g/L$ . The minimum value of pore water concentration is approximately 4 orders of magnitude less than its maximum value.



Figure 3.10. Histogram of Cd water column concentration (µg/L) for Trial 1

Figure 3.12 is plotted to compare water column and pore water concentrations at various percentiles. The concentrations are compared at 5<sup>th</sup>, mean, 50<sup>th</sup> (median), 95<sup>th</sup> and maximum values. The pore water concentration is more than the water column concentration by orders of magnitude. The pore water concentration is more than the water column concentration by approximately 10 orders of magnitude at their 95<sup>th</sup> percentiles. The uncertainty in the concentration outputs can be estimated by comparing their minimum and maximum values, which differ by 4 to 5 orders of magnitude.



Figure 3.11. Histogram of Cd pore water concentration (µg/L) for Trial 1

The heavy metals maintain higher aquivalence (A) in sediments in comparison to the water column. Heavy metals are very persistent in nature and have practically infinite half-lives. Metal distribution in multimedia is a function of their partitioning abilities with sediments ( $K_{o}$ ) and particulates ( $K_{o}$ ). The aquivalence based steady state model determines long-term leaching of metals into pore water. The biodegradation and decay of heavy metals is not expected, although oxidation reaction, speciation and other chemical reactions may be possible.



Figure 3.12. Comparison of Cd concentrations in water column and pore water (Trial 1)

# 3.3.3. SENSITIVITY ANALYSIS (SA)

Sensitivity analysis (SA) is the process of estimating the degree to which output of a fate model changes as values of input parameters are changed. The American Standard for Testing and Materials (ASTM, 1998) has recognized the role of SA in the fate modeling as follows:

- 1. SA can identify the input parameters that have the most influence on model output;
- 2. SA can identify the processes which have greatest influence on model output; and

 SA can quantify the change in output caused by uncertainty and variability in the values of input parameters.

Sensitivity of the fate model is linked to input parameters through governing equations. The input parameters in probabilistic fate modeling may be represented by given ranges and/or distributions. Field investigations and monitoring can only provide the best sitespecific estimate. There are several reasons for identifying key model inputs, which contribute to uncertainty in model outputs. An identification of significant contributors to output variance gives the analyst an awareness of which input variable is controlling the output results. The basic exploration of the models, inputs and results, promotes improved understanding and interpretation of the analysis (Culen and Frey, 1999).

In a probabilistic analysis, the majority of the variance in the output is attributable to variability and or uncertainty in a small subset of the inputs. There are varieties of methods of identifying key input variables from model outputs. These methods include the scatter plot, partial and rank correlation coefficients, multivariate regression, contribution to variance and probabilistic sensitivity analysis. These methods are discussed in detail in the literature, e.g. Iman and Helton (1988), and Culen and Frey (1999) and they are briefly discussed below.

A correlation coefficient is a measure of the degree of association or covariance between two random variables. Correlation coefficients provide an estimate of the linear dependence of a model output on a particular model input. Correlation coefficients are

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sensitive to the strength of the linear relationship between model inputs and output and the range of variation of input. Correlation coefficients are estimated based on sample values of both the inputs and output and their relative means.

$$\rho_{r} = \frac{\sum_{k=l}^{m} (x_{k} - \bar{x}) (\bar{x}_{k} - \bar{z})}{\left( \sum_{k=l}^{m} (x_{k} - \bar{x}) (\bar{x}_{k} - \bar{z})^{2} \right)^{1/2}}$$
(3.1)

where *m* is the sample size or number of iterations in the simulations, *x* is an input and *z* is an output, *x*, and *z*, are sample values of *x* and *z*. The value of correlation coefficient p, may vary from -1 to +1 where:

 $\rho_r = +1$ , implies linear dependence, a positive slope (z increases as x increases)

 $\rho_r = 0$ , implies no linear dependence, a value of x provides no useful information about value of z, and

 $\rho_{c} = -1$ , implies linear dependence, a negative slope (z increases as x decreases)

For typical probabilistic simulations, correlation between an input and an output greater than [0.5] indicates substantial dependence of the variation in the output on variation of the input. Values as low as [0.2] also may be of interest, depending on the sample size. Larger correlation coefficients indicate less dispersion of sample values from an idealized linear relationship between input and output. Generally correlation coefficients are sensitive to both the strength of linear relationship between an input and output, and to the range of variation of the input (Culen and Frey, 1999).

If several values of the input or output are clustered together in a random pattern, and one point is located away from the cluster, an artificially high estimate of the correlation coefficient may result. Alternatively, in cases where underlying dependence is nonlinear (but monotonic), the sample correlation coefficient will underestimate the dependence. A way to avoid these problems is to calculate the correlation coefficient using the rank of both x and y. Each sample of a random variable has a rank. The rank is determined by ordering the sample values in ascending order. The lowest value has a rank one, the next lowest has a rank two and so on. The largest sample value has rank equal to the number of samples. The rank correlation coefficient is robust to different monotonic dependencies between input and output. A rank correlation coefficient correctly characterizes the strong nonlinear dependence by being equal to or almost equal to positive unity.

Another common method used for SA is to estimate the relative approximate contributions of each parameter to the variance of final outputs by squaring the rank correlation coefficients and normalizing them to 100% (Maxwell and Kastenberg, 1999). The parameters having the greatest effect are considered to be those for which additional data should reduce the amount of overall uncertainty in the results. Hammonds et al. (1994) and Maxwell and Kastenberg (1999) used this technique in human health risk assessment for identifying the key input variables. In the following section, SA using

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rank correlation coefficient and percent contribution of input variables are discussed for two pollutants namely, ester and Cd.

### Organic Pollutant (Ester)

The SA results for ester concentrations in water column and pore water are reported in Table 3.8. The rank correlation coefficients of all input variables are reported. The negative values of rank correlation coefficient  $(\rho_i)$  represents an inverse relationship with the output.

| Parameters | C.    | Crw   |
|------------|-------|-------|
| 05         | -0.62 | 0.00  |
| A          | -0.53 | 0.01  |
| δ          | -0.51 | 0.02  |
| ρ          | -0.18 | 0.00  |
| u          | 0.04  | -0.02 |
| Δ          | -0.01 | -0.78 |
| 5          | 0.00  | -0.03 |
| V4         | -0.01 | 0.12  |
| Vş         | 0.01  | 0.00  |
| H12        | -0.01 | -0.01 |
| H          | -0.01 | 0.00  |
| υ,         | 0.00  | 0.00  |
| H1/20      | 0.00  | 0.37  |
| Ø.         | 0.00  | -0.34 |
| ρι         | -0.01 | -0.25 |

Table 3.8. Sensitivity analysis (SA) of ester using rank correlation coefficient ( $\rho_r$ )

For ester concentrations in the water column  $C_s$ , the organic content of SS ( $\phi_2$ ) has the highest rank correlation coefficient of -0.62. The other key input variables are A and  $\delta$ . The corresponding rank correlation coefficient values are -0.53 and -0.51, respectively. This implies that an increase in organic content and size of SS will result in decrease in the ester concentration in the water column. The higher organic fraction captures pollutants from the water column and settles down. This implies that the deposition process is the most important process in decreasing pollutant concentration in the water column. The remaining input parameters do not significantly affect the water column concentration. Figure 3.13 is a tomado graph for representing percent contribution of each input variable. The three input parameters ( $\phi_2$ , A and  $\delta$ ) contribute more than 96% of the total variance of the water column concentration and the remaining variables contribute less than 4%.

The key input variables for pore water concentration are sediment depth ( $\Delta$ ), half-life in sediments ( $H_{i;0;0}$ ), density ( $\rho_0$ ), and organic fraction ( $\varphi_0$ ) of sediments (Table 3.8). The thickness of the sediment layer ( $\Delta$ ) inversely affects the pore water concentration. It contributes more than 60% of the total variance of pore water concentration. The longer half-life in sediment provides more time for organic pollutants to leach into pore water from the sediment. The organic fraction (12%) and density (7%) of sediments also contribute significantly to the pore water concentration (Figure 3.14).



% contribution

Figure 3.13. Percent contribution of input variables to ester water column concentration

# Heavy Metals (Cd)

Sensitivity analysis results of the aquivalence based model for Cd is presented in Table 3.9. The sensitivity of the output concentrations ( $C_w$  and  $C_{rw}$ ) is measured with the help of the rank correlation coefficient and by determining the percent contribution of input variables. The key input variables for water column concentration are the partitioning coefficients of Cd for sediment ( $K_w$ ) and SS ( $K_w$ ). The other important parameter is advective velocity (m).

The advective velocity affects water column concentration in two ways; the flushing of water from the control volume negatively affects the water column concentration; at the same time it can also scour the deposited material to increase the resuspension, which results in an increase in concentration, which is a positive effect. Overall it has a positive effect on  $C_{p}$ . The partitioning coefficients and velocity contribute to more than 90% of the variance in the water column concentration (Figure 3.15).



Figure 3.14. Percent contribution of input variables to ester pore water concentration

The Cd pore water concentration ( $C_{rw}$ ) is mainly affected by the partitioning coefficient of sediment ( $K_{w}$ ). The pore water concentration is highly sensitive to  $K_{w}$  as it contributes more than 95% to the overall variance (Figure 3.16). The inverse effect of  $K_{w}$  does not allow heavy metal to leach into the pore water.

| Parameters | C.    | C <sub>re</sub> |
|------------|-------|-----------------|
| Ka         | -0.69 | -0.98           |
| u          | 0.45  | -0.02           |
| Κ.         | -0.20 | 0.00            |
| Ug         | -0.16 | -0.16           |
| A          | -0.12 | 0.01            |
| δ          | -0.11 | 0.01            |
| Vs         | 0.12  | 0.01            |
| MT         | 0.07  | -0.01           |
| p.         | -0.02 | 0.01            |
| V.         | 0.04  | 0.03            |
| ρ,         | 0.02  | 0.01            |
| 0,         | 0.00  | 0.00            |
| ş          | 0.06  | 0.01            |
| 4          | 0.01  | 0.01            |
| 0,         | -0.02 | -0.02           |

Table 3.9. Sensitivity analysis (SA) of Cd using rank correlation coefficient ( $\rho_c$ )

The SA results are presented for ester and Cd concentrations in the water column and pore water. The intention here is to identify and highlight the key input parameters and processes that determine the fate of contaminants in the marine environment. The results of SA show that in the case of organic pollutants, the water column concentration is mainly affected by the deposition process. The organic pore water concentration is mainly affected by half-life ( $H_{1/2\alpha}$ ) in sediments, density ( $\rho_0$ ), organic content ( $\phi_0$ ) and depth ( $\Delta$ ) of sediments. The fate of heavy metals in the water column and pore water ( $K_{\phi}$ and  $K_{\phi}$ ) is a function of partitioning coefficients and advective velocity ( $\mu$ ).



% contribution

Figure 3.15. Percent contribution of input variables to Cd water column concentration

# **3.4. MODEL OUTPUT**

# 3.4.1. LOGNORMAL DISTRIBUTION

The concentration of pollutants in environmental media generally follows a positive skewed distribution with a long tail to the right (Sadiq, 1997; Standard Methods, 1998). The environmental concentrations may vary by orders of magnitude and wide variation is observed. The fugacity/aquivalence model outputs;  $C_w$  and  $C_{rw}$  for ester and Cd are shown in Figures 3.7 to 3.12. The MC simulations show that concentrations in both media vary over a wide range and mean values are more than the median for all cases. The major reason for right hand long tail and positive skew in concentration outputs is that fugacity/aquivalence based models are of multiplicative type, which leads to a lognormal (LN) distribution.



Figure 3.16. Percent contribution of input variables to Cd pore water concentration

Burmaster and Hull (1996) described the importance of the two-parameter lognormal distribution in human and ecological risk assessment. They have reasoned that the lognormal distribution is commonly observed in contaminant fate modeling due to the following:

- Many physical, chemical, biological, and toxicological processes tend to create random variables that follow lognormal distribution (Hattis and Burmaster, 1994).
- The mathematical process of multiplying a series of random variables will produce a new random variable, which tends to be lognormal in character (Benjamin and Cornell, 1970).

 Lognormal distributions are self-replicating i.e., products and divisions of lognormal distributions are also lognormal (Crow and Shimizu, 1988).

The above three reasons for obtaining positive skewed distributions are also applicable to contaminant fate modeling outputs in our case as well. Figures 3.17 and 3.18 are lognormal distribution fits to the fugacity model outputs:  $C_w$  and  $C_{rw}$  for the first simulation trial for ester. The Figure 3.17 represents an empirical distribution function (EDF) of water column concentration estimated from MC simulations, and the fitted lognormal cumulative distribution function (CDF). The lognormal distribution (LN) is defined by 2 parameters: the arithmetic mean ( $\mu_x$ ) and the standard deviation ( $\sigma_x$ ) as shown in Figure 3.17.



Figure 3.17. An EDF of ester water column concentration estimated from the fugacity based model and fitted lognormal distribution (LN)

Figure 3.18 represents an EDF of ester pore water concentration, which is the second output of MC simulations using the fugacity based model. The fitted lognormal cumulative distribution function (CDF) is also shown. In both cases a good agreement between simulation results and fitted distributions can be observed.



Figure 3.18. An EDF of ester pore water concentration estimated from the fugacity based model and fitted lognormal distribution (LN)

The two-parameter lognormal distribution takes its name from its fundamental property that the logarithm of the random variable is distributed according to normal or Gaussian distribution (Burmaster and Hull, 1996).

$$(\ln(C)) \sim N(\mu_{\gamma}, \sigma_{\gamma})$$
 (3.2)

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where.

 $\mu_r$  = mean of log of concentration

 $\sigma_r$  = standard deviation of log of concentration

C = lognormally distributed concentration

The @Risk (Palisade, 1997) uses the arithmetic mean ( $\mu_t$ ) and standard deviation ( $\sigma_\tau$ ) of concentration data to generate the lognormal distribution (e.g., Figures 3.17 and 3.18). The arithmetic mean ( $\mu_t$ ) and standard deviation ( $\sigma_\tau$ ) are related to  $\mu_r$  and  $\sigma_\tau$  by the following relationships

$$\mu_{\gamma} = ln \left[ \frac{\mu_{\chi}^2}{\sqrt{\sigma_{\chi}^2 + \mu_{\chi}^2}} \right]$$
(3.3)

and

$$\sigma_{\gamma} = \sqrt{ln \left[\frac{\sigma_{\chi}^{2} + \mu_{\chi}^{2}}{\mu_{\chi}^{2}}\right]} \qquad (3.4)$$

The  $(100 \times p)^{th}$  percentile value for the underlying normal distribution can be calculated as :

Therefore by extension, the  $(100 \times p)^{th}$  percentile for a lognormal distribution can be calculated as follows:

$$|C|_{p} = |exp[N \sim (\mu_{\gamma}, \sigma_{\gamma})]|_{p}$$
  
(3.6)

$$= exp[\mu_{\gamma} + z(p) \times \sigma_{\gamma}]$$

where.

 $z(p) = \Phi^{-1}(Y) = inverse of cumulative distribution function$  $<math>\mu_t = arithmetic mean of concentration (water column or pore water)$  $<math>\sigma_t = standard deviation of concentration$  $<math>/CI_p = Pollutant concentration in any media at desired percentile level (p)$ 

Generally, mean, median, 90<sup>th</sup> or 95<sup>th</sup> percentiles are used as representative environmental concentrations. The U.S. EPA (2000a) used the average value for defining representative environmental concentration. Lenwood et al. (1998) have proposed the 90<sup>th</sup> percentile of concentration as the representative environmental concentration of heavy metals as an exposure to the fresh water community. In this research, a conservative approach was adopted and the 95<sup>th</sup> percentile is defined as the representative estimate of the environmental concentration. This concentration is called the predicted environmental concentration (*PEC*) in the CHARM model (Thatcher et al., 1999).

For  $p = 0.95 \implies z(p) = 1.645$ 

$$C_{ggg} = exp[\mu_{\gamma} + 1.645 \times \sigma_{\gamma}]$$
(3.7)

where.  $C_{0.95}$  is the representative concentration in the media. Substituting equations 3.3 and 3.4 in equation 3.7 will give

$$C_{\star\,0\,se} = exp \left[ ln \left( \frac{\mu_{\star}^{2}}{\sqrt{\sigma_{\star}^{2} + \mu_{\star}^{2}}} \right) + 1.645 \times \sqrt{ln \left( \frac{\sigma_{\star}^{2} + \mu_{\star}^{2}}{\mu_{\star}^{2}} \right)} \right]$$
(3.8)

Similarly.

$$C_{\mu\nu\delta\Psi} = exp\left[ln\left(\frac{\mu_{\mu\nu}^{2}}{\sqrt{\sigma_{\mu\nu}^{2} + \mu_{\mu\nu}^{2}}}\right) + 1.645 \times \sqrt{ln\left(\frac{\sigma_{\mu\nu}^{2} + \mu_{\mu\nu}^{2}}{\mu_{\mu\nu}^{2}}\right)}\right]$$
(3.9)

where.

 $C_{+,0+}$  = pollutant water column concentration at 95<sup>th</sup> percentile (p = 0.95)  $C_{p+,0+}$  = pollutant pore water concentration at 95<sup>th</sup> percentile (p = 0.95)  $\mu_{+}$  = arithmetic mean of lognormally distributed water column concentration  $\sigma_{+}$  = standard deviation of lognormally distributed water column concentration  $\mu_{p+}$  = arithmetic mean of lognormally distributed pore water concentration  $\sigma_{+}$  = standard deviation of lognormally distributed pore water concentration Equations 3.8 and 3.9 can be used to determine the water column and pore water concentrations at the 95<sup>th</sup> percentile while knowing the mean and standard deviation of the underlying lognormal distribution.

### 3.4.2 OUTPUT CONCENTRATION RESULTS

The fugacity or aquivalence ( $f_2$  and/or  $A_2$ ) determines the concentration of pollutants in the water column and pore water, respectively. The  $f_2$  and  $A_2$  are functions of *D*-values, which further linearly relate to impact area ( $A_2$ ). The impact area is assumed to be circular, which is a function of the square of the radius. Therefore,  $f_2$  (and  $A_2$ ) may be inversely proportional to the square of the impact radius (*R*). The  $f_2$  (and  $A_2$ ) are linear functions of loading rates ( $E_2$  and  $E_4$ ). Therefore  $f_2$  (and  $A_2$ ) increases linearly with an increase in total pollutant loading.

Tables 3.10 and 3.11 summarize the mean  $(\mu_n)$  and standard deviation  $(\sigma_n)$  of concentration in the water column, respectively, for the pollutants under study. The results of 15 simulation data sets are summarized in these tables. The comparison of average ester concentration in the water column  $(\mu_n)$  for trials 1 and 2 shows that with an increase in the impact area, the concentration decreases (Table 3.10). By increasing the impact radius from 50 m to 100 m, the mean concentration is decreased from 0.94 to 0.24  $\mu g/L$  for a 150 kg/hr loading rate. The concentration further decreases to 0.01  $\mu g/L$ approximately for a 500 m impact radius. The mean concentration decreases proportionally with an increase in the square of the radius. A similar effect can also be observed by comparing trials 6 and 7 or 11 and 12. Since lognormal distribution is a skewed distribution, the standard deviation ( $\sigma_w$ ) may not necessarily follow the trend observed for mean values. The comparison of standard deviation of ester concentration in the water column ( $\sigma_w$ ) for trials 1 and 2 shows that by increasing the impact radius from 50 m to 100 m. the  $\sigma_w$  is decreased from 0.41 to 0.10 for the 150 kg/hr loading rate (Table 3.12). The  $\sigma_w$  further decreases to 0.004 µg/L approximately for the 500 m impact radius. A similar effect can also be observed by comparing that 5 and 7 or 11 and 12.

The comparison of trial 1 and 6 for ester shows that average concentration decreases from 0.94 to 0.47 µg/L when pollutant loading is reduced from 150 kg/hr to 75 kg/hr. It is further reduced to 0.03 µg/L for 5 kg/hr (Trial 11). This shows an approximately linear relationship between loading rate and concentration in the water column. The comparison of trials 1 and 6 for ester shows that the standard deviation of concentration decreases from 0.41 to 0.20 µg/L when pollutant loading is reduced from 150 kg/hr to 75 kg/hr. It is further reduced to 0.013 µg/L for 5 kg/hr (Trial 11).

A similar trend can be seen for other pollutants listed in Table 3.10 and 3.11. Figures 3.19 and 3.20 are the 3D surface plots of the mean ( $\mu_{pn}$ ) and standard deviation ( $\sigma_{pn}$ ) of water column concentrations for ester as a function of pollutant loading rates (*E*) and impact radius (*R*).

Tables 3.12 and 3.13 summarize the mean  $(\mu_{pw})$  and standard deviation  $(\sigma_{pw})$  of pore water concentrations, respectively for various pollutants. The results of 15 simulation data sets are summarized. The comparison of ester concentration in pore water for trials 1 and 2 shows that with an increase in the impact area, the concentration decreases. For impact radii of 50 m to 100 m the average concentration is decreased from 11.8 to 2.96 g/L for the 150 kg/hr loading rate (Table 3.12). The concentration further decreases to 0.12 g/L at 500 m impact radius. The average concentration decreases proportionally with an increase in square of the impact radius. A similar effect of impact radius can also be observed by comparing trials 6 and 7 or 11 and 12.



Figure 3.19. A 3-D surface plot of mean water column concentration  $(\mu_w)$  of ester as a function of loading rate and impact radius

The comparison of the standard deviation of ester concentration in the water column  $(\sigma_{pw})$  for trials 1 and 2, shows that by increasing the impact radius from 50 m to 100 m, the  $(\sigma_{pw})$  is decreased from 2.52 to 0.63 g/L for the 150 kg/hr loading rate (Table 3.13). The  $\sigma_{pn}$  further decreases to 0.025 g/L approximately at the 500 m impact radius. A similar effect can also be observed by comparing trials 6 and 7 or 11 and 12.

The comparison of trials 1 and 6 for ester shows that mean pore water concentration ( $\mu_{pw}$ ) decreases from 11.8 to 5.91 g/L when pollutant loading is reduced from 150 kg/hr to 75 kg/hr. It is further reduced to 0.39  $\mu$ g/L for 5 kg/hr (Trial 11). This implies an approximately linear relationship between loading rate and concentration in pore water. The comparison of trial 1 and 6 for ester shows that the standard deviation of pore water concentration decreases from 2.52 to 1.25 g/L when pollutant loading is reduced from 150 kg/hr to 75 kg/hr. It is further reduced to 0.084  $\mu$ g/L for 5 kg/hr (Trial 11).

A similar trend can be seen for other pollutants listed in Tables 3.12 and 3.13. Figures 3.21 and 3.22 are the 3D surface plots of the mean and standard deviation of pore water concentration for ester as a function of pollutant loading rate and impact radius.

The results of MC simulations are summarized in Tables 3.10 to 3.13. The results are provided in the form of mean and standard deviation of 10.000 iterations for each simulation. As the mean and standard deviation are the characteristic parameters of the lognormal distribution, the parameters of the lognormal distribution can be used to predict representative environmental concentration in the water column and pore water at any desired percentile level. In the next section these characteristic parameters are used to develop regression models for each pollutant in terms of impact radius and pollutant loading rates.

| Trials | As       | Cd       | Cr       | Cu       | Hg       | Ni       | Pb       | Zn       | Naphthalene | Ester    |
|--------|----------|----------|----------|----------|----------|----------|----------|----------|-------------|----------|
| 1      | 7.75E-05 | 6.77E-05 | 1.11E-04 | 3.11E-05 | 6.58E-08 | 3.12E-05 | 3.12E-05 | 1.44E-04 | 1.22E-11    | 9.40E-01 |
| 2      | 2.09E-05 | 1.85E-05 | 2.95E-05 | 8.20E-06 | 1.66E-08 | 8.00E-06 | 8.00E-06 | 3.67E-05 | 3.06E-12    | 2.35E-01 |
| 3      | 9.65E-07 | 7.85E-07 | 1.26E-06 | 3.39E-07 | 6.59E-10 | 3.50E-07 | 3.50E-07 | 1.62E-06 | 1.23E-13    | 9.45E-03 |
| 4      | 2.52E-07 | 2.06E-07 | 3.24E-07 | 8.92E-08 | 1.65E-10 | 8.77E-08 | 8.77E-08 | 4.20E-07 | 3.07E-14    | 2.36E-03 |
| 5      | 1.02E-08 | 8.32E-09 | 1.34E-08 | 3.51E-09 | 6.63E-12 | 3.50E-09 | 3.50E-09 | 1.71E-08 | 1.23E-15    | 9.43E-05 |
| 6      | 2.53E-05 | 2.34E-05 | 3.64E-05 | 1.04E-05 | 2.20E-08 | 1.00E-05 | 1.00E-05 | 4.66E-05 | 4.08E-12    | 4.69E-01 |
| 7      | 6.99E-06 | 6.27E-06 | 9.58E-06 | 2.70E-06 | 5.52E-09 | 2.62E-06 | 2.62E-06 | 1.26E-05 | 1.02E-12    | 1.18E-01 |
| 8      | 3.13E-07 | 2.62E-07 | 4.07E-07 | 1.16E-07 | 2.23E-10 | 1.13E-07 | 1.13E-07 | 5.33E-07 | 4.09E-14    | 4.71E-03 |
| 9      | 8.46E-08 | 6.70E-08 | 1.03E-07 | 2.86E-08 | 5.54E-11 | 2.91E-08 | 2.91E-08 | 1.37E-07 | 1.02E-14    | 1.18E-03 |
| 10     | 3.55E-09 | 2.67E-09 | 4.23E-09 | 1.20E-09 | 2.16E-12 | 1.17E-09 | 1.17E-09 | 5.39E-09 | 4.10E-16    | 4.71E-05 |
| н      | 6.23E-07 | 5.69E-07 | 9.23E-07 | 2.57E-07 | 5.54E-10 | 2.58E-07 | 2.58E-07 | 1.20E-06 | 1.02E-13    | 3.12E-02 |
| 12     | 1.76E-07 | 1.53E-07 | 2.39E-07 | 6.80E-08 | 1.36E-10 | 6.86E-08 | 6.86E-08 | 3.17E-07 | 2.56E-14    | 7.85E-03 |
| 13     | 8.18E-09 | 6.52E-09 | 1.07E-08 | 2.83E-09 | 5.50E-12 | 2.80E-09 | 2.80E-09 | 1.33E-08 | 1.02E-15    | 3.15E-04 |
| 14     | 2.03E-09 | 1.63E-09 | 2.55E-09 | 7.27E-10 | 1.38E-12 | 7.21E-10 | 7.21E-10 | 3.53E-09 | 2.56E-16    | 7.86E-05 |
| 15     | 8.58E-11 | 6.50E-11 | 1.08E-10 | 2.98E-11 | 5.46E-14 | 2.87E-11 | 2.87E-11 | 1.37E-10 | 1.02E-17    | 3.14E-06 |

Table 3.10. The results of MC simulations for mean ( $\mu_w$ ) water column concentration ( $\mu g/L$ )

| Trials | As       | Cd       | Cr       | Cu       | Hg       | Ni       | РЬ       | Zn       | Naphthalene | Ester    |
|--------|----------|----------|----------|----------|----------|----------|----------|----------|-------------|----------|
| I      | 1.54E-04 | 1.64E-04 | 3.23E-04 | 7.61E-05 | 1.32E-07 | 7.72E-05 | 7.72E-05 | 3.79E-04 | 6.66E-12    | 4.07E-01 |
| 2      | 4.25E-05 | 5.01E-05 | 9.69E-05 | 2.08E-05 | 3.36E-08 | 2.15E-05 | 2.15E-05 | 1.03E-04 | 1.68E-12    | 1.00E-01 |
| 3      | 2.58E-06 | 2.24E-06 | 4.87E-06 | 1.01E-06 | 1.20E-09 | 1.03E-06 | 1.03E-06 | 6.05E-06 | 6.79E-14    | 4.09E-03 |
| 4      | 7.34E-07 | 6.12E-07 | 1.50E-06 | 2.62E-07 | 3.13E-10 | 2.71E-07 | 2.71E-07 | 1.61E-06 | 1.73E-14    | 1.02E-03 |
| 5      | 3.42E-08 | 2.54E-08 | 6.18E-08 | 1.05E-08 | 1.23E-11 | 1.02E-08 | 1.02E-08 | 7.78E-08 | 6.82E-16    | 4.01E-05 |
| 6      | 4.58E-05 | 6.06E-05 | 1.08E-04 | 2.62E-05 | 4.25E-08 | 2.47E-05 | 2.47E-05 | 1.19E-04 | 2.32E-12    | 1.98E-01 |
| 7      | 1.48E-05 | 1.76E-05 | 3.09E-05 | 6.80E-06 | 1.06E-08 | 6.41E-06 | 6.41E-06 | 3.73E-05 | 5.56E-13    | 5.03E-02 |
| 8      | 7.94E-07 | 7.61E-07 | 1.51E-06 | 3.47E-07 | 5.21E-10 | 3.20E-07 | 3.20E-07 | 1.88E-06 | 2.31E-14    | 2.01E-03 |
| 9      | 2.55E-07 | 1.95E-07 | 3.69E-07 | 8.36E-08 | 1.24E-10 | 8.63E-08 | 8.63E-08 | 4.80E-07 | 561E-15     | 5.07E-04 |
| 10     | 1.26E-08 | 7.74E-09 | 1.84E-08 | 4.32E-09 | 3.88E-12 | 3.83E-09 | 3.83E-09 | 1.85E-08 | 2.40E-16    | 2.02E-05 |
| 11     | 1.07E-06 | 1.38E-06 | 2.71E-06 | 6.24E-07 | 1.03E-09 | 6.52E-07 | 6.52E-07 | 3.18E-06 | 5.90E-14    | 1.31E-02 |
| 12     | 3.53E-07 | 4.13E-07 | 7.68E-07 | 1.75E-07 | 2.67E-10 | 1.77E-07 | 1.77E-07 | 9.51E-07 | 1.56E-14    | 3.38E-03 |
| 13     | 2.21E-08 | 1.93E-08 | 4.03E-08 | 7.80E-09 | 1.11E-11 | 7.96E-09 | 7.96E-09 | 4.44E-08 | 6.03E-16    | 1.36E-04 |
| 14     | 5.97E-09 | 4.48E-09 | 9.46E-09 | 2.07E-09 | 2.67E-12 | 2.28E-09 | 2.28E-09 | 1.54E-08 | 1.49E-16    | 3.38E-05 |
| 15     | 2.70E-10 | 1.94E-10 | 5.12E-10 | 9.36E-11 | 9.56E-14 | 8.26E-11 | 8.26E-11 | 5.68E-10 | 5.96E-18    | 1.35E-06 |

Table 3.11. The results of MC simulations for standard deviation ( $\sigma_w$ ) of water column concentration (µg/L)

| _           |          |          |          |          |          |          |          |          |          |          |          |          |          |          |          |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|
| Ester       | 1.18E+07 | 2.96E+06 | 1.19E+05 | 2.96E+04 | 1.19E+03 | 5.91E+06 | 1.48E+06 | 5.93E+04 | 1.48E+04 | 5.93E+02 | 3.94E+05 | 9.86E+04 | 3.95E+03 | 9.88E+02 | 3.95E+01 |
| Naphthalene | 7.39E-04 | 1.85E-04 | 7.41E-06 | 1.85E-06 | 7.42E-08 | 2.46E-04 | 6.18E-05 | 2.47E-06 | 6.17E-07 | 2.47E-08 | 6.16E-06 | 1.55E-06 | 6.17E-08 | 1.54E-08 | 6.18E-10 |
| Zn          | 8.53E+05 | 2.13E+05 | 8.51E+03 | 2.12E+03 | 8.67E+01 | 2.85E+05 | 7 12E+04 | 2.87E+03 | 7.24E+02 | 2.88E+01 | 7.04E+03 | 1.81E+03 | 7.20E+01 | 1.79E+01 | 7.03E-01 |
| Pb          | 4.37E+05 | 1.10E+05 | 4.62E+03 | 1.12E+03 | 4.61E+01 | 1.44E+05 | 3.63E+04 | 1.50E+03 | 3.78E+02 | 1.53E+01 | 3.61E+03 | 9.31E+02 | 3.72E+01 | 9.29E+00 | 3.74E-01 |
| z           | 4.37E+05 | 1.10E+05 | 4.62E+03 | 1.12E+03 | 4.61E+01 | 1.44E+05 | 3.63E+04 | 1.50E+03 | 3.78E+02 | 1.53E+01 | 3.61E+03 | 9.31E+02 | 3.72E+01 | 9.29E+00 | 3.74E-01 |
| Hg          | 3.09E+00 | 7.72E-01 | 3.08E-02 | 7.70E-03 | 3.13E-04 | 1.04E+00 | 2.58E-01 | 1.03E-02 | 2.58E-03 | 1.04E-04 | 2.57E-02 | 6.44E-03 | 2.60E-04 | 6.51E-05 | 2.61E-06 |
| Cu          | 4.30E+05 | 1.09E+05 | 4.48E+03 | 1.15E+03 | 4.54E+01 | 1.44E+05 | 3.72E+04 | 1.51E+03 | 3.74E+02 | 1.51E+01 | 3.61E+03 | 9.24E+02 | 3.74E+01 | 9.37E+00 | 3.75E-01 |
| Ċ           | 2.61E+05 | 6.55E+04 | 2.60E+03 | 6.50E+02 | 2.59E+01 | 8.62E+04 | 2.16E+04 | 8.76E+02 | 2.20E+02 | 8.64E+00 | 2.18E+03 | 5.40E+02 | 2.17E+01 | 5.49E+00 | 2.17E-01 |
| Cd          | 9.64E+05 | 2.54E+05 | 1.05E+04 | 2.64E+03 | 1.08E+02 | 3.26E+05 | 8.60E+04 | 3.55E+03 | 8.80E+02 | 3.47E+01 | 8.02E+03 | 2.13E+03 | 8.81E+01 | 2.18E+01 | 9.10E-01 |
| As          | 2.73E+05 | 6.90E+04 | 2.81E+03 | 7.08E+02 | 2.85E+01 | 9.09E+04 | 2.29E+04 | 9.44E+02 | 2.36E+02 | 9.48E+00 | 2.28E+03 | 5.76E+02 | 2.35E+01 | 5.90E+00 | 2.36E-01 |
| Trials      | -        | 2        | 3        | 4        | \$       | 9        | 7        | 8        | 6        | 0        | =        | 12       | 13       | 4        | 15       |

Table 3.12. The results of MC simulations for mean ( $\mu_{pu}$ ) pore water concentration (µg/L)

| Cd              | _      | 5    | C.       | Hg       | ï        | Pb       | Zn       | Naphthalene | Ester    |
|-----------------|--------|------|----------|----------|----------|----------|----------|-------------|----------|
| 2.64E+06 8.28   | 8.28   | E+05 | 1.30E+06 | 4.27E+00 | 1.31E+06 | 1.31E+06 | 2.46E+06 | 2.92E-04    | 2.52E+06 |
| 7.22E+05 2.05   | 2.05   | E+05 | 3.28E+05 | 1.07E+00 | 3.28E+05 | 3.28E+05 | 6.08E+05 | 7.29E-05    | 6.32E+05 |
| 3.03E+04 8.17I  | 8.171  | E+03 | 1.39E+04 | 4.25E-02 | 1.46E+04 | 1.46E+04 | 2.41E+04 | 2.95E-06    | 2.53E+04 |
| 7.65E+03 2.03H  | 2.03   | 3+03 | 3.53E+03 | 1.06E-02 | 3.42E+03 | 3.42E+03 | 5.93E+03 | 7.35E-07    | 6.33E+03 |
| 3.19E+02 8.13E  | 8.13E  | 10+  | 1.38E+02 | 4.36E-04 | 1.45E+02 | 1.45E+02 | 2.51E+02 | 2.94E-08    | 2.52E+02 |
| 8.98E+05 2.73E  | 2.73E  | +05  | 4.38E+05 | 1.46E+00 | 4.33E+05 | 4.33E+05 | 8.29E+05 | 9.78E-05    | 1.25E+06 |
| 2.46E+05 6.74E  | 6.74E  | 104  | 1.14E+05 | 3.57E-01 | 1.08E+05 | 1.08E+05 | 2.03E+05 | 2.50E-05    | 3.16E+05 |
| 1.05E+04 2.77E+ | 2.77E+ | 03   | 4.66E+03 | 1.43E-02 | 4.63E+03 | 4.63E+03 | 8.19E+03 | 9.87E-07    | 1.27E+04 |
| 2.58E+03 6.98E+ | 6.98E+ | 02   | 1.14E+03 | 3.57E-03 | 1.15E+03 | 1.15E+03 | 2.10E+03 | 2.44E-07    | 3.17E+03 |
| 9.95E+01 2.69E+ | 2.69E+ | 10   | 4.60E+01 | 1.446-04 | 4.82E+01 | 4.82E+01 | 8.23E+01 | 9.79E-09    | 1.27E+02 |
| 2.20E+04 6.92E+ | 6.92E+ | -03  | 1.08E+04 | 3.53E-02 | 1.08E+04 | 1.08E+04 | 2.00E+04 | 2.44E-06    | 8.42E+04 |
| 6.13E+03 1.70E  | 1.70E  | +03  | 2.81E+03 | 8.90E-03 | 2.84E+03 | 2.84E+03 | 5.23E+03 | 6.25E-07    | 2.10E+04 |
| 2.56E+02 6.79E  | 6.79E  | 10+  | 1.17E+02 | 3.64E-04 | 1.15E+02 | 1.15E+02 | 2.07E+02 | 2.44E-08    | 8.42E+02 |
| 6.30E+01 1.75E  | 1.75E  | 10+  | 2.84E+01 | 9.05E-05 | 2.86E+01 | 2.86E+01 | 5.12E+01 | 6.17E-09    | 2.11E+02 |
| 2.73E+00 6.76E  | 6.76E  | 10-  | 1.15E+00 | 3.67E-06 | 1.15E+00 | 1.15E+00 | 1.98E+00 | 2.44E-10    | 8.36E+00 |

Table 3.13. The results of MC simulations for standard deviation ( $\sigma_{pu}$ ) of pore water concentration ( $\mu g/L$ )



Figure 3.20. A 3-D surface plot of standard deviation of the water column concentration  $(\sigma_w)$  of ester as a function of loading rate and impact radius



Figure 3.21. A 3-D surface plot of the mean pore water concentration ( $\mu_{pw}$ ) of ester as a function of loading rate and impact radius



Figure 3.22. A 3-D surface plot of the standard deviation of pore water concentration  $(\sigma_{pw})$  of ester as a function of loading rate and impact radius

# 3.5. REGRESSION MODELS

The immediate objective is to develop a relationship for environmental concentration (equations 3.8 and 3.9) as a function of loading rate and impact radius. The characteristic parameters of the lognormal distribution are the mean and standard deviation, which are estimated from MC simulations. They are used to estimate predicted environmental concentration (*PEC*). The environmental concentration can be expressed in terms of pollutant loading rate (E) and impact radius (R).

A similar approach is adopted at Louisiana Bay for the discharges of produced waters. The CORMIX model is used to estimate the dilution factors (*DF*) available at known distances from the point of discharge. Under site specific conditions, the dilution factor (DF) is calculated as a function of discharge rate (G) (Meinhold et al., 1995) at known distances using regression analysis.

Regression methods are frequently used to analyze data from unplanned experiments and are also very useful in the design of experiments. The control or independent variables are varied to observe the effects of these factors on the output of the model. The multiple regression models involve more than one variable. The general form for fitting multiple linear regression is given as

$$Z = \alpha_0 + \alpha_1 X_1 + \alpha_2 X_2 + \alpha_1 X_3 + \dots + \alpha_k X_k + \varepsilon$$
(3.10)

where  $X_i, X_2, ..., X_k$  are the independent or control variables, the unknown parameters " $\alpha$ 's" are regression coefficients and " $\epsilon$ " is an error term. The method of least squares is generally used to estimate the regression coefficients.

In multiple linear regression, the relationship between variables is assumed to be linear. In practice this assumption can virtually never be confirmed: fortunately, multiple regression procedures are not greatly affected by minor deviations from this assumption. Transformation of data is generally performed to convert into a linear form. Linear regression assumes that the scatter of the points around the line follows a Gaussian distribution and that the standard deviation is the same at all values of independent variables (Xr). The MC simulation results show that the concentrations in multimedia are a function of pollutant loading and impact radius under given conditions that can also be determined from the physics of the processes. When allowing for any type of relationship between the independent variables and the dependent variable, two issues arise - First, what types of relationships are logical, that is, are they interpretable in a meaningful manner. The second issue is how to compute this relationship. The fugacity  $(f_2)$  or aquivalence  $(A_2)$  of water is a linear function of pollutant loading rate (E) and area of impact  $(A_0)$ . From a general observation of the data in Tables 3.10 to 3.13, a decrease in concentration is observed with a decrease in pollutant loading and an increase in impact radius. A multiplicative regression model form can be anticipated.

$$C_r \sim f\left(E \times \frac{I}{R}\right)$$
  
(3.11)

The transformation of data is very common in regression analysis. The log transformation is the most common type of transformation. The log transformation of concentration (LC), pollutant loading rates (LE) and impact radius (LR) can convert the above relationship into a linear form, which is similar to equation 3.10.

$$LC = a + b \times LE + c \times LR \tag{3.12}$$

where a, b and c are regression coefficients and can be calculated by multiple linear regression. The objective of fitting a best linear model is achieved through minimizing the sum-of-squares (SS,) of the vertical distances of the point from the predicted line. The most common technique employed to estimate the parameters in linear models is the method of least squares and the best estimates of the parameters are obtained by using

$$\min SS_{i} = \min \sum (Le_{i})^{2} = \min \sum (LC_{i} - LC_{i})^{2}$$
(3.13)

where the min SS, represents a minimum sum of squares of the errors. Le, represents the  $i^{th}$  term of errors. LC, is the  $i^{th}$  term of simulated values and the LC, is the  $i^{th}$  value determined from the model equations (Pai et. al. 2000).

After estimating the regression parameters, an essential aspect of the analysis is to test the appropriateness of the overall model. There are several parameters to evaluate the appropriateness of a model. The ratio of the regression sum of squares to the total sum of squares ( $SS_R/Sr_T$ ) explains the proportion of variance accounted for in the dependent variable (LC) by the model. This ratio is equivalent to the coefficient of determination ( $R^2$ ) that has a value between 0 and 1. Even when the dependent variable is not normally distributed, this measure may help evaluate how well the model fits the data. The plot of observed and predicted values also gives an indication of the appropriateness of the model. If the model is appropriate for the data, then the points roughly follow a straight line. The normal probability plot of residuals will give an indication of whether or not the residuals (errors) are normally distributed. This plot is used to check the basic assumption behind the method of least squares.

### 3.5.1. DEVELOPMENT OF REGRESSION MODELS

In this section, a methodology for development of empirical models is given for the mean  $(\mu_{pn})$  and standard deviation  $(\sigma_{pn})$  of ester pore water concentration. The Minitab software (version 12.1) was used for the multiple regression (Minitab. 1998). The mean concentration  $(\mu_{pn})$ , loading rate and impact radius data were first log transformed and multiple regression was performed (Table 3.10). The following equations were obtained for the mean  $(\mu_{pn})$  and standard deviation  $(\sigma_{pn})$ 

$$LC_{\mu} = 2.296 - 1.999 LR + 1.000 LE$$
 (3.14)

where.

$$al = (2.296, 0.003)$$
  
 $bl = (1.000, 0.002)$   
 $cl = (1.999, 0.0001)$   
 $\hat{LC}_{\mu} = an estimated value of  $\mu_{pre}$  (or  $LC_{\mu}$ ) from regression model$ 

Similarly,

$$LC_{a} = 1.625 - 2.000 LR + 1.001 LE$$
 (3.15)

where.

a2 = (1.625, 0.002)

b2 = (1.001, 0.001)

c2 = (2.000, 0.001)

 $LC_{\sigma}$  = an estimated value of  $\sigma_{pw}$  (or  $LC_{\sigma}$ ) from regression model

Table 3.14 provides the regression statistics of the above models. By comparing equations 3.14 and 3.15 with equation 3.12, the coefficients (*a. b* and *c*) can be estimated. The uncertainty in the coefficient is estimated by the standard deviation of the estimate (*SE*). The *SE* values are very low in comparison to the estimates and coefficients are highly significant i.e., p < 0.0001. The errors or residuals in the regression are defined by  $Le_i = LC_i - LC_i$ . The normality of residuals is also checked for both models as shown in Figures 3.23 and 3.24. No serious deviation from assumptions is observed.

|                         | Log tr     | Bias        | Dendiator estual |        |       |                   |        |
|-------------------------|------------|-------------|------------------|--------|-------|-------------------|--------|
| Parameter               | Predictor  | Coefficient | SE               | 1      | p > t | correction<br>(B) | value  |
| Mean<br>µ <sub>P*</sub> | al         | 2.296       | 3.0E-04          | 9019   | 0.000 |                   | 197.70 |
|                         | ы          | 1.000       | 2.0E-04          | 6837   | 0.000 | 1.0000003         | 1.00   |
|                         | cl         | -1.999      | 1.3E-04          | -15309 | 0.000 | 1                 | -2.00  |
| St. dev.                | a2         | 1.623       | 1.3E-03          | 1284   | 0.000 |                   | 41.98  |
|                         | <i>b</i> 2 | 1.001       | 1.0E-03          | 1377   | 0.000 | 1.000007          | 1.00   |
| $\sigma_{pw}$           | c2         | -2.000      | 1.0E-03          | -3083  | 0.000 | 1                 | -2.00  |

Table 3.14. Statistics of regression models for mean and standard deviation of ester pore water concentration




The residuals were also plotted against the log of predicted concentration in Figure 3.25. This plot is used to observe the patterns that could suggest serious deviation from assumption of constant variance, error in analysis or model inadequacy (Dingman and Sharma, 1997).

Regression that predicts the logarithm of a quantity usually gives low biased predictions of the anti-logarithm, which is the actual model (Dingman and Sharma, 1997). A correction is used for this bias by multiplying the real space predictions by a robust biascorrection factor (*B*).



Figure 3.24. Normal probability plot of residuals for standard deviation of ester pore water concentration

$$B = \frac{\sum I 0^{Le_i}}{N}$$
(3.16)

where, N are the number of data points. The bias correction factors (B) are very low for both models, i.e., 1.0000003 and 1.000007 for mean and standard deviation models, respectively. By applying the correction and taking anti-logarithms of equations 3.14 and 3.15, the following relationships can be obtained.



Figure 3.25. Model residuals scatter plot for ester pore water concentration

$$C_{\mu} = 197.70 \left( \frac{E^{1.00}}{R^{2.00}} \right) \text{ for mean ester pore water concentration}$$
(3.17)

and

$$C_{\sigma} = 41.98 \left(\frac{E^{100}}{R^{200}}\right)$$
for standard deviation of ester pore water concentration (3.18)

The model appropriateness can be determined by inspecting the coefficient of determination ( $R^2$ ) and its adjusted value ( $R_a^2$ ). The adjusted values of  $R^2$  values for both models are approximately 1, which shows a very good agreement between predicted and simulated values (Table 3.14). The average residual values E(Le) and standard deviation of residuals S(Le) are used for checking accuracy and precision of a model, respectively. Both parameters are grouped into one, by defining  $RMSE_{\mu}$  (root mean squared error).

$$RMSE_{i} = (E^{2}(Le) + S^{2}(Le))^{1/2}$$
(3.19)

The RMSE<sub>12</sub> are calculated for both models as given in Table 3.15.

R<sup>2</sup>  $R_a^2$ Pollutants RMSE F p > FEster 1.000 1.000 3.61E-04 1.41E+08 0.000 Cd 1.000 1.000 1.79E-03 5.70E+06 0.000

Table 3.15. Model appropriateness statistics for regression models

#### 3.5.2. SUMMARY OF REGRESSION MODELS

The regression model development procedure described in the preceding section was applied to all pollutants for water column and pore water individually. A summary of the coefficients for different regression models is given in Tables 3.16 and 3.17.

The  $R_a^2$  value estimated for all models were more than 0.9999 and were rounded off to the third decimal place to make 1.000. The bias correction factors (*B*) calculated for these models are very small and do not have any significant effect on the final result after transformation. The bias correction factors are not reported in Tables 3.16 and 3.17. The *RMSE<sub>c</sub>*, are calculated and reported for all regression models. The smaller values show high accuracy and precision in regression model predictions. The values of regression coefficients and their standard error values are reported to determine the uncertainty in the estimates.

The regression models are developed as a function of loading rate (*E*) and impact radius (*R*) which can be used in equations 3.8 and 3.9 to get the predicted environmental concentration at any desired percentile (p = 0.95). The mean and standard deviations of the water column ( $\mu_{x}$  and  $\sigma_{x}$ ) and pore water ( $\mu_{pw}$  and  $\sigma_{pw}$ ) concentrations are a function of pollutant loading and impact radius. the coefficients are given in Tables 3.16 and 3.17. The final form of the equations 3.8 and 3.9 becomes

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|---|--------------|-----------------|-----------------|----------------|---------|-------|-----------------|----------------|----------------|--------|-------|
| As         (-1001, 0.02)         (1.002, 0.002)         (1.92, 0.001)         (0.193, 0.014)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.291, 0.021)         (0.211, 0.021)         (0.291, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.211, 0.021)         (0.212, 0.021)         (0.213, 0.021)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)         (0.223, 0.012)   | FOILUTION    | +al             | 19              | cl             | RMSE.   | R'.   | *u2             | <i>b</i> 2     | c2             | RMSEu  | R'.   |
| Cd         (-10.20, 0.02)         (1.005, 0.004)         (1.905, 0.004)         (1.905, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)         (1.002, 0.004)   | As           | (-10.01, 0.02)  | (1.002, 0.005)  | (1.932, 0.007) | 0.01798 | 1.000 | (-9.67, 0.03)   | (1101,0011)    | (1.798, 0.014) | 0.0391 | 0.999 |
| Cr         (-10.99, 0.01)         (1.007, 0.004)         (1.955, 0.004)         (0.12.06)         (0.10.3)         (1.01.30)         (0.10.3)         (1.01.30)           Cu         (-10.154, 0.01)         (1.01, 0.003)         (0.71, 0.003)         (0.971, 0.003)         (0.971, 0.003)         (0.0236)         (1.001, 0.003)         (0.101, 0.003)         (0.1021, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.101, 0.013)         (0.102, 0.013)         (0.102, 0.013)         (0.102, 0.013)         (0.102, 0.013)         (0.102, 0.013)         (0.103, 0.013  | Cd           | (-10.20, 0.02)  | (1.005, 0.004)  | (1.967, 0.005) | 0.01423 | 1.000 | (-9.76, 0.03)   | (1.009, 0.009) | (1.932, 0.011) | 0.0316 | 1.000 |
| Cu         (-10.54, 0.01)         (1.001, 0.003)         (0.971, 0.003)         (0.971, 0.003)         0.0238         1.00           Hg         (-11.24, 0.013)         (1.001, 0.003)         (2.001, 0.001)         (2.001, 0.001)         0.0023         1.00           N         (-10.37, 0.013)         (0.971, 0.003)         0.0125         1.000         (-10.36, 0.007)         (1.01, 0.013)         0.0129         1.00           N         (-10.17, 0.01)         (0.997, 0.004)         0.0125         1.000         (-0.058, 0.017)         (1.01, 0.013)         0.0129         1.00           Ps         (-10.37, 0.01)         (0.997, 0.004)         0.0126         1.000         (-0.058, 0.012)         0.0129         1.00           Za         (-10.87, 0.012)         (0.997, 0.013)         0.0136         1.000         (-0.058, 0.012)         0.0139         1.00           Za         (-10.87, 0.012)         (0.997, 0.013)         0.0136         1.000         (-0.058, 0.013)         0.0137         1.00           Za         (-10.87, 0.013)         (0.997, 0.013)         0.0136         1.000         (-0.058, 0.013)         0.0339         0.033         1.00           Za         (-10.87, 0.013)         (0.998, 0.013)         (0.998, 0.013)         (-0.958, 0.013) <td>5</td> <td>(-10.99, 0.01)</td> <td>(1.003, 0.004)</td> <td>(1.965, 0.004)</td> <td>0.01200</td> <td>1.000</td> <td>(-10.41, 0.04)</td> <td>(1.008, 0.010)</td> <td>(1.871,0.013)</td> <td>0.036</td> <td>1.000</td>             | 5            | (-10.99, 0.01)  | (1.003, 0.004)  | (1.965, 0.004) | 0.01200 | 1.000 | (-10.41, 0.04)  | (1.008, 0.010) | (1.871,0.013)  | 0.036  | 1.000 |
| Hg         (-13.26, 0.003)         (1.001, 0.001)         (2.001, 0.001)         (2.001, 0.001)         (0.017, 0.001)         (0.017, 0.011)         (0.017, 0.011)         (0.017, 0.011)         (0.017, 0.011)         (0.017, 0.011)         (0.0120)         (1.001, 0.001)         (1.011, 0.011)         (0.0120)         (1.001, 0.001)         (1.012, 0.0120)         (1.001, 0.001)         (1.012, 0.0120)         (1.002, 0.001)         (1.012, 0.0120)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.002, 0.001)         (1.001, 0.001)   | 5            | (-10.54, 0.01)  | (1.001, 0.003)  | (1.971, 0.003) | 0.00866 | 1.000 | (-10.09, 0.02)  | (1.006, 0.007) | (1.911, 0.009) | 0.0238 | 1.000 |
| Ni         (-10.17, 0.01)         (0.971, 0.004)         (1.971, 0.005)         (0.126)         (1.000, 0.002)         (1.071, 0.005)         (0.126)         (1.000, 0.002)         (1.072, 0.005)         (1.022, 0.006)         (1.023, 0.012)         (1.023, 0.012)         (1.023, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.032, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (1.036, 0.012)         (   | Hg           | (-13.26, 0.003) | (1001,0001)     | (2.001, 0.001) | 0.0032  | 1.000 | (-12.99, 0.03)  | (1.006, 0.007) | (2.013, 0.013) | 0.0370 | 1.000 |
| Pb         (-10.53, 001)         (1.001, 0000)         (1.972, 0004)         0.0116         1.000         (-10.088, 0019)         (1.022, 0004)         0.0320         1.003           Zn         (-10.87, 0012)         (0.999,0004)         (1.966, 0005)         0.0119         1.000         (-10.286, 0006)         0.0351         0.999         0.0331         1.099           Zn         (-10.87, 0102)         (0.999,0001)         (1.996, 0005)         0.0119         1.000         (-10.286, 0106)         0.0351         0.0351         0.0351         0.0351         0.0391         0.0351 <td>ïŻ</td> <td>(-10.17, 0.01)</td> <td>(0.997,0.004)</td> <td>(1.971, 0.005)</td> <td>0.0126</td> <td>1.000</td> <td>(-9.687, 0.02)</td> <td>(0.989, 0.007)</td> <td>(1.941, 0.009)</td> <td>0.0239</td> <td>1.000</td> | ïŻ           | (-10.17, 0.01)  | (0.997,0.004)   | (1.971, 0.005) | 0.0126  | 1.000 | (-9.687, 0.02)  | (0.989, 0.007) | (1.941, 0.009) | 0.0239 | 1.000 |
| Zn         (-10.87, 0.02)         (0.999, 0.041)         (1.966, 0.005)         (0.11)         (1.00)         (-10.55, 0.015)         (1.869, 0.015)         (0.1869, 0.015)         (0.1869, 0.015)         (0.1869, 0.015)         (0.1869, 0.015)         (0.1996, 0.015)         (0.1001)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.996, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.996, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.997, 0.016)         (0.994, 0.012)         (0.   | P            | (-10.55, 0.01)  | (1.001, 0.003)  | (1.972, 0.004) | 0.0116  | 1.000 | (-10.088, 0.03) | (1.002, 0.009) | (1.925, 0.012) | 0.0320 | 1.000 |
| Apphhalene         (-16.99, 0.001)         (0.999, 0.001)         0.0097         0.0097         1.000           Easer         (-4.844,0001)         (1.990,00004)         (1.990,00004)         (1.990,00004)         0.0097         1.000  | Zn           | (-10.87, 0.02)  | (0.999,0.004)   | (1.966, 0.005) | 0.0139  | 1.000 | (-10.286, 0.06) | (0.994, 0.015) | (1.869, 0.019) | 0.0531 | 0.999 |
| Ester (-4.804,0.001) (1.000,0.0004) (1.999,0.003) 0.0009 1.000 (-5.172,0.004) (1.001,0.002) (1.998,0.022) 0.0049 1.00   | Vaphthalene  | (-16.99, 0.001) | (100.0,999.0.0) | (1000,0.6661)  | 0.00006 | 1.000 | (-17.20, 0.01)  | (0.987, 0.003) | (1.996, 0.004) | 7600.0 | 1.000 |
|   | Ester        | (-4.804.0.001)  | (1.000,0,0004)  | (1.999, 0.003) | 0.0009  | 1.000 | (-5.172, 0.004) | (1.001, 0.002) | (1.998, 0.002) | 0.0049 | 1.000 |

he values are log transformed, take antilog to get the actual values

Table 3.17. Statistics of regression models for mean and standard deviation pore water concentration

|            |                     | _               |                 |                 |                 |                |                |                |                 | _                 |                  |                       |
|------------|---------------------|-----------------|-----------------|-----------------|-----------------|----------------|----------------|----------------|-----------------|-------------------|------------------|-----------------------|
|            | R'.                 | 1.000           | 1.000           | 1.000           | 1.000           | 1.000          | 1.000          | 1.000          | 1.000           | 1.000             | 1.000            |                       |
|            | RMSE <sub>ie</sub>  | 0.0047          | 0.0171          | 0.0060          | 1600.0          | 0.0051         | 0.0158         | 0.0108         | 1600.0          | 0.0038            | 0.0018           |                       |
| - mail     | 62                  | (1.987, 0.002)  | (1.968, 0.006)  | (2.002, 0.002)  | (1.988, 0.003)  | (1.997, 0.002) | (1.975, 0.006) | (1.981, 0.004) | (2.001, 0.003)  | (2:001, 0:001)    | (2.000, 0.001)   |                       |
| 0          | 102                 | (1000,0001)     | (0.998, 0.005)  | (0.999, 0.002)  | (1.003, 0.026)  | (0.998, 0.002) | (0.097, 0.005) | (1.003, 0.003) | (0000,0000.1)   | (100.0,999,0.001) | (1001,0001)      |                       |
|            | •u2                 | (-0.546, 0.004) | 0.4104, 0.014)  | (-1.163, 0.006) | (0.058, 0.007)  | (-5.44, 0.004) | (0.409, 0.013) | 0.0574, 0.009) | (-0.692, 0.010) | (£00.0.809.9-)    | (1.625,0.0013)   |                       |
|            | R'.                 | 1.000           | 0001            | 1.000           | 1.000           | 0001           | 0007           | 0001           | 1.000           | 00001             | 0007             |                       |
|            | RMSE <sub>i</sub> , | 0.0028          | 0.0097          | 0.0030          | 0.0050          | 0.0023         | 0.010          | 0.0057         | 0.0050          | 0.0055            | 0.0004           |                       |
| hu         | c1                  | (100/0/166/1)   | (1.981, 0.004)  | (2.000, 0.001)  | (1.991, 0.002)  | (1.998, 0.002) | (1.981, 0.004) | (1.990, 0.002) | (1.999, 0.002)  | (2.000, 0.001)    | (1000)0.999.1    | he actual values      |
| -          | 14                  | (1000,0001)     | (0.999, 0.003)  | (100.0,0001)    | (1.000, 0.002)  | (100.0,000.0)  | (0.000,0003)   | (1.002, 0.002) | (1000,000.1)    | (1000,0001)       | 1.0000.00000     | the multiple to not 1 |
|            | 10+                 | (-0.629,0.002)  | (-0.056, 0.008) | (-1.163, 0.006) | (-0.427, 0.004) | -5.584, 0.002) | 0.0567 0.008)  | -0.431, 0.005) | -1.146, 0.005)  | -9.209, 0.001)    | 2.296, 0.0003) ( | the transformed to    |
| Pollutants |                     | As              | S               | 5               | 5               | Hg             | ī              | 4              | Zn (            | Naphthalene (     | Ester            | The values are b      |

$$C_{\text{reduct SV}} = exp\left[ln\left(\frac{\left(a\left[\frac{E^{st}}{R^{st}}\right]\right]^{2}}{\sqrt{\left(a\left[\frac{E^{st}}{R^{st}}\right]\right]^{2} + \left(a\left[\frac{E^{st}}{R^{st}}\right]\right]^{2}}\right)} + 1.645 \times \sqrt{ln\left(\frac{\left(a\left[\frac{E^{st}}{R^{st}}\right]\right]^{2} + \left(a\left[\frac{E^{st}}{R^{st}}\right]\right]^{2}}{\left(a\left[\frac{E^{st}}{R^{st}}\right]\right]^{2}}\right)}\right)$$



Equation 3.20 is the most general form of the predicted environmental concentration. The coefficients in the above equations are given in Tables 3.16 and 3.17 for water column and pore water concentration, respectively.

The empirical model developed for predicted environmental concentration estimates the concentration of various pollutants in the water column and pore water. The predicted environmental concentration was the adjusted to bioavailable fraction (BF) and probability of exposure (p) to estimate ecological and human health risk. These are determined in Chapters 4 and 5, respectively.

# Chapter 4

# ECOLOGICAL RISK ASSESSMENT

# 4.1. INTRODUCTION

There has been a growing concern to quantify the risk associated with contaminants in air, water, soil and food. These contaminants are also known as *risk agents or toxicants* or stressors. They enter the environment through anthropogenic activities including industrialization, improper waste disposal and spills, and contamination of water resources due to leachate from abandoned landfill sites. Until very recently, environmental research was focused on the identification of sources, movement and dispersion of pollution in the environmental media without giving much attention to how these pollutants are transmitted to human and ecological receptors.

An emerging field of research, environmental risk assessment uses an integrated approach to estimate risk quantitatively. The estimate is based on the knowledge of physicochemical characteristics of the risk agents and their toxicity profiles, routes and exposure pathways, acute (short-term exposure with high dose) and chronic (long-term exposure) receptor response. Such quantitative assessment can be carried out with good understanding of exposure and toxicity assessment of the pollutant release into the media. The U.S. EPA has developed risk assessment guidelines for carcinogenicity, mutagenicity, developmental toxicity, chemical mixtures, and exposure assessment, which have been published in the Federal Register (U.S. EPA, 1984; 1986a-e). These guidelines provide a framework to promote consistency in conducting various types of risk assessments, but do not discuss the type of modeling tools and specific field measurements that are the basis for quantitative risk assessment.

In this chapter, the concept of ecological risk assessment is introduced with the help of a step by step approach using the case of drilling waste discharges. This chapter deals with different components of risk assessment, but more emphasis is given to risk characterization. The exposure assessment is based on contaminant fate in multimedia and its exposure pathways to receptors. The exposure assessment conponent is an ouput of the contaminant fate modeling methodology which was developed in Chapter 3.

#### 4.2. FRAMEWORK OF ECOLOGICAL RISK ASSESSMENT

The term ecological risk assessment can be defined as "a process that evaluates the likelihood that adverse ecological effects may occur or are occurring as a result of exposure to one or more stressors" (e.g., U.S. EPA, 1998; CCME, 1997). The U.S. EPA (1998) has provided a comprehensive framework for ecological risk assessment as illustrated in Figure 4.1. This framework is followed to explain the essential elements of ecological risk assessment. As shown in Figure 4.1, there are four basic steps involved in ecological risk assessment. These steps are:



Figure 4.1. The ecological risk assessment framework (U.S. EPA, 1998)

- 1. Problem formulation
- 2. Analysis
- 3. Risk characterization
- 4. Risk management and communication

Problem formulation is used for data collection and hazard identification. It also deals with the assessment of endpoints (receptors) and the development of a plan for exposure assessment. The second step is the analysis phase, which is the key component of ecological risk assessment. The exposure and toxicity assessments for different ecological entities are the two products of the analysis phase. Once stressor response and exposure profiles are prepared, characterization of risk on short-term and long-term bases can be performed in the next step. Mitigation measures and communicating risk to stakeholders and interested groups are covered in the fourth and the final step of risk assessment. In this chapter the first three steps will be discussed in detail in the perspective of drilling waste discharges in the marine environment.

# 4.3. PROBLEM FORMULATION

Problem formulation is a process of producing and evaluating hypotheses about the occurrence of ecological effects from anthropogenic activities. It provides a foundation for the ecological risk assessment process. In problem formulation, objectives are defined, the nature of the problem is evaluated, and plans for data analysis and risk characterization are developed. Problem formulation comprises three components: (1) assessment endpoints. (2) conceptual models, and (3) analysis plan. Drilling waste discharges contain base fluid, heavy metals and some organic priority pollutants. The ecosystems have naturally occurring levels of these pollutants, however, additional amounts of these chemicals can stress the delicate balance necessary for the efficient performance and survival of an ecological community. The background level concentrations in the ecosystem are reported in Table 4.1. The mean water column concentrations are very low in comparison to mean sea level concentrations for all pollutants (see Chapter 3, Table 3.10). The estimated mean arsenic water column concentration is  $7.75 \times 10^{-3} \, \mu g/L$  for an impact radius (*R*) of 50 m and loading rate (*E*) of 3 g/hr. This is considerably below the mean sea background concentration of  $3.7 \times 10^{-2} \, \mu g/L$ . Similar conclusions can be drawn for other pollutants by comparing the predicted water column concentrations with background levels. The other exposure to the ecosystem is through pore water concentration, which is higher than background levels (see Chapter 3, Table 3.12).

| Table 4.1. The background concentration level of potential contaminants in drilling waste |
|---|
| (Aus-NZ WQG, 1999)  |
|   |

| Contaminants | Mean sea background concentrations (µg/L) |
|--------------|---|
| Cd           | 0.044                                     |
| Ni           | 0.400                                     |
| Cu           | 0.720                                     |
| Cr           | 0.079                                     |
| As           | < 0.037                                   |
| РЬ           | 0.480                                     |
| Hg           | 0.054                                     |
| Naphthalene  | 0.700                                     |

The potential drilling waste contaminants emitted to the marine ecosystem cause environmental damage and have been reported extensively in the literature, e.g., Lee and Nicol (1978), Neff (1979), Rice et al. (1977) and Reish et al. (1976, 1977, 1978, 1979, 1980, 1981, and 1982). Reish et al. (1976-1982) have periodically tabulated toxic effects of heavy metals on individual species. Heavy metals have adverse effects on many species in the marine ecosystem, namely polychaetes, bivalves, gastropods, crabs, algae, crustaceans and fish. For example, chromium (Cr) is toxic to bivalves, crustaceans and fish. The particular effects of heavy metals are inhibition of spawning and suppressed reproduction. The three basic components of problem formulation are explained with the help of examples in the following sections.

## 4.3.1. SELECTION OF ENDPOINTS

The selection of endpoints should adequately reflect management goals and represent the ecosystem where they live. The management goals are set to save an ecosystem for commercial, economical and recreational reasons. The selection of end points should also be based on ecological relevance and susceptibility to known potential stressors. Ecologically relevant endpoints reflect important characteristics of the system and are functionally related with other endpoints (U.S. EPA, 1992a). These endpoints may be identified at any level of organisation (e.g. individual, population, community, ecosystem and landscape). Ecological entities are not ecologically relevant unless they are currently, or were historically part of the ecosystem under consideration. The relevance is linked to the nature and intensity of potential effects and to the spatial and temporal scales where effects may occur. Marine organisms are susceptible to heavy metals and other pollutants present in the drilling waste if the concentration is above a certain level. The groups/organisms at the lower trophic levels in the food web are more susceptible. From a management and scientific viewpoint, selection of organisms at various trophic levels could be the nght choice for endpoints as they have both economical and ecological importance. Considering the various organisms in Table 4.2 as endpoints may provide better insight to hold the three basic criteria: management goal, susceptibility and ecological relevance. If the whole ecological community is defined as endpoints, it can meet the susceptibility and ecological relevance criteria because the most sensitive species are taken into account. Similarly, considering those ecological entities, which may have commercial value as endpoints (e.g., fish) fulfill the management goal criterion.

The selection of the foodweb as an endpoint is more realistic than defining a single species, or groups, as an endpoint (Husain et al., 2001). The acute and chronic toxicity data for different pollutants are reported in Table 4.2. The  $LC_{50}$  or  $EC_{50}$  (concentration at 50% adverse effect) are used for measuring short-term or acute toxic effects, which refers to lethal concentration at 50% kill or adverse effect, respectively. The NOEC (No observed effect concentration) is used to estimate long-term or chronic effects. The NOEC refers to the concentration level at which no adverse effects are expected. Reported data are collected from various sources, which include Reish et al. (1976, E2), Hall (2000), U.S. EPA (1976; 1986a-e), Atchison et al. (1977), Holcombe et al. (1976), Booman and Foyn (1996) and Australian and New Zealand water quality guidelines (Aus-NZ WQG, 1999).

| Toxicant | Organisms/Groups            | Tine<br>(hr) | LCsa<br>(ppm) | NOEC (ppb) | Toxicant | Organisms/Groups       | (hr) | LC so<br>(ppm) | NOEC<br>(ppb) |
|----------|-----------------------------|--------------|---------------|------------|----------|------------------------|------|----------------|---------------|
|          | Crustaceans                 | 96           | 9             |            |          | Pelecypod (sp. 1)      | 96   | 2.5            |               |
|          | Bivalves                    | 96           | 3.5           |            |          | Sea star (sp. 1)       | 96   | > 10           |               |
|          | Crustacean larvae           | 96           | 0.23          |            |          | Sea star (sp. 2)       | 96   | 7.1            |               |
|          | Algue (2 species)           | p 6 - 9      |               | 1000 -     |          | Amphipod (sp. 1)       | 5 d  | 0.2 - 0.4      |               |
| As       | Crustacean (3 species)      | 8-51d        | 0.89-70       |            |          | Amphipod (sp. 2)       | 96   | 0.32           |               |
|          | Echinoderms                 |              |               | 1500       |          | Decupod (sp. 1)        | 96   | 6.4 - 6.8      |               |
|          | Annelids                    |              | 3 - 7.5       |            |          | Decapod (sp. 2)        | 1d   | 14             |               |
|          | Molluses                    |              | 1.5 - 7.4     |            |          | Decapod (sp. 3)        | 96   | 1.3            |               |
|          | Pelecypod                   | 96           | 3.49          |            |          | Decapod larvae (sp. 4) | 96   | 0.49           |               |
|          | Fish                        |              | 3.4 - 83      |            | C.q      | Bivalves               | 96   | 1.6 - 2.5      |               |
|          | Polychaetes (sp. 1)         | 96           | 5.8 - 7.5     |            |          | Bivalves larvae        | 96   | 1.5            |               |
|          | Polychaetes (sp. 2)         | 96           | 12            |            |          | Gastropods             | 96   | 35             |               |
|          | Polychaetes (sp. 3)         | 7.4          | 6.4 - 9.3     |            |          | Crustaceans            | 96   | 0.015 -        |               |
| P        | Polychaetes (sp. 4)         | 96           | 2.5-5.0       |            |          | Crustacean larvae      | 96   | 0.25 - 3.8     |               |
|          | Polychaetes (sp. 5)         | 96           | 4.2           |            |          | Echinoderms            | 96   | 7.1 - 10       |               |
|          | Polychaete larvae (sp. 1)   | 96           | 0.22          |            |          | Fish (sp. 1)           | P1   | 16             |               |
|          | Polychaete juvenile (sp. 2) | 28 d         | 3             |            |          | Fish (sp. 2)           | 7.4  | 21             |               |
|          | Pelecypod (sp. 1)           | 96           | 1.48          |            |          | Fish embryo (sp. 3)    |      |                | < 0.5         |

Table 4.2. Toxicity data for various contaminants present in the drilling waste

| Toxicunt | Organisms/Oroups            | Time<br>(hr) | LC <sub>50</sub><br>(ppm) | NOEC<br>(ppb) | Toxicant | Organisms/Groups            | Time<br>(hr) | (ppm)    | NOEC<br>(ppb) |
|----------|-----------------------------|--------------|---------------------------|---------------|----------|-----------------------------|--------------|----------|---------------|
|          | Fish (sp. 4)                | 96           | 22                        |               |          | Polychuete juvenile (sp. 3) | 96           | 5        |               |
|          | Fish lawae                  | 96           | 0.60 - 14                 |               |          | Polychaete (sp. 4)          | 96           | 4.3      |               |
|          | Fish (6 species)            | P 61 - L     |                           | 108 - 16,000  |          | Polychaete (sp. 5)          | 96           | 2        |               |
|          | Crustacean (19 species)     |              |                           | 0.45 - 10400  |          | Polychaete (sp. 6)          | 96           | > 5.0    |               |
|          | Echinoderms (18 species)    | P 2 4        |                           | 011           |          | Pelecypod                   | 96           | 57       |               |
|          | Molluses (5 species)        |              |                           | 30 - 3200     |          | Gastropods                  | 96           | 105      |               |
|          | Annelids (5 species)        |              |                           | 7.8 - 1302    |          | Bivalves                    | 96           | 57       |               |
|          | Nematodes                   |              |                           | 400 - 1000    |          | Crustaceans                 | 96           | 3.4 - 45 |               |
| S        | Rotifers                    | 72           |                           | 1040          |          | Echinoderms                 | P.4          | 1.7      |               |
|          | Algue (2 species)           | 10-14 d      |                           | 5.7 - 1780    |          | Diatom                      | 72           | 2.4      |               |
|          | Isopods                     | 96           | 1.25                      |               |          | Macroalgae                  |              | 360      |               |
|          | Shrimp                      | 96           | 0.2 - 0.3                 |               | Cr       | Australian crab             |              | 0.3      |               |
|          | Hermit crub                 | 96           | 0.07                      |               |          | Tidepole fish               |              | 2.6      |               |
|          | Cupepod (sp. 1)             | 96           | 1.8                       |               |          | Fish (3 species)            | 14 - 21 d    |          | 776 -         |
|          | Copepod (sp. 2)             | 96           | 0.01                      |               |          | Crustucean (13 species)     | 7-20 d       |          | 4 - 3090      |
|          | Mysid                       | 96           | 0.015                     |               |          | Echinoderms                 | P 4          |          | 2000          |
|          | Polychaete (sp. 1)          | P 4          | 1.44 -                    |               |          | Molluses (3 species)        | 7 - 16 d     |          | 1600 - 10000  |
| 2        | Polychaete (sp. 2)          | 96           | 0.16-3.2                  |               |          | Sipunculid                  | 11 - 52 d    |          | 1995          |
| ;        | Polychaete juvenile (sp. 2) | 96           | > 1.0                     |               |          | Algue (7 species)           | P 2          |          | 4.8 -         |
|          | Polychaete (sp. 3)          | 96           | 0.3-8.0                   |               |          | Brittle star                | 72           | < 3.75   |               |

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|        |                           | Time | LC 30    | NOEC  |          |                         | Time      | LC w         | NOEC        |
|--------|---------------------------|------|----------|-------|----------|-------------------------|-----------|--------------|-------------|
| OXICAN | Organisms/Uroups          | (hr) | (mqq)    | (qdd) | TOXICALI | Organisms/uroups        | (hr)      | (uudd)       | (hph)       |
|        | Decapod                   | 96   | 30 - 45  |       |          | Bivalves larvae         | 48        | 0.0055       |             |
|        | Fish (sp. 1)              | 96   | 31.2     |       | _        | Gastropods              | 96        | 0.058        |             |
|        | Fish (sp. 2)              | 96   | 16       |       |          | Gastropods larvae       | 48        | 0.11         |             |
|        | Copepod                   | 96   | 4.5      |       |          | Crustaceans             | 96        | 0.1 - 250    |             |
| ŝ      | Decapod                   | 96   | 9        |       |          | Crustacean larvae       | 96        | 0.048 - 0.17 |             |
| 5      | Decapod (larvae)          | 96   | 0.17     |       |          | Fish larvae             | 96        | 0.028 - 0.13 |             |
|        | Sea star                  | 96   | 32       |       |          | Fish ( 6 species)       | 11-14d    |              | 30 - 260    |
|        | Oyster                    | 48   | 0.611    |       |          | Crustaceans (3 species) | 10 - 14 d |              | 1.7 - 42    |
|        | Mussels                   | 48   | 1.20     |       | _        | Molluscs (7 species)    | 5-30d     |              | 0.4 - 20MKO |
|        | Crabs                     | 96   | 0.247    |       | ð        | Annelids (3 species)    | Sd        |              | 17 - 1000   |
|        | Polychaete (sp. 1)        | 96   | 0.2      |       |          | Phytoplankton           |           |              | 0.3 - 10    |
|        | Polychaete (sp. 2)        | 96   | 0.3      |       |          | Hydroid                 |           |              | 0.05 - 14.3 |
|        | Polychaete (sp. 3)        | 96   | 0.2      |       |          | Dinoflagellate          |           |              | 20          |
|        | Polychaete larvae (sp. 1) | 96   | 0.18     |       |          | Macroalgae              |           |              | 10 - 50     |
|        | Polychaete larvae (sp. 2) | 96   | 0.3      |       | _        | Shrimp                  |           |              | 5000        |
| 5      | Amphipod (sp. 1)          | 96   | 1.25     |       |          | Echinoderms             |           |              | 0           |
|        | Amphipod (sp. 2)          | 96   | 1.45     |       |          | Oyster (embryos)        | 48        | 0.0053       |             |
|        | Amphipod (sp. 3)          | 96   | > 10     |       |          | Mussels (embryos)       | 48        | 0.0058       |             |
|        | Decapod                   | 96   | 250-1000 |       | _        | Crubs (larvae)          | 96        | 0.049        |             |
|        | Sea urchin                |      | 0.30     |       |          | Clams                   | 96        | 0.57         |             |

| Toxicant | Oreanisme/Grouns           | Time     | LC.0         | NOEC      | Toxicant | Oreanisms/Groups        | Time | LC.90          | NOEC       |
|----------|----------------------------|----------|--------------|-----------|----------|-------------------------|------|----------------|------------|
|          | of Printing of Contraction | (hr)     | (udd)        | (qdd)     |          | admonto annual de la    | (hr) | (mqq)          | (ddd)      |
|          | Fish                       | 5 - 32 d |              | 7.4 - 160 |          | Crustacean lurvae       | 96   | 0.0082 - 0.017 |            |
|          | Crustareaus (3 species)    | 7 - 35 d | 0.0018 -     | 0.8 - 10  |          | Fish (sp. 1)            | 96   | 0.8            |            |
|          | Echinoderms                | P.4      | 0.02         | +         |          | Fish (sp. 2)            | 96   |                | < 860      |
|          | Molluses (7 species)       | 5 - 12 d | 0.004 - 5.07 | 0.12 -    |          | Phytoplankton (sp. 1)   | 4    |                | 1.10       |
|          | Annelids (2 species)       | 7 - 28 d | 0.017 -      | 3.4 - 18  | a<br>B   | Phytoplankton (sp. 2)   |      |                | 2030       |
|          | Algae (7 species)          |          |              | 0.9 - 88  |          | Pelecypod               | 37   | 1.0            |            |
|          | Polychaete (sp. 1)         | 96       | 0.022        |           |          | Decapod (sp. 1)         | 48   | 0.01 - 0.0156  |            |
|          | Polychaete (sp. 2)         | 96       | < 0.1        |           |          | Decupod larvae (sp. 2)  | 96   | 0.017          |            |
| Hg       | Polychaete (sp. 3)         | 96       | 0.09         |           |          | Pelecypod (sp. 1)       | 96   | 0.4            |            |
|          | Polychaete (sp. 4)         | 96       | 0.07         |           |          | Pelecypod (sp. 2)       | 96   | 0.058 - 0.122  |            |
|          | Polychaete larvae (sp. 1)  | 96       | 0.1          |           |          | Sea star                | 96   | 0.06           |            |
|          | Polychaete larvae (sp. 2)  | 96       | 0.014        |           |          | Polychaetes             | 96   | 25             |            |
|          | Copepod                    | 96       | 0.23         |           |          | Bivulve                 | 96   | 1.2            |            |
|          | Decapod                    | 96       | 0.05         |           |          | Bivalve larvae          | 48   | 0.35 - 0.89    |            |
|          | Bivalves                   | 96       | 0.058 - 0.4  |           | ź        | Gastropods              | 96   | 72             |            |
|          | Bivalve larvae             | 48       | 0.00065      |           |          | Fish                    | 96   | 350            | 30KKK)     |
|          | Gastropods                 | 96       | 32           |           |          | ('rusturems (4 species) | 5-84 |                | 640 - 6000 |
|          | Crustaceans                | 96       | 0.05 - 0.23  |           |          | Echinoderms             | 7 d  |                | 2640)      |

| Toxicant | Organisms/Groups            | Time<br>(hr) | LC30<br>(ppm) | NOEC<br>(ppb)  | Toxicant | Organisms/Groups          | Time<br>(hr) | LC <sub>30</sub><br>(ppm) | NOEC (ppb) |
|----------|-----------------------------|--------------|---------------|----------------|----------|---------------------------|--------------|---------------------------|------------|
|          | Molluses (5 species)        | 7 - 12 d     |               | 240 - 450000   |          | Brine shrinp              |              | 0.085 - 0.25              | 25 - 180   |
|          | Annelid (2 species)         | P L          |               | 1540 -<br>5000 | £        | Fish (larvae)             |              | 0.065 - 0.1               | 10 - 45    |
| ž        | Pelecypod                   | 96           | 320           |                | _        | Mixed marine bacteria     |              | 0.08                      | 20         |
|          | Copepod                     | 96           | 6.0           |                |          | Fish                      | 7.4          |                           | 10400      |
|          | Decapod                     | 96           | 47            |                |          | Crustaceans (8 species)   | 8 - 28 d     |                           | 250        |
|          | Seu star                    | 96           | 150           |                |          | Echinoderms (8 species)   | 7.4          |                           | 460        |
|          | Polychaete (sp. 1)          | 96           | 6.8           |                |          | Molluses (5 species)      | 7 - 11 d     |                           | 15 - 27500 |
|          | Polychaete (sp. 2)          | 96           | > 10          |                |          | Annelids (5 species)      | p6-2         |                           | 70 -3260   |
|          | Polychaete juvenile (sp. 1) | 96           | 1.2           |                |          | Algue (3 species)         | 5-104        |                           | 13 - 796   |
|          | Polychaete juvenile (sp. 2) | 96           | > 7.5         |                |          | Polychaete (sp. 1)        | 96           | 3.5 - 10.7                |            |
|          | Bivalves                    | P.4          | 8.8           |                | 7.0      | Polychaete (sp. 2)        | 96           | 1.8                       |            |
|          | Bivalves larvae             | 96           | 2.5           |                | 117      | Polychaete (sp. 3)        | 96           | 5.5                       |            |
| đ        | Crustaceans                 | 96           | 0.58          | 29 . 51        |          | Polychaete (sp. 4)        | 96           | 1.7                       |            |
|          | Fish                        | 96           | 0.52 - 0.8    |                |          | Polychaete (sp. 5)        | 96           | 8.1                       |            |
|          | Molluscs                    | P 4          | 4.4 - 4.52    | 880 - 904      |          | Polychaete (sp. 6)        | 96           | 2.7                       |            |
|          | Annelids (2 species)        | 28 d         | 0.84 - 7.55   | ×              |          | Polychaete larvae (sp. 1) | 96           | 1.7                       |            |
|          | Algue ( 3 species)          | 10 - 14 d    | 3.11 - 7.94   | 8 - 27         |          | Polychaete larvae (sp. 2) | 96           | 0.0                       |            |
|          | Bacteria                    |              | 1.9           |                |          | Oyster (embryos)          | 48           | 0.119                     |            |

| (ddd)            |                   |                 |                 |                        |                 |             |                                 |                    |                    |                    |                     | 21                      |                       |                   |                   | 13.4                      |
|------------------|-------------------|-----------------|-----------------|------------------------|-----------------|-------------|---------------------------------|--------------------|--------------------|--------------------|---------------------|-------------------------|-----------------------|-------------------|-------------------|---------------------------|
| LC w<br>(ppm)    | 0.58              | 9.5 - 13.1      | =               | 1.23                   | 0.4             | 39          | 145,600                         | 87,000             | > 25,000           | > 32,000           | > 10,000            | 0.85 - 5.7              | 57                    | 3.8               |                   | PNEC value                |
| Time<br>(hr)     | 96                | 96              | 96              | 96                     | 96              | 96          |                                 |                    |                    |                    |                     | 48 - 96                 | 96                    | 96                |                   | reported                  |
| Organisms/Groups | Amphipod          | Decapod (sp. 1) | Decupod (sp. 2) | Decapod larvae (sp. 2) | Decapod (sp. 3) | Sea star    | Skeletonema costatum<br>(algae) | Crustacean (sp. 1) | Crustacean (sp. 2) | Crustacean (sp. 3) | Corophium volutator | Crustuceans (6 species) | Molluses              | Annelids          | 1.1000            | Narman et al. (1990) have |
| Toxicant         |                   |                 | 7.0             | 5                      |                 |             |                                 |                    | ENCL               |                    |                     |                         |                       | Naphthalene       |                   |                           |
| NOEC (ppb)       |                   |                 |                 |                        |                 |             |                                 |                    |                    |                    |                     |                         |                       |                   |                   |                           |
| LCsu<br>(ppm)    | 0.175             | 0.456           | 2.5 - 4.3       | 0.31                   | 50              | 0.4 - 13    | 0.18 - 1.2                      | > 10 - 39          | 6                  | 33.5- 40.5         | (9)                 | 1.45                    | 11.5 - 14.0           | 2.5 - 4.3         | 7.7               | > 10                      |
| Time<br>(hr)     | 48                | 96              | 96              | 96                     | 96              | 96          | 96                              | 96                 | 96                 | 96                 | 96                  | 96                      | 96                    | 96                | 96                | 96                        |
| Organisms/Groups | Mussels (embryos) | Crabs (larvae)  | Bivalves        | Bivalve larvae         | Gastropods      | Crustaceans | Crustacean larvae               | Echinoderms        | Fish (sp. 1)       | Fish (sp. 2)       | Fish (sp. 3)        | Copepod                 | Fish juvenile (sp. 1) | Pelecypod (sp. 1) | Pelecypod (sp. 2) | Sea star                  |
| Toxicant         |                   |                 |                 |                        |                 |             |                                 | Zu                 |                    |                    |                     |                         |                       |                   |                   |                           |

Experimental data can be combined to generate multiple point estimates that can be displayed as stressor-response relationships. If the response of interest is composed of many individual variables, multivariate techniques may be useful. In the case of multiple stressors, the stressor-response relationships are constructed separately and then they are overlapped. A classical stressor-response relationship is shown in Figure 4.2 in which mortality as a response is shown. The intensity of stressors (e.g. dose) is shown by toxicology terms like *NOEC* and *LC*<sub>30</sub>. More complex stressor-response relationships can be described using intensity, time, or space. For chemical stressors, the intensity (e.g., concentration) is the most familiar one. Exposure duration may also be used in chemical stressor-response relationships (Sadiq et al., 2001f)

The dosage for 10%. 50% or 90% kill can be easily calculated from the dose response relationships. The lowest observed effect concentration (*LOEC*) and *NOEC* can be derived with higher uncertainty from these relationships. Statistical hypotheses have been used with chronic toxicity tests of chemical stressors that evaluate multiple endpoints. For each endpoint, the objective is to determine the highest test level for which effects are not statistically different from the controls (*NOEC*) and the lowest level at which effects are statistically significant from the control (*LOEC*). The range between *NOEC* and *LOEC* is called the maximum acceptable toxicant concentration (*MATC*). The *MATC* is the geometric mean (GM) of *NOEC* and *LOEC*, which provides a useful reference to compare toxicities of various chemicals. These parameters are also derived based on uncertainty factors (UF) or by using extrapolation models.



Figure 4.2. Typical stressor-response relationship showing toxicity parameters

## 4.3.2. CONCEPTUAL MODELS

Conceptual models explain relationships between ecological entities and stressors. They describe exposure pathways, ecological effects and receptors. The complexity of the conceptual model depends on the complexity of the problem: the number of stressors, number of assessment endpoints, nature of effects and characteristics of the ecosystem. Conceptual models provide powerful learning tools and a framework for future applications for risk assessors and also help in modification as knowledge improves.

A set of risk hypotheses predicting relationships among stressors, and exposure and assessment endpoint response is first developed. The stressors in the drilling waste are As, Cd, Cr, Cu, Hg, Ni, Pb, Zn, naphthalene and ester. The assessment endpoint is the food chain of marine organisms depending on the availability of data (Table 4.2). It is assumed that the available assessment endpoint data are representative of the ecological entities in the Atlantic environment. The responses to these stressors are measured in terms of  $LC_{sp}$  and *NOEC* as given in Table 4.2. The endpoint toxicity data are tabulated at various exposure times. To use this data for further analysis, the following rules and assumptions are made which are illustrated with the help of a flowchart in Figure 4.3.

- If a range of LC<sub>50</sub> and EC<sub>50</sub> data is given, convert data into their geometric mean (GM = \Lowestx Highest ) using the lowest and highest values (French and French. 1989).
- 2. If exposure time for LC50 (or EC50) is missing, take it as 96 hours.
- 3. If a range of exposure times is given for LC 50 (or EC 50), use GM of exposure time.
- 4. Karman and Reerink (1998) have used a time dependence function of LC<sub>50</sub> for dynamic assessment of the ecological risk of the produced water discharged from oil and gas platforms. In current research the following relationship was used to convert (LC<sub>50</sub>), into (LC<sub>50</sub>), (Sprazue, 1969; Buikema et al., 1982; French and French, 1989).

$$(LC_{50})_4 = (LC_{50})_6 \left(\frac{t}{4}\right)^{0.0175}$$
(4.1)

where

 $(LC_{50})_t = LC_{50}$  at any time t

### $(LC_{50})_4 = LC_{50}$ at 96 hour exposure time

- Divide (LC<sub>50</sub>)<sub>1</sub> by an uncertainty factor (UF) of 100 to convert into predicted-noeffect-concentration (PNEC) as suggested by Thatcher et al. (1999).
- 6. If NOEC data are given, take it as PNEC.
- 7. If a range of NOEC is given, take GM value as PNEC.
- If NOEC reported exposure time is other than 96 hours, assume the same value for a 96-hour exposure time.
- If different values of LC<sub>50</sub> are given for a group (e.g., fish), assume that the values are for two different species.
- If NOEC and (LC<sub>50</sub>)<sub>4</sub> are both available, divide (LC<sub>50</sub>)<sub>4</sub> by 100 and take the smaller value as PNEC.

The PNEC values derived from the above rules are plotted in Figures 4.4 and 4.5 for potential pollutants. The empirical distribution function (EDF) of PNEC values is developed for each individual case. These values are assumed to represent any possible exposure pathways, from ingestion of contaminated seawater (bioconcentration) and consumption of the lower trophic level organisms (biomagnification). The PNEC values derived from the above rules are for responses, which include survival, inhibition and reduction in growth, inhibition in sexual reproduction, morphological changes, and reduced fertilization and others.



Figure 4.3. Methodology for deriving PNEC for a group/species



Figure 4.4. Empirical distribution functions of PNEC for As, Cd, Cr, Cu and Hg

Figures 4.4 and 4.5 show a wide variation in response of the aquatic community from  $10^{-3}$ to  $10^{7}$  µg/L for different risk agents. From the management goals and risk assessment points of view, the selection of a *PNEC* value which is representative of the whole community is a difficult task. Lenwood et al. (1998) has recommended an arbitrary value of the lowest  $10^{th}$  percentile on empirical distribution function (EDF). The accuracy of these estimates is limited, by data availability. Generally, statistical distributions are used to fit the EDF to account for those cases when data are limited like ester and naphthalene (Husain et al., 2001).



Figure 4.5. Empirical distribution functions of PNEC for Ni, Pb, Zn, ester and naphthalene

### 4.3.3. ANALYSIS PLAN

The analysis plan is the final stage in problem formulation. Risk hypotheses are evaluated to determine how they will be assessed using available data. As risk assessment becomes more unique and complex, the importance of a good analysis plan increases. This includes pathways and relationships identified in problem formulation, and will be pursued during the analysis phase.

## 4.4. ANALYSIS PHASE

The analysis phase encompasses the two components of risk assessment: characterization of exposures and corresponding effects on endpoints. The analysis phase connects problem formulation with risk characterization. The assessment endpoints and conceptual models developed during problem formulation provide the basic skeleton and structure for the analysis.

Uncertainty evaluation is also an integral part of the risk analysis procedure. The objective is always to describe, and where possible, quantify the errors and limitations associated with exposures and effects. Uncertainty analysis increases both the credibility and the utility of the assessment. Uncertainty characterized during the analysis phase is used during risk characterization and can influence the management decisions. Uncertainty characterization in risk estimation is the main theme of this research.

## 4.4.1. CHARACTERIZATION OF EXPOSURE

Characterising exposure describes the potential or actual contact of stressors with the endpoints. It is based on the measures of exposure and the ecosystem and also on characteristics of the endpoints. It analyses sources of pollution, distribution of contaminants and mode of contact between stressors and endpoints. At this stage, focus is directed to the identification of pollutant sources, the exposure pathway, the intensity and distribution of stressors spatially and temporally.

Source identification is the first and the most important component of exposure analysis. There are two types of sources. The first is the place from where the stressor originates, e.g., in case of drilling waste discharges the disposal facility outlet is the original source of stressors. The second source is the current location of stressors, e.g., the contaminants transported from the disposal location on the seabed could be the future potential source. In analyzing the exposure, the background level of the stressor should be measured first to evaluate the effects of a particular source. The location of the source and the environmental media that are first exposed to the stressor need special attention. The source characterization should also consider whether other constituents emitted by a source influence transport, transformation, or bioavailability of the stressor of interest. The primary source of stressors associated with drilling waste is the disposal outlet in the marine ecosystem. The steady state fate models consider not only the primary source, but in addition the continuous contributions from already deposited waste on the seabed, to determine pore water and water column concentration.

In the case of drilling waste, the heavy metals speciate into various oxidation states. These metal species can behave very differently in the marine environment. The toxicity profile of Cr (III) is different from Cr (VI). Similarly As and other heavy metals are also present in various forms in the marine system. The physico-chemical parameters like hardness, pH, temperature and presence of other compounds in the marine environment affect the speciation of metals (Aus-NZ WQG, 1999). In this study metal speciation is not taken into account.

Bioavailability is the extent to which a substance can be absorbed by living organisms and can cause an adverse physiological or toxicological response. For ecological risk assessment involving sediments, the definition includes the extent that the substance can desorb, dissolve or dissociate. The uptake of metals from sediments and water is a dynamic process that involves all levels of the food web (Bernhard, 2000). Organisms

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take bioavailable metal from sediments and water through external surfaces, ingestion of contaminated sediments and inhalation of vapor phase metals. In addition, organisms take bioavailable metals from their food. A fish can take up metal directly from environmental media through its gills, its skin or through sediment ingestion (Campbell et al., 1988).

In this research, the metal concentrations in sediment pore water are calculated using an aquivalence based approach. The pollutant bioavailable concentration is determined by dissolved and/or bioavailable fraction of the metals. The U.S. EPA (2000a) used leaching factors (*LF*) to determine the bioavailable fraction of heavy metals in the pore water for drilling waste discharges. The U.S. EPA (1996b) defined another term called conversion factor (*CF*) to determine the dissolved fraction in metals. Table 4.3 provides a summary for leaching factors and conversion factors for heavy metals. The predicted environmental concentration (*PEC*) in pore water is adjusted for bioavailable fraction by multiplying by a factor. For organic compounds (base fluid and naphthalene) the estimated concentration is be completely dissolved and biologically available.

The second objective of exposure analysis is to describe the spatial and temporal distribution of stressors in the environment. Stressor distribution in the environment is examined by evaluating pathways from the source as well as the formation and subsequent distribution of secondary stressors. The evaluation of chemical stressor distribution in the environment is related to partitioning in different media. Ecosystem characteristics influence the fate of all types of stressors. The stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution of the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution of the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution of the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution of the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution is the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution of the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution is the stressor distribution in the environment is calculated by monitoring or by using fate models or a combination of the stressor distribution is the stressor distribution is

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two. In this research, a probabilistic fugacity/aquivalence based model was used to predict the concentration in the pore water and water column.

| Heavy Metals | LF<br>(U.S. EPA, 2000a) | CF<br>(U.S. EPA, 1996b) |
|--------------|-------------------------|-------------------------|
| As           | 0.005                   | 1.000                   |
| Cd           | 0.110                   | 0.994                   |
| Cr           | 0.034                   | 0.993                   |
| Cu           | 0.0063                  | 0.830                   |
| Hg           | 0.018                   | 0.85                    |
| Ni           | 0.043                   | 0.990                   |
| РЪ           | 0.020                   | 0.951                   |
| Zn           | 0.0041                  | 0.946                   |

Table 4.3. Factors used to determine the bioavailable fraction in saltwater

The selection of the foodweb as an endpoint depicts a realistic picture of the aquatic environment where survival of organisms at higher trophic levels depends on the existence of sensitive organisms. The exposure of some of the sensitive organisms in the foodweb might lead to high risk of reduction of commercial aquatic life. The organisms do not necessarily stay close to the impact site and move within the whole area under study. Stansbury (1991) considered the migration rate of finfish and shellfish to determine the exposure probability (p) for characterization of ecological risk for dredged material disposal in the ocean. The U.S. EPA (2000a) calculated exposure probability by dividing the volume of the plume by the total volume in which fish are expected. A similar approach was used in this research to estimate the probability (p) of exposure. which is defined as the ratio of impact area to the total area of the region under consideration. Therefore the exposure concentration becomes:

$$EC = PEC \times p \times BF$$
 (4.2)

where,

EC = Exposure concentration: p = Probability of exposure:

BF = Bioavailable fraction; and

PEC = Predicted environmental concentration (calculated from equation 3.20)

Pore water concentrations are higher than water column concentration by orders of magnitude due to the hydrophobic nature of drilling waste. Therefore, the major ecological concerns related to drilling waste are benthic organisms, which are at a lower trophic level in the food web. Figure 4.6 describes a conceptual illustration of marine aquatic life living close to a drilling waste plume.

The third objective is to describe the contact between stressor and receptor. For chemicals, contact is quantified as the amount of the chemical ingested, inhaled, or material applied to the skin. Some stressors must not only be contacted but also must be internally absorbed. In that case uptake is evaluated by considering the amount of stressor internally adsorbed by an organism. The organisms have different capacities to absorb depending on the physico-chemical characteristics of the pollutants and the uptake processes within the body. In this research the exposure concentrations are assumed to be taken up by endpoints with 100% absorption efficiency for simplicity.



Figure 4.6. A conceptual model of marine foodweb and drilling waste plume

# 4.4.2. CHARACTERIZATION OF ECOLOGICAL EFFECTS

This step evaluates the relationship between stressor levels and ecological effects or response. Information on the probability that effects may occur as a result of exposure to that stressor, and on their linkages, is collected. The stressor-response relationships used in a particular assessment depend on the scope and nature of the ecological risk assessment as defined in the problem formulation stage. The shape of the stressorresponse curve is useful to determine the presence of effects for evaluating risk.

Stressor-response data collected from laboratory experiments are preferably fitted to the statistical models from which risk can be estimated for a selected dose. In practice, due to lack of data, the models like one-hit, multi-stage, multi-hit and Weibull are found to be useful in estimating risk. The details of the models can be found in Suter (1993, 1996) and Covello and Merkhofer (1993). Many empirical and process based approaches are also available for linking measures of effect to assessment endpoints. These approaches are used when an adequate database is available.

For characterisation of ecological effects, the CHARM model uses toxicity test results of algae, crustaceans and/or fish (Thatcher et al., 1999). Only chronic *NOEC* and acute  $EC_{50}$ or *LC*<sub>50</sub> are used, but *NOEC* results are preferred, if available. The CHARM model does not accept other chronic test results like *LOEC*,  $EC_{10}$  or *LC*<sub>10</sub>. The CHARM model also requires an exposure time of at least 96 hours.

In this research, characterisation of ecological response was based on PNEC data as shown in Figures 4.4 and 4.5. The EDFs were fitted to the statistical distributions using BEST-FIT (1993). The goodness of fit (GoF) of the model was determined by the Anderson-Darling GoF test, which described the best-fit distribution. The criterion for the fit of a distribution in the Anderson-Darling test used was A-statistics. The 5% and 10% critical A\* values were 0.752 and 1.035, respectively. If the calculated A\* was less than critical values, the data were distributed to that particular distribution for which the test was performed. The lognormal distribution was found to be the best distribution overall for response data (*PNEC*) among 21 different candidate distributions. Other distributions, which also fit to the data, were Pearson Type VI and Gamma distributions. Figure 4.7 shows a lognormal fit to the *PNEC* data for Zn.

Table 4.4 summarises the statistics for *PNEC* data for various pollutants. The mean  $(\mu_{\tau})$ and standard deviation  $(\sigma_{\tau})$  for lognormal fit are also provided. The A-statistics for response of all different contaminants except Hg is within the 5% critical value, but the

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lognormal fit has the lowest A-statistics value among the candidate distributions. Data for naphthalene and ester are limited, so any conclusion about the distribution choice is difficult. To make our case more generalised *PNEC* response distributions are assumed lognormally distributed for all contaminants.



Figure 4.7. Lognormal distribution fit to the PNEC empirical distribution (Zn)

An ecological objective can be defined in terms of events, where an event consists of three components: object, endpoint and level (Jooste, 2000). For example, more than 10% decrease in biodiversity may cause an irreversible change in the ecosystem. Therefore, an ecological objective in the event form can be defined as {foodweb biodiversity, reduction or damage, 10%).
| Toxicant    | Data<br>Points<br>(N) | Lognormal rank<br>among candidate<br>distributions | A.     | Mean (μg/L)<br>(μ <sub>s</sub> ) | Std. dev. (μg/L)<br>(σ <sub>ι</sub> ) |
|-------------|-----------------------|--|--------|----------------------------------|---------------------------------------|
| As          | 10                    | 1  | 0.3368 | 703.3                            | 4.873                                 |
| Cd          | 42                    | 2  | 0.3497 | 274.5                            | 2.904                                 |
| Cr          | 34                    | 1  | 0.4420 | 897                              | 9.458                                 |
| Cu          | 31                    | 1  | 0.5216 | 235                              | 10.210                                |
| Hg          | 29                    | 1  | 1.0950 | 59.6                             | 1.944                                 |
| Ni          | 16                    | 3  | 0.7519 | 8.866                            | 215.812                               |
| РЬ          | 15                    | 3  | 0.3968 | 53.2                             | 198                                   |
| Zn          | 37                    | 1  | 0.3550 | 358.1                            | 2.543                                 |
| Naphthalene | 4                     | 2  | 0.4775 | 143.5                            | 388.8                                 |
| Ester       | 5                     | 3  | 0.2294 | 623.861                          | 749.329                               |

Table 4.4. Statistics for lognormally distributed response

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A criterion for safety level was set at the lowest 10<sup>th</sup> percentile on response distributions to save 90% of the ecological community or to maintain biodiversity in the aquatic foodweb. 10<sup>th</sup> percentile response should be more than exposure concentration. To determine the 10<sup>th</sup> percentile level and its associated uncertainty, bootstrapping was performed on the *PNEC* derived data, as shown in Figures 4.5 and 4.6 (Sadiq et al., 2001d).

Bootstrapping relies on an analogy between the sample and the population from which the sample is drawn. Bootstrapping involves re-sampling the data with replacement many times in order to estimate the statistics of interest (Sadiq et al., 2001e). 1000 bootstrap samples (B) were performed for each contaminant PNEC data. For each bootstrap run, mean ( $\mu_i$ ) and standard deviation ( $\sigma_i$ ) were calculated assuming a lognormal distribution. The 10<sup>th</sup> percentile on lognormal distribution fit calculated for each bootstrap run with the help of a set of mean and standard deviation. The mean ( $\mu_{e0}$ ) and standard deviation ( $\sigma_{e0}$ ) of 10<sup>th</sup> percentiles were calculated from 1000 (*B*) bootstrap runs. The statistics of criteria *PNEC* value is derived to save the whole foodweb (Table 4.5). The macro was written in Minitab (1998) and given in Appendix-A1. The formulation of estimating means and standard deviation of *PNEC* criteria value is given in equation 4.3.

$$\begin{array}{c} Y_{i} = \log \left[ X_{i} \right] \\ \mu_{i} = mean \left[ Y_{i} \right] \\ \sigma_{i} = standard deviation \left[ Y_{i} \right] \\ X_{in} = e^{i \left( u_{i}^{-1/2BT \times \sigma_{j}^{-1}} \right)} \\ \mu_{in} \equiv \sum (X_{in}) B \\ \sigma_{in} \equiv \sum (X_{in} - \mu_{in})^{2} (B - 1) \end{array}$$

$$(4.3)$$

where

- $X_i = PNEC$  values derived from Table 4.2 and shown in Figures 4.5 and 4.6.
- $Y_i = \text{Log transformed PNEC values}$
- $\mu_r$  = Mean value of data by fitting lognormal distribution for each bootstrap run (B)
- $\sigma_r$  = Standard deviation of data by fitting lognormal distribution for each bootstrap run
- $X_{10} = 10^{th}$  percentile value on lognormal distribution for each bootstrap run
- B = No. of bootstrap runs (1000)
- N = No. of PNEC values for each data set (i = 4 42; see Table 4.4)
- $\mu_{10}$  = Mean of PNEC criteria value
- $\sigma_{10}$  = Standard deviation of PNEC criteria value

The PNEC criteria values were calculated and compared with the most stringent U.S. Federal water quality criteria (U.S. EPA, 2000a), CCME (1999) and Aus-NZ WQG (1999) used for manne water.

| Toxicant    | μ <sub>10</sub> | $\sigma_{l0}$ | U.S. (2000a)<br>FWQC | CCME<br>(1999) | Aus-NZ (1999)<br>WQG |
|-------------|-----------------|---------------|----------------------|----------------|----------------------|
| As          | 12.12           | 11.10         | 14.00                | 12.50          | 2.40                 |
| Cd          | 1.87            | 1.10          | 9.30                 | 0.12           | 1.70                 |
| РЬ          | 2.12            | 1.65          | 8.10                 | 7.00           | 0.80                 |
| Cr          | 5.77            | 2.62          | 50.00                | 1.50           | 3.10                 |
| Cu          | 0.20            | 0.12          | 2.40                 | 4.00           | 0.32                 |
| Hg          | 0.075           | 0.041         | 0.025                | 0.10           | 0.029                |
| Ni          | 25.82           | 38.80         | 8.20                 | 25.00          | 32.60                |
| Zn          | 4.24            | 1.80          | 81.00                | 30.00          | 2.70                 |
| Naphthalene | 13.04           | 36.08         |                      | 1.40           | 8.40                 |
| Ester       | 177.475         | 877.810       |                      |                |                      |

Table 4.5. Comparison of *PNEC* response criteria value parameters and other international standards (µg/L)

## 4.5. RISK CHARACTERIZATION

Risk characterization is the most important step in ecological risk assessment and involves planning and analysis of predicted adverse effects of assessment endpoints. Risk characterization develops the relationships between stressor and effects. The risk assessor uses the results of the analysis phase to develop an estimate of the risk posed to the ecological entities selected as assessment endpoints. After assessing the risk, the assessor describes the risk estimate in the context of the significance of adverse effects. The risk assessor also identifies the uncertainties and assumptions in the risk assessment. The risk characterization provides information for environmental decision making. Risk characterization includes risk estimation and risk description as discussed in the following sections.

#### 4.5.1. RISK ESTIMATION

Risk estimation is the process of integrating exposure and response data and evaluating any associated uncertainties. This process uses exposure and stressor response profiles according to an analysis plan. A general methodology for risk estimation is shown in Figure 4.8. Risk estimation can be done using one or more of the following techniques:

- 1. Field observational studies:
- 2. Categorical rankings:
- 3. Comparisons of single-point exposure and effects estimates:
- 4. Comparisons incorporating the entire stressor-response relationship:
- 5. Incorporation of variability in exposure and/or effects estimates, and
- Process models that rely partially or entirely on theoretical approximations of exposure and effects

#### **Field Observational Studies**

This technique provides empirical evidence which links exposure to response. Field surveys measure changes in the natural setting by collecting exposure and effects data for different ecological entities in the problem formulation phase. The main advantage of field surveys is that multiple stressors and very complex ecosystem relationships can be measured which are difficult to perform in laboratory studies (U.S. EPA, 1998; U.S. DOE, 1998). This technique should be used for validating modeling results. The field observational and monitoring studies are popular in offshore drilling waste disposal due to difficulties in modeling the complexities of transport and fate behavior of the drilling waste.



Figure 4.8. A general approach for risk estimation using exposure-response relationship

## **Categories and Rankings**

Qualitative evaluation techniques are performed through professional judgment, which are very popular in risk estimation. Risk can be categorized as low, medium, high or ranked on some numeric scale. This approach is adopted in case of limited data availability or when data can not be expressed in quantitative form. Ranking techniques translate qualitative judgment into mathematical forms. Fuzzy set theory is generally used to rank the risk from different stressors by considering the degree of immediate risk. duration of impacts, and prevention and remediation management. Lu and Hu (1999) have used the fuzzy approach to analyze eutrophication effects for water quality assessment in Taiwan.

#### Single-Point Exposures and Effects Comparisons

When sufficient data are available to estimate exposure and response, the most common technique is to take the ratio. Typically the ratio or quotient is expressed by <u>exposure concentration</u>. The advantage of the quotient method is that it is simple and

easy to understand. The quotient method is also used to integrate risks of multiple chemical stressors. It can be obtained by dividing each exposure level with corresponding toxicity endpoints ( $LC_{10}$ ,  $EC_{10}$ , NOEC, LOEC, MATC etc.) and generating quotients for the individual constituents in a mixture. The toxicity of a chemical mixture may be greater or less than predicted from the toxicities of individual constituents of the mixture. The quotient addition approach assumes that toxicities are additive or approximately additive. This assumption may be the most applicable when the modes of action of chemicals in a mixture are similar. There is evidence that even with chemicals having dissimilar action, additive or near-additive interactions are common (U.S. DOE, 1998; Jooste, 2000; McCarty and Mackay, 1993; Koneman, 1981; Broderius, 1991 etc.).

However the U.S. EPA (1998) has cautioned that assuming chemicals in a mixture act independently might cause erroneous conclusions. When the modes of action for constituent chemicals are unknown, the assumption and rationale should be clearly stated. In most cases, the quotient method does not explicitly consider uncertainty. Some uncertainties can be incorporated into single-point estimates to provide a statement of likelihood that the effects point estimate exceeds the exposure point estimate. A comparison of deterministic values of exposure concentrations and *PNEC* are shown in Figure 4.9a. The uncertainty in exposure and response estimates can also be incorporated (see Figures 4.9 b and c). If uncertainty is considered in exposure and response, the CDF of *HQ* can be established and the probability of exceeding 1, i.e.,  $p(HQ \ge 1)$  can also be calculated.

Mukhtasor et al. (2001) used a risk characterization approach by generating the CDF of response ( $LC_w$ ) of fish and crustaceans. They compared the CDF of response with the CDF of exposure to determine the uncertainty in risk estimates. Some attempts have also been made to take into account the uncertainty of the pollutant concentration in seawater (Karman and Reerink, 1998). In the CHARM model, the uncertainty is addressed very vaguely. The calculated hazard or risk quotient (HQ/RQ) is divided and multiplied by 3 to define the lower and upper 95% confidence levels, respectively (Thatcher et al., 1999).



Figure 4.9. Risk estimation techniques (U.S. EPA, 1998)

- (a) Comparison of point estimates:
- (b) Comparison of point estimate of a stressor-response relationship with uncertainty associated with an exposure point estimate; and
- (c) Comparison of point estimates with associated uncertainties

#### Comparisons Incorporating the Entire Stressor-Response Relationship

If a dose response curve is available, then risk can be determined at different levels of

exposure as shown in Figure 4.10. The advantage of this technique is that it considers the

whole range of stressor-response relationship for one species. In this method calculating uncertainty bounds on the stressor-response or exposure estimates can incorporate uncertainty which is lacking in the quotient method.



Concentration

Figure 4.10. Risk estimation considering variability in stressor-response relationship

### Comparisons Incorporating Variability in Exposure and/or Effects

If the exposure or stressor-response profiles are used to express the variability in exposure or effects, then many different risk estimates can be calculated. Variability in exposure can be used to estimate risks to moderately or highly exposed members of a population being investigated, while variability in effects can be used to estimate risks to average or sensitive population members. A major advantage of this approach is its ability to predict changes in the magnitude and likelihood of effects for different exposure scenarios and thus provide a means for comparing different risk management options. Comparing distributions also allows one to identify and quantify risks to different segments of the population (Figure 4.11). Husain et al. (2001) used a similar approach for estimating the risk of soot deposition in the Persian Gulf. Lenwood et al. (1998) and Solomon et al. (1996) used  $LC_{50}$  values to plot the CDF of response and compare with the CDF of exposure.



Figure 4.11. Risk estimation technique considering variability in exposure and response

## **Application of Process Models**

Process models integrate both exposure and effects information to estimate risk. During risk estimation, it is important that both the strengths and limitations of a process model approach be highlighted. Brody et al. (1989 and 1993) linked two process models to integrate exposure and effects information and forecast spatial and temporal changes in forest communities and their wildlife habitat value.

### 4.5.2. PROPOSED METHODOLOGY FOR RISK ESTIMATION

In the current research, risk was estimated by calculating a hazard/risk quotient. The hazard or risk quotient can be calculated by taking the ratio of exposure concentration (*EC*, equation 4.2, adjusted predicted environmental concentration from output of fugacity/aquivalence based models) and the 10<sup>th</sup> percentile on response CDF (*PNEC* criteria value, equation 4.3). The uncertainty in 10<sup>th</sup> percentile response level was estimated from Table 4.5.

$$HQ = \frac{Exposure \ concentration(\ adjusted \ 95'^{h} \ percentile, \ from \ regression \ mod \ el)}{10'^{h} \ percetile \ on \ PNEC \ log \ normal \ CDF}$$
(4.4)

The hazard quotient/risk quotient was calculated from the above equation. A coefficient value more than 1 shows the probability or risk of adverse effects. An empirical distribution function of HQ can be developed from equation 4.4, which shows uncertainty in hazard quotient estimates. The HQ critical value of 1 means that 90% of the ecological community is protected 95% of the time.

The quotient method is useful in explaining the relative risk but cannot estimate and quantify risk, which is important in management decisions. The CHARM model is used extensively in the North Sea as a tool for determining ecological risk of offshore discharges. The CHARM model determines HQ/RQ for a given compound and relates the estimate to a risk value by the following relationship:

$$Risk = \int_{y=0}^{lm(RQ)} \left\{ \frac{l}{S_m \sqrt{2\pi}} e^{\frac{(-\sqrt{2\pi})^2}{2\cdot S_m}} \right\}$$
(4.5)

where

 $X_m$  = Average of the lognormally transformed data = 2.85

 $S_m$  = Standard deviation of the lognormally transformed data =1.74

y = Variable to describe the normal probability curve

Karman et al. (1996) and Karman and Reerink (1998) used a similar relationship to convert the HQ/RQ into risk values for produced water discharges. The above relationship is calibrated to a given RQ value of 1 at an acceptable risk level of 5% (Thatcher et al., 1999). Karman et al. (1996) used 17 chemicals to calibrate this relationship. The organic chemicals include oil, benzene, phenol, toluene, xylene, ethylbenzene, methanol, naphthalene, cresol, and PCP; and heavy metals include Cr. Hg. Ni, Pb. Cd, Cu and Zn.

This calibration level is a political choice, which can be used as an acceptable risk. The mean ( $X_m = 2.85$ ) and standard deviation ( $S_m = 1.74$ ) values are adjusted to calibrate to a 5% acceptable risk. In this research the empirical distribution function of HQ calculated from equation 4.4 will be converted into risk estimates using equation 4.5. Figure 4.12 shows a proposed methodology for converting HQ into corresponding risk values for a single contaminant.

The CHARM model uses a chemical specific approach in which all chemicals are dealt with separately. The effect of each waste component is integrated by considering each chemical effect as independent of the others. The assumption of independent mode of action enables the use of statistical calculation rules for combining independent probabilities (Jooste, 2000). For example, for the mixture of three chemicals, total risk can be calculated by the following equations:





$$Risk (A + B) = Risk (A) + Risk (B) - Risk (A) \times Risk (B) \quad and \quad (4.6)$$

$$Risk (A + B + C) = Risk (A + B) + Risk (C) - Risk (A + B) \times Risk (C)$$

$$(4.7)$$

Similarly, the relationships for four and more pollutants can be developed.

For a waste which contains more than 4 to 5 chemicals, the synergistic and antagonistic effects are significantly negligible (Jooste, 2000). The total hazard is not the addition of individual HQRQ (Karman and Reerink, 1998). The HQ/RQ are first converted into their corresponding risk values and then total risk is estimated by considering the components to be statistically independent events (e.g., equations 4.6 and 4.7). The drilling waste is an amalgamation of toxic compounds. The HQ/RQ will be calculated for individual contaminants and then converted into the risk values (equations 4.4 and 4.5). The risk values are then grouped together to estimate the total risk posed to the ecosystem from drilling waste discharges.

## 4.5.3. RISK DESCRIPTION

The risk estimation results are interpreted in this phase. The risk assessor needs to comprehend the available information about risk estimates of endpoints. Risk description includes an evaluation of lines of evidences to support or refute the risk estimates and interpret the significance of adverse effects. The description of risk is an important step and the results of risk estimation are used in Chapter 6 for risk management in risk-cost tradeoff analysis.

## Chapter 5

# HUMAN HEALTH RISK ASSESSMENT

Offshore drilling operations may pose two types of human health risks: exposure of toxic chemicals to workers on offshore drilling rigs through inhalation, ingestion and skin contact: and exposure related to consumption of contaminated seafood due to discharge of drilling waste chemicals in the ocean environment. This chapter focuses on human health risk assessment, based on exposure to carcinogenic and non-carcinogenic contaminants through consumption of contaminated seafood. A description of adverse human health effects due to consumption of contaminated fish is provided in this chapter.

## 5.1. INTRODUCTION

The framework of human health risk assessment includes hazard identification, exposure and toxicity assessment, risk characterization and risk communication. A probabilistic approach is employed for risk characterization and estimation of uncertainty while developing a methodology for human health risk assessment.

An organization chart of this chapter is illustrated in Figure 5.1. A description of contaminants found in drilling waste and their toxicity profile are presented from an adverse human health effects point of view. Human health risk assessment can be estimated based on cancer and non-cancer models. The input parameters required in these models are discussed in detail. The risk characterization involves uncertainty in the estimates of risk due to input parameter uncertainties. The U.S. EPA (1999b) human health risk assessment (HHRA) approach for drilling waste discharges is also presented. A brief description of probabilistic and fuzzy based approaches is given in the context of HHRA. In the end, the proposed methodology for HHRA is presented.



Figure 5.1. Chapter organization for human health risk assessment

## 5.2. TOXICITY PROFILE OF RISK AGENTS

Human health risk assessment can be performed for those contaminants for which oral reference dose R/D (for non-carcinogenic effects) and slope factor SF (for carcinogenic

effects) have been established. Drilling waste contains heavy metals, priority pollutants and base fluid, as discussed in previous chapters. The oral reference dose and slope factor for any synthetic base fluid has not been established for exposure through ingestion. Therefore, traditional human health risk assessment methodology can not be applied to esters (U.S. EPA, 1999b).

The toxicity of various contaminants is established through laboratory studies on animals or sometimes on human tissues. The epidemiological data are another important source for estimating toxicity effects on humans. To establish and collect the evidence of toxicity effects on humans for a particular contaminant is very difficult because humans are exposed to various contaminants simultaneously. Laboratory studies are used when animals are exposed to one specific contaminant at higher dosage and then effects are extrapolated through mathematical techniques for humans. Different modes of action in various animals lead to questions of the variability of intra and inter species extrapolations. Epidemiological studies are considered the most authentic source to estimate adverse human health effects because extrapolations and uncertainty factors are not involved in the analysis. The collection of epidemiological data is very expensive and scarce. The U.S. EPA (2001) has developed a database. Integrated Risk Information System (IRIS), for various contaminants and has reported their human health effects through ingestion, inhalation and dermal contact, which are collected through peerreviewed sources. The contaminants are categorized based on the toxicity information available through animal or human studies as given in Table 5.1.

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|                       | Animal evidence |         |            |         |                          |  |
|-----------------------|-----------------|---------|------------|---------|--------------------------|--|
| Human evidence        | Sufficient      | Limited | Inadequate | No data | Evidence of<br>no effect |  |
| Sufficient            | A               | A       | A          | A       | A                        |  |
| Limited               | BI              | B1      | B1         | B1      | B1                       |  |
| Inadequate            | B2              | С       | D          | D       | D                        |  |
| No data               | B2              | С       | D          | D       | E                        |  |
| Evidence of no effect | B2              | С       | D          | D       | E                        |  |

Table 5.1. Categorization of contaminants for human health risk assessment

The contaminants under "A" category are those for which there is sufficient evidence of adverse health effects including human cancer and non-cancer ailments. The information collected from human epidemiological studies has more weight. If a contaminant has sufficient evidence of adverse health effects through epidemiological studies and even if there is no evidence from animal studies, the contaminant will still be categorized as "A". The group "B" (B1 and B2) is for limited and inadequate evidences (through human and animal studies) of adverse effects. The categories "C" and "D" are for those contaminants that lack sufficient evidence from human and/or animal studies. If there are no data and no evidence through animal and human studies the contaminant is categorized as "E".

Based on the information from *IRIS* the data for reference doses (R/D) and slope factor (*SF*) were collected for heavy metals and naphthalene (U.S. EPA, 2001). The arsenic (As) is the only risk agent that has sufficient evidence of cancerous effects in the list of drilling waste contaminants. It is categorized as "A" type of contaminant. The slope factor of arsenic is 1.5 (mg/kg-day)<sup>1</sup>. The human health cancer risk was studied for arsenic only. Similarly, the oral reference doses for copper (Cu) and lead (Pb) are not established yet. The non-cancer effects were studied only for those contaminants for which R/D is available. The non-cancer risk will be estimated for As, Cd, Cr, Hg, Ni, Zn and naphthalene. Table 5.2 summarizes the reference doses and slope factor of contaminants of concern.

#### 5.3. HUMAN HEALTH RISK ASSESSMENT

The HHRA starts with hazard identification and ends at risk communication. LaGrega et al. (1994) have classified quantitative human health risk assessment into the following five components:

- 1. Hazard identification:
- 2. Exposure assessment:
- 3. Toxicity assessment:
- 4. Risk characterization; and
- 5. Risk communication

## 5.3.1. HAZARD IDENTIFICATION

Human health risk assessment generally deals simultaneously with a number of risk agents, which are emitted from a contaminated source. The hazard identification examines the data of all contaminants detected from the source and emphasizes the chemicals that are of concern. Risk assessment requires knowledge of the source of contamination, concentration of contaminants and transport mechanisms, i.e., how they will reach the receptor. Naphthalene and heavy metals including As, Cd, Cr, Hg, Ni and Zn are identified as the risk agents that may cause adverse human health effects through an oral route.

## 5.3.2. EXPOSURE ASSESSMENT

The exposure assessment is the second stage in quantitative human health risk assessment. It estimates the contaminant exposure to the population at risk. To understand the source of contamination, exposure assessment begins with delineation of the sources (e.g., contaminated sediment pore water and/or water column near the drilling waste disposal site) and their spatial distribution (impact area, A,). When the chemicals are released it is necessary to understand how they migrate to a potential receptor. The exposure assessment encompasses various steps, which include identifying exposure pathway, process mechanisms, exposed populations and scenarios of exposure point and receptor dosage.

#### **Environmental Pathway**

To identify exposure pathways, the following elements are studied:

- 1. Source (e.g., disposal site);
- 2. Chemical release mechanism (e.g., offshore disposal of drilling waste);
- 3. Transport mechanism (e.g., advection, deposition, resuspension at the disposal site):
- 4. Transfer mechanism (e.g., adsorption to sediments, leaching into pore water);
- 5. Transformation mechanism (e.g., biodegradation);
- 6. Exposure point (e.g., contaminated fish):
- 7. Receptor (e.g., general public); and

Table 5.2. Toxicity profile of drilling waste contaminants from human health risk point of view

| SF<br>(mg/kg-day)'          | gan weights            | superoxide concentration in adult<br>of Zn exposure  | Decreased mean terminal body weight in males   |  |
|-----------------------------|------------------------|--|--|--|
| Critical effects            | Decreased hody and org | Decrease in crythrocyte<br>females after 10 weeks  |  |  |
| RfD<br>(mg/kg-day)          | 2E-2                   | 3E-1   | 2E-2   |  |
| Carcinogenic classification |                        | D<br>Not classifiable us to human carcinogen due to lack<br>of evidence in human and animals | C<br>Possible human carcinogen. Inadequate data of<br>carcinogenicity in humans, and the limited evidence<br>of carcinogenicity in animuls |  |
| Risk agents                 | ïŻ                     | Zu   | Vaphthalene  |  |

Table 5.2. Toxicity profile of drilling waste contaminants from human health risk point of view (Contd.)

8. Exposure route ( e.g., ingestion of contaminated fish)

The above steps are integrated to determine the environmental exposure pathway of drilling waste contaminants to reach the general public. A conceptual model for human exposure through contaminated fish is given in Figure 5.2.



Figure 5.2. Exposure pathway through ingestion of contaminated fish - a conceptual model

## Process Mechanisms

The release of drilling waste from a drilling rig is through a disposal outlet. Different processes are involved in transport, transfer and transformation of contaminants in the marine environment. The transport mechanisms may include advection, deposition, and resuspension of drilling waste. The transfer mechanisms include adsorption of chemicals to sediments and biota, and leaching of chemicals back into the water column and pore water.

#### Potentially Exposed Population

The potentially exposed population could be a community living close to the affected site (fishermen), future population, special population (e.g., infants) or general public. In this study the general public was assumed to be the potentially exposed population.

## Exposure Scenarios

When the exposed population and exposure pathways are identified through a conceptual model, the exposure estimates are made through various discharge scenarios. These discharge scenarios may include 10%, 8.5%, 7%, 5.5% and 4% attached base fluid discharge conditions of drilling waste.

## Exposure Point Concentrations

The concentrations of contaminants are estimated at the exposure point or location (through exposure pathway). The monitoring values, e.g., bioassays of fish found at the contaminated site may be the best estimates. In this study, the modeling results will determine the exposure point concentrations. The predicted environmental concentration (*PEC*) adjusted for bioavailable fraction (*BF*) and exposure probability (p) can be used as the exposure point concentration in the water column and pore water, which is assumed uniformly distributed over the whole area under study.

#### Receptor Doses

The final step in the exposure assessment is to estimate the dose of chemicals to which the receptors are actually exposed. The Guidelines for Exposure Assessment (U.S. EPA, 1992b) has defined the oral route for ingestion of contaminated food in Figure 5.3.



Figure 5.3. A schematic of dose and exposure (modified after U.S. EPA, 1992b)

In the current study the potential dose to the receptor was used for risk and potential effect estimates. The calculation for receptor dose was based on the following relationship.

$$CDI = \frac{(IR \times C \times EF \times ED)}{(BW \times AT)}$$

where

CDI = Chronic daily intake (mg/kg-day);

IR = Intake rate (mg/day);

C = Pollutant concentration (mg/kg of fish);

BW = Body weight (kg);

AT = Averaging time (days);

EF = Exposure frequency (day/yr); and

ED = Exposure duration (yr)

(5.1)

The above equation determines the average daily dose of a chemical or a risk agent, per unit body weight, that a potential receptor may be exposed to over its life time (Sadiq et al., 20001e).

#### 5.3.3. TOXICITY ASSESSMENT

The toxicity assessment uses dose response relationships to estimate the adverse health effects. The output of the assessment is used in risk calculations. The chemicals are generally categorized as potential carcinogens and non-carcinogens. The carcinogens receive more public attention generally. The mathematical relationships and computational methods of carcinogenic and non-carcinogenic effects are different. The dose response relationships for carcinogens are reported in the form of slope factors. It is a 95% upper confidence limit of the cancer on the dose-response curve and is expressed in (mg/kg-day)<sup>11</sup>. The non-carcinogens are expressed by their threshold values, reference dose (*R*(*D*). It is a daily dose, which is believed not to cause any adverse effect. It plays the same role as *PNEC* criteria value in ecological risk assessment (Chapter 4).

## 5.3.4. RISK CHARACTERIZATION

The fourth and the most important step in the human health risk assessment is the characterization of risk. The risk characterization includes quantitative estimates of both carcinogenic and non-carcinogenic risks. Risks are generally categorized for different exposure routes including ingestion, inhalation and dermal contact. In this study, the oral ingestion route is the only exposure pathway for which risks can be characterized.

### Cancer Risk Model

Carcinogenic risk is a product of the chronic daily intake (CDI) and the carcinogenic slope factor (SF). This risk is the excess lifetime cancer risk from exposure to any particular chemical. The traditional methodology for risk assessment assumes additivity of risks from individual toxicants. Total carcinogenic risk is the summation of all exposure routes and for all contaminants. The excess lifetime risk can be calculated as

$$R_{i} = Risk = CDI \times SF$$
 (5.2)

where.

CDI = Chronic daily intake (mg/kg-day); and

SF = Slope factor (mg/kg-day)<sup>-1</sup>.

The conventional approach for estimating risks using input variables either using

- 1. mean values only;
- regulatory defaults, in combination with mean values for parameters that are uncertain and for inter-individual variability, upper bounds (95<sup>th</sup> percentile) or with 5<sup>th</sup> percentile values when parameters are in denominator; and
- 3. upper bounds exclusively for all parameters.

When the mean values of input parameters are used it is called a "best estimate" ( $\hat{R}_{\epsilon}$ ). When the 95<sup>th</sup> (and/or 5<sup>th</sup> %) values are used in risk assessment, it is called an estimate of "reasonably maximum exposed" person ( $\hat{R}_{eNE}$ ), and the third case corresponds to an upper conservative bound. ( $R_{Men}$ ). The reasonably maximum exposed person estimate.  $\hat{R}_{Mel}$  is considered a conservative estimate of risk (i.e. above average) but not a worst-nossible case (U.S. EPA, 1989, 1991; U.S. DOE, 1999).

#### Non-cancer Risk Model

Non-carcinogenic risk is generally characterized in terms of a *hazard index*. The index is the ratio of estimated chronic daily intake (*CDI*) to safe reference dose (*RfD*) and is estimated by the following relationship.

$$HI = \frac{CDI}{RfD}$$
(5.3)

where.

CDI = Chronic daily intake (mg/kg-day):

RfD = Reference dose (mg/kg-day); and

HI = Hazard index

If the acceptable level of intake is equal to the reference dose. *HI* will be 1. An exposure to chemicals is typical for multiple non-carcinogens. The hazard index scores for all contaminants are summed to provide the final measure of the risk for non-carcinogenic toxic effects. The target is that the summation of all indices should be less than 1 for the safe dose. If the mechanisms of toxic action of different non-carcinogens are well known, it is preferable to sum the hazard indices on an organ specific basis, e.g., if one contaminant is affecting the brain and another is affecting the kidneys, their effects are not generally summed up to get a total *HI*. However in this study, to simplify the problem the *HI* was grouped on the basis of number of exposure pathways and number of risk agents.

$$HI_{i} = \sum_{j=1}^{n} \sum_{i=1}^{n} HI \qquad (5.4)$$

where

i to m = number of exposure pathways:

j to n = number of risk agents; and

HI, = Composite HI

Risk can be obtained from *HI* as a non-dimensional probability of the hazard index exceeding unity (Maxwell and Kastenberg. 1999). The risk to an individual can be expressed in terms of probability of exceedence as given below:

$$Risk = p(HI_r \ge 1) \tag{5.5}$$

#### **Background Risk**

When the risks are characterized from exposure involving contaminants, an important consideration could be whether to include or exclude the background levels of the contaminants. The background level risks are important in deciding what maximum level of contamination is acceptable to protect the aquatic life. For drilling waste the background level concentration in the seawater of the heavy metals is more than the concentration in the water column due to the waste. But in comparison to pore water concentration the background levels are too low to cause a significant change in risk estimates.

#### **Iterative Process in Risk Estimation**

Risk assessment is an iterative process. The steps involved in the risk assessment process need assumptions due to unavailability of data and missing information. Therefore the sensitivities in the magnitude of the calculated risk should be quantified with other possible assumptions as well.

#### Uncertainties in Risk Estimation

For quantitative risk assessment, the uncertainty is an integral part of the calculations, but generally when risk estimates are reported, associated uncertainty is not reported. The quantitative methods used in different steps of human health risk assessment introduce a high degree of uncertainty and might raise questions as to the validity of the results.

Exposure assessment on future conditions is based on fate and transport modeling tools. The model structure and input variables may introduce uncertainty into the predictions. The toxicity assessment stage is a source of the highest uncertainty in the quantitative human health risk assessment, which is associated with slope factors and reference doses. For most of the contaminants, these values are not available, but even where data exist, extrapolation from animal tests is performed from values as high as 10-90% to 0.0001% risk, through models. These extrapolations impair the accuracy of values. The computation of risk is the final stage, which is the measure of probability of rare events. It is not possible to calculate every conceivable outcome. To entertain various types of uncertainties, most of the time worst-case exposure scenarios are used for human health risk assessment. This brings inherent conservatism into calculations and could possibly include events that will never be experienced (LaGrega et al., 1994).

## 5.3.5. RISK COMMUNICATION

After charactenzing the risk of exposure to contaminants and their associated uncertainties, this information is used in decision-making. This generally involves the public or at least public concerns and their response. Risk communications deal with identifying the "level of acceptability of risk" and "risk perception". The U.S. EPA has defined acceptable risk as one in a million (10<sup>th</sup>) to 1 in ten thousand (10<sup>th</sup>) for cancer and HI less than 1 for non-cancer risk. Risk perception means the acceptability and understanding of risk to the general public. For the drilling waste case, the contamination of fish may not be very high, but it is the involuntary risk to which people are exposed that is important. Therefore its quantification becomes an important issue.

## 5.4. U.S. EPA STUDY - HUMAN HEALTH RISK ASSESSMENT

The U.S. EPA (1999b) has estimated human health risk associated with consumption of finfish and shrimps for various scenarios of discharge. The analysis was performed for those contaminants for which bioconcentration factors (*BCFs*), oral reference dose (*RJD*) and/or oral slope factors (*SF*) for carcinogenic risk have been established. This analysis considered barite and formation oil as the source of contamination but did not consider synthetic base fluid itself due to unavailability of *RIDs* and *SFs* data (U.S. EPA, 1999b).

The U.S. EPA (1999b) used the results of a study by Brandsma (1996) to predict the effective concentration to which finfish are exposed in the water column. Instead of predicting concentration at 100 m (regulatory boundary for mixing), the environmental concentration was predicted at various distances within the mixing zone. The arithmetic mean of the environmental concentration was used further in the human health risk assessment. The mean predicted environmental concentration was divided by the number of dilutions (*DF*) available within 100 m, to calculate the effective exposure concentration for finfish. The effective environmental pollutant concentration (mg/L) was multiplied by the bioconcentration factor (*BCF*) of contaminants to estimate the concentration in finfish tissues. The estimated tissue concentration was then used as the human exposure concentration for calculating *HI* and risk. The following equations were used to estimate the contaminant concentration in fish tissues:

Mean environmental concentration = 
$$C_{mean} = \frac{C_{tl} + C_{t2} + \dots + C_{tm}}{n}$$
 (5.6)

Effective finfish exposure concentration = 
$$C_{the} = C_{mean} \times DF$$
 (5.7)

Human exposure concentrations = 
$$C_{max} = C_{ff} \times BCF$$
 (5.8)

The following values for input parameters were used in the human health risk assessment:

FIR = Fish intake rate (99th percentile in California) = 177 (g/day);

Causer (mg/kg of fish)= Human exposure concentration:

BW = Body weight = 70 kg:

AT (days) = Averaging lifetime exposure =  $70 \times 365$ :

EF (days/yr) = Exposure frequency = 365; and

ED (yr) = Exposure duration = 30 and 70 (for two different exposure scenarios)

To calculate the pore water exposure concentration, the U.S. EPA (1999b) relied on a regression model based on a seabed survey, and developed a relationship of exposure concentration (mg/kg of sediment) as a function of distance. The average exposure concentration was then converted into shrimp tissue concentration by multiplying the average exposure concentration by the leaching factor (*LF*), *BCF* and lipid content (*Lc*) of the shrimps and then adjusted for density. In this analysis the deterministic input variables are used to estimate cancer and non-cancer risks.

In another study by the U.S. Department of Energy (U.S. DOE), Meinhold et al. (1996) estimated the human health risks in Louisiana Bay due to produced water discharges. Uncertainty analyses using Monte Carlo (MC) simulations were performed. Exposure and response parameters were defined with appropriate probability density functions. Radium (Ra) and lead (Pb) adverse effects were studied and concentrations in commercial fish were calculated based on their bioaccumulation potential. The input variables including dilution factor (DF), bioaccumulation coefficient ( $K_{ov}$ ), and fish ingestion rates (*FIR*) were defined by uncertain input parameters. The incorporation of uncertainty in input parameters determines the uncertainty in human cancer and noncancer risk estimation.

## 5.5. HUMAN HEALTH RISK ASSESSMENT PARAMETERS

The input parameters used in the human health risk assessment include exposure concentration, bioconcentration factor, lipid content of fish, intake rate, exposure frequency and duration, average body weight and average exposure time. The pore water exposure concentration is calculated from fate modeling (Chapter 3) which is then converted into fish tissue concentration by the following relationship:

Fish tissue concentration = Exposure concentration × BCF × Lipid content (Lc) (5.9) (mg/kg of fish) (mg/L) (L/kg of fish fat) (kg of fish fat/kg of fish)

The fish tissue concentration is then used in equation 5.1 to calculate the chronic daily intake (CDI). The details of risk assessment input parameters are given in the subsequent sections.

### 5.5.1. BIOCONCENTRATION FACTOR (BCF)

The bioconcentration factor is the ratio of concentration in organism tissue (mg/kg) to the concentration in water (mg/L). Bioconcentration factor (BCF) values are selected from empirical field and/or laboratory data generated from peer reviewed studies that are published in the scientific literature. If two or more *BCF* values are available, or two or more sets of collocated data are available, the geometric mean is used (U.S. EPA. 1999d).

Generally *BCF* values estimated from field studies are given more weight and considered more indicative of the natural environment than the laboratory studies. These field studies values are defined as 'recommended *BCF* values'. Sometimes laboratory-derived values are recommended when confidence in reported field values is less. When an appropriate surrogate is not identified, regression equation based on chemical *LogK*<sub>ow</sub> is used to calculate the 'recommended *BCF* values'.

The *BCF* values for various heavy metals are reported in Table 5.3. The values are collected from various key sources including Thompson et al. (1972), Saiki et al. (1995), and U.S. EPA (1992c, 1999b). The *BCF* values are reported for different species of finfish and shellfish in seawater and freshwater. The reported values for different fish species are assumed to be valid for the marine environment under study.

## 5.5.2. FISH INGESTION RATE (FIR)

Contaminated fish is a potential source of human exposure to toxic chemicals. Pollutants enter into surface waters and accumulate in sediments and biota due to their affinity towards organic matter through complex physico-chemical processes. To estimate the human exposure of toxic chemicals, fish ingestion rates (*FIR*) are determined for a potential exposed population. Commercially caught fish are marketed widely and exposure to the general public is difficult to estimate (U. S. EPA, 1997). The U.S. EPA (1999b) used a conservative approach and assumed *FIR* as the highest 99<sup>th</sup> percentile value (i.e., 177 g/day) from human consumption data in California.

|               | Deferrer                           | BCF (L/kg)  |             |  |
|---------------|------------------------------------|---|-------------|--|
| Reference     |                                    | Values  | Recommended |  |
|               | Thompson et al. (1972)             | 333<br>100  | 114         |  |
| As            | U.S. EPA (1992c); U.S. EPA (1999b) | BC.           Values           333           100           44           558           1295           729           1286           716           480           161           51           333           4.4           3000           200           4100           64           1.27           1.34           200           400           64           1.27           1.34           200           400           64           1.27           1.34           200           400           64           1.27           1.34           200           400           16           1800           4380 |             |  |
| Cd            | Saiki et al. (1995)                | 558<br>1295<br>729<br>1286  |             |  |
|               | Benoit et al. (1976)               | 716   |             |  |
|               | Eisler et al. (1972)               | 480   |             |  |
|               | Harrison and Klaverkamp (1989)     | 161<br>51   |             |  |
|               | Kumada et al. (1980)               | 33  | 907<br>     |  |
|               | Kumada et al. (1973)               | 8<br>3333   |             |  |
|               | Spehar (1976)                      | 4.4   |             |  |
| -             | Thompson et al. (1972)             | 3000<br>200   |             |  |
|               | Williams and Giesy (1979)          | 4100  | -           |  |
|               | U.S. EPA (1999b)                   | 64  |             |  |
| Cr<br>(Total) | Williams and Giesy (1979)          | 1.27<br>1.34  |             |  |
|               | Thompson et al. (1972)             | 200<br>400  | 19          |  |
|               | U.S. EPA (1999b)                   | 16  |             |  |
| Hg            | Boudou and Ribeyre (1984)          | 1800  | 3530        |  |
|               | Snarski and Olson (1982)           | 4380<br>5580  |             |  |
|               |                                    |   |             |  |

Table 5.3. Water-to-fish bioconcentration factor (BCF) values
| Metal     | D.C.                               | BC                          | F (L/kg)    |
|-----------|------------------------------------|-----------------------------|-------------|
|           | Reference                          | Values                      | Recommended |
| Ni        | Thompson et al. (1972)             | 100<br>100                  | 78          |
|           | U.S. EPA (1992c); U.S. EPA (1999b) | 47                          |             |
|           | Saiki et al. (1995)                | 2299<br>2265<br>4290<br>804 |             |
| Zn        | Deutch et al. (1980)               | 50<br>130<br>130<br>200     | 2059        |
|           | Pentreath (1973)                   | 373<br>8853                 |             |
|           | Thompson et al. (1972)             | 1000<br>2000<br>2000        |             |
|           | U.S. EPA (1992c)                   | 47                          |             |
| nhthelene | Howard (1989)                      | 40 - 1000                   | 426         |
| phthalene | U.S. EPA (1999b)                   | 426                         | 420         |

## Table 5.3. Water-to-fish bioconcentration factor (BCF) values (Contd.)

Meinhold et al. (1996) have derived ingestion rates from reported data on a meals per week basis for recreational fishermen at Louisiana Bay. They have reported the ingestion rates for two types of population including (i) recreational fishermen and their families and (ii) children. Table 5.4 summarizes the values used in this study.

The U.S. EPA's exposure factor handbook (1997) reports the fish ingestion rates for various types of fish for various exposed populations. Survey data on fish consumption were collected using different approaches, which included telephone and mail surveys, personal interviews and diary and creel surveys. Table 5.5 summarizes the results of *FIR* values for the general public. The results of 50<sup>th</sup> and 95<sup>th</sup> percentile *FIR* values with 90<sup>th</sup> bootstrap intervals (*BI*) are listed. For the mean value instead of *BI*, the confidence interval (*CI*) is reported. Marine fish ingestion rates are reported either as *uncooked* or as consumed. The "uncooked" values are reported in g/day and "as consumed" in mg/kg-day. The 95<sup>th</sup> percentile of "as consumed" (3.888 mg/kg-day) with a 90<sup>th</sup> percentile interval of 3.771 - 4.113 mg/kg-day are used in this analysis. The advantage of using fish mass ingested per unit body weight is that body weight (*BW*) and fish ingestion rate (*FIR*) may be correlated. If these parameters are used as independent variables in the analysis, this might lead to erroneous risk estimations. In the absence of a reliable correlation coefficient, it is preferred to use fish dosage per unit body weight as a single parameter in human health risk assessment.

Table 5.4. Fish intake rate caught at open bay platform

| Statistics                  | Recreational fishermen and families<br>(g/day) | Children<br>(g/day)<br>16.6 |  |
|-----------------------------|--|-----------------------------|--|
| Arithmetic mean             | 38.4   |                             |  |
| Median                      | 31.5   | 13.6                        |  |
| Standard deviation          | 26.4   | 11.6                        |  |
| Minimum                     | 3.3  | 1.3                         |  |
| Maximum                     | 228.6  | 115.7                       |  |
| 95 <sup>th</sup> percentile | 89.5   | 38.5                        |  |

| (modified | 1 after | Meinhol | d et a | 1., 1996) |
|-----------|---------|---------|--------|-----------|
|-----------|---------|---------|--------|-----------|

| Category/Comments              | Estimate        | Values | Interval            | Values        |
|--------------------------------|-----------------|--------|---------------------|---------------|
| Marine (Finfish and shellfish) | 50th percentile | 93     | 90 <sup>th</sup> BI | 108 - 118     |
| (g/day - uncooked)             | 95th percentile | 272    | 90 <sup>th</sup> B1 | 261 - 280     |
| Marine (Finfish and shellfish) | 50th percentile | 1.927  | 90 <sup>th</sup> B1 | 1.830 - 2.024 |
| (mg/kg·day - uncooked)         | 95th percentile | 5.019  | 90 <sup>th</sup> BI | 4.852 - 5.267 |
| Marine (Finfish and shellfish) | 50th percentile | 88     | 90 <sup>th</sup> B1 | 84 - 92       |
| (g/day - as consumed)          | 95th percentile | 209    | 90 <sup>th</sup> B1 | 198 - 222     |
| Marine (Finfish and shellfish) | mean            | 1.495  | 90 <sup>th</sup> CI | 1.423 - 1.568 |
| (mg/kg-day - as consumed)      | 95th percentile | 3.888  | 90 <sup>th</sup> B1 | 3.771 - 4.113 |

Table 5.5. Fish intake rate caught at open bay platform (mg/kg·day)

The U.S. EPA (1997) has also provided a checklist for establishing authenticity and validity of the results reported in Table 5.5. The reported values are derived from sample sizes ranging from 10.000 to 37.000 people. The reported data are collected from mid and south Atlantic regions. Table 5.6 summarizes these considerations and rates the confidence of reported values. The overall ranking of confidence of the reported value is medium to high as shown in Table 5.6.

## 5.5.3. LIPID CONTENT (Lc)

The fat or lipid content present in fish may vary from one species to another. The toxic chemicals attach themselves to the fat present in the biota. The U.S. EPA (1999b) has used 1.1% Lc for shrimps for calculating exposure concentration through drilling waste discharges for human consumption. The U.S. EPA (1997) has reported lipid or fat content values for various marine species of finfish and shellfish. Table 5.7 summarizes lipid content of some of these important marine species.

| Considerations                 | Rationale  | Rating         |
|--------------------------------|--|----------------|
| Level of peer review           | Peer reviewed by U.S. DOE and U.S. EPA               | High           |
| Accessibility                  | In public domain                                     | High           |
| Reproducibility                | Enough information is available to reproduce results | High           |
| Focus on factor of interest    | The study focussed on fish ingestion                 | High           |
| Data pertinent to Newfoundland | Data includes Atlantic regions as well               | Medium         |
| Study size                     | Sample size vary from 10, 000 to 37,000              | High           |
| Lack of bias in study design   | Response rates are fairly high                       | High           |
| Measurement error              | Estimates of measurements are imprecise              | Medium         |
| Overall ranking                |  | Medium to High |

Table 5.6. Confidence in fish intakes for general population (after U.S. EPA. 1997)

## 5.5.4. EXPOSURE DURATION (ED)

The exposure duration (*ED*) denotes household residence time in that city or place that would be supplied with contaminated fish. The U.S. EPA (1999b) used two exposure scenarios in the analysis: 30 and 70 years.

According to U.S. DOE (1999). *ED* accounts for households moving into and out of that area, and is generally modeled through joint uncertainty and interindividual variability (*JUV*). The U.S. DOE (1999) has reported values of the *ED* for risk estimation for "best estimate" ( $\hat{R}_{E}$ ), estimate of "reasonably maximum exposed" person ( $\hat{R}_{RME}$ ), and for upper conservative bound, ( $\hat{R}_{RME}$ ). The values are defined in Table 5.8.

| Species                | Lipid content (Lc)<br>(%) | Comments      |
|------------------------|---------------------------|---------------|
|                        | 0.46                      | Raw           |
| Cod. Atlantic          | 0.58                      | Canned        |
|                        | 1.61                      | Dried         |
| Canadras Automatics    | 2.70                      | Raw           |
| Croaker, Atlantic      | 11.71                     | Cooked        |
| El                     | 0.81                      | Raw           |
| riounder               | 1.08                      | Cooked        |
| Haddock                | 0.49                      | Raw           |
| Mullet. Striped        | 2.91                      | Raw           |
| Ocean Perch, Atlantic  | 1.30                      | Raw           |
| Salmon, Pink           | 2.85                      | Raw           |
| Salmon, Red            | 4.56                      | Raw           |
| Sardine, Atlantic      | 10.55                     | Canned in oil |
| Shrimps, Mixed Species | 1.25                      | Raw           |
| Mussel, Blue           | 1.54                      | Raw           |
| Squid                  | 0.99                      | Raw           |

Table 5.7. Lipid content (Lc) of selected finfish/shellfish species (modified after U.S. EPA, 1997)

## 5.5.5. AVERAGING TIME (AT)

The averaging time is generally taken as 70 years (U.S. EPA, 1999b; U.S. DOE, 1999). According to Statistics Canada (2001) the expected average life in various provinces of Canada is reported in Table 5.9. These statistics represent the age census data for years 1990 to 1992. The overall average expectancy of life (AT) for both sexes is 77 years for the Newfoundland population. The U.S. DOE (1999) has also recommended AT as a point estimate in human health risk calculations.

Table 5.8. Exposure duration (ED) in years for various types of risk estimates (U.S. DOE, 1999)

| R E  | RRME  | Retires   |  |
|--|---|---|--|
| 7.6  | 30  | 55.3  |  |
| U.S. DOE (1999) - The<br>number of years an<br>individual residing at<br>the impacted site in the<br>span of 70 years<br>lifetime. | Regulatory values reported by<br>U.S. EPA (1999b) and U.S. DOE<br>(1999). | Approximation for 95°<br>percentile for upper bound of<br>exposure distribution. The<br>procedure used to obtain <i>ED</i><br>distributions adapts the Israeli<br>and Nelson (1992) model of<br>variability in the time of<br>residence for households. |  |

## 5.5.6. EXPOSURE FREQUENCY (EF)

The U.S. EPA has reported an exposure frequency (*EF*) of 365 days/yr for human health risk assessment, whereas the U.S. DOE (1999) has recommended an *EF* of 350 days/yr for regulatory purposes in human health risk assessment. Both studies used *EF* as a point estimate in the risk assessment calculations

| Province or State          | ince or State Both sexes |    | Female |  |
|----------------------------|--------------------------|----|--------|--|
| Newfoundland (NF)          | 77*                      | 74 | 80     |  |
| Prince Edward Island (PEI) | 77                       | 73 | 81     |  |
| Nova Scotia (NS)           | 77                       | 74 | 80     |  |
| New Brunswick (NB)         | 78                       | 74 | 81     |  |
| Quebec (QB)                | 77                       | 74 | 81     |  |
| Ontario (ON)               | 78                       | 75 | 81     |  |
| Manitoba (MB)              | 78                       | 75 | 81     |  |
| Saskatchewan (SK)          | 78                       | 75 | 82     |  |
| Alberta (AB)               | 78                       | 75 | 81     |  |
| British Columbia (BC)      | 78                       | 75 | 81     |  |

Table 5.9. Averaging time (AT) in years for various provinces in Canada (Statistics Canada, 2001)

· 28,105 days

## 5.5.7. SUMMARY OF INPUT PARAMETERS

Table 5.10 summarizes the proposed design values of various HHRA input parameters and also compares them with regulatory values. The minimum, most likely and maximum are reported in proposed design values. The most likely value (MLV) is defined based on either recommended values or most quoted values. The point estimates of averaging time (AT) and exposure frequency (*EF*) are reported. The maximum value of lipid content (*Lc*) for some species are more than 10% but most reported values have *Lc* less than 5% and this is taken as the maximum value.

| Parameter  |   | Regulatory values                   |  |  |  |
|--|---|-------------------------------------|--|--|--|
| (type)   | *Proposed values  | U.S. DOE<br>(1999)                  | U.S. EPA<br>(1999b)                                    | U.S. EPA<br>(1999d)  |  |
|  | As:<br>(44, 114, 333)<br>Cd:  |                                     | 44   | 114  |  |
|  | (4.4, 907, 4100)  |                                     | 64   | 907  |  |
| PCC (1 A-)   | Cr:<br>(1.27, 19, 400)  |                                     | 16   | 19   |  |
| (variability)  | (1800, 3530, 5580)  |                                     | 5.500  | 3530   |  |
|  | (47, 78, 100)   |                                     | 47   | 78   |  |
|  | Zn:<br>(47, 2059, 8853)<br>Naphthalene:   |                                     | 47   | 2059   |  |
|  | (40, 426, 1000)   |                                     | 426  |  |  |
| FIR (mg/kg-day)<br>( as consumed)<br>(variability)   | R (mg/kg-day)<br>s consumed)<br>riability)<br>(3771, 3888, 4113)<br>95 <sup>th</sup> percentile at 90% BI |                                     | Used 99 <sup>th</sup><br>percentile.<br>i.e., 177g/day |  |  |
| Le (%)<br>(variability)  | (0.4, 1.1, 5)<br>Generally 5% as maximum<br>value, is reported.   |                                     | 1.1  |  |  |
| ED (yr)<br>(Not applicable)  | (7.6, 30, 55.3)   | 95 <sup>th</sup> percentile<br>55.3 | 30 and 70  |  |  |
| EF (day/yr)<br>(not applicable)  | 350   | 350                                 | 365  |  |  |
| AT (days)<br>(Not applicable)  | 28,105<br>Based on local census<br>values   | 25.550                              | 25.550   |  |  |
| C (mg/L)<br>(uncertainty)<br>concentration in<br>media<br>C (mg/L)<br>Based on 95 <sup>th</sup> percentile.<br>exposure<br>regression<br>Chapter 3. The 90% C/ is<br>used as lower and upper<br>bound. |   | 95 <sup>th</sup> percentile         | Mean   | 90 <sup>th</sup> percentile is<br>recommended by<br>Lenwood et al.<br>(1998) |  |

Table 5.10. Proposed parameters for human health risk assessment (HHRA)

\*(minimum, most likely or mode, maximum)

## 5.6. PROPOSED METHODOLOGY

Because of simplifying assumptions and the imprecise nature of available information, human health risk assessments are prone to uncertainty. The uncertainty is inherent to risk and an integral part of risk assessment. In human health, the risk is often defined as the probability that damage or adverse effects may occur. Human health risk assessment models consider uncertainties associated with the exposure of the receptor to the contaminants emitted from the source. The other type of uncertainty is related to input parameters. The uncertainties are generally estimated with the help of Monte Carlo (MC) simulations or a fuzzy based approach (Guvonnet et al., 2000).

The MC method assumes that model parameters are random variables and are represented by probability density functions (PDFs). The basis for MC simulation is quite simple in that point estimates are replaced by PDFs and samples are taken randomly from each distribution. The output of the model is also in the form of a PDF or CDF (cumulative distribution function). The MC simulations have been explained in detail in Chapter 3.

Fuzzy arithmetic is a generalized form of interval analysis, which also addresses uncertainty. Instead of representing a number with a single interval or range, it uses a fuzzy number, a group of intervals stacked on top of each other to represent an estimate of the uncertainty. Fuzzy arithmetic is a very useful way to propagate uncertainty because it covers both worst case results and best estimates at the same time. Fuzzy arithmetic is appropriate for handling non-statistical uncertainty including that due to the inability to make accurate measurements. The methodology is computationally fast and does not need extensive sampling (Ferson, 1996).

The framework for treating vague or imprecise information was introduced by Zadeh (1965). In the fuzzy approach, instead of representing uncertainty with PDFs, fuzzy numbers are allotted to variables. A fuzzy number describes the relationship between an uncertain quantity X and a membership function  $\mu_{e}$ . This function is comprised of a range between 0 and 1. Any shape of fuzzy number is possible, but the selected shape should be justified by available information. The basic difference between PDFs and fuzzy numbers is that the area under PDFs is equal to 1, which is not valid for a fuzzy number. In recent years fuzzy arithmetic applications in environmental problems have increased. Dou et al. (1995). Bardossy et al. (1995) and Guyonnet et al. (2000) are a few examples of fuzzy calculus in environmental engineering.

To maintain consistency with the methodologies developed in preceding chapters. MC simulations were used for performing human health risk assessment. The following steps were followed for conducting human health risk assessment:

 The MC simulations using Latin Hypercube Sampling was proposed for human health risk assessment. The parameters of the cancer and non-cancer risk models were defined by simple triangular probability density functions (PDF).  The equation for CDI was modified as given below. The minimum, most likely (MLV or mode) and maximum values were defined for each input parameter (Table 5.11).

$$CDI = \left(\frac{FIR \cdot (C \cdot 10^{-5} \cdot BCF \cdot Lc) \cdot EF \cdot ED}{AT}\right)$$
(5.10)

where

- CDI = Chronic daily intake (mg of contaminant/kg of human body weight/day):
- FIR = Fish ingestion rate (mg of fish/kg of human body weight-day):
- C = Pollutant concentration (mg of contaminant/L of water):
- AT = Averaging time (days);
- EF = Exposure frequency (day/yr);
- ED = Exposure duration (yr);
- 10° = Unit conversion (kg/mg):
- Lc = lipid content (fat fraction); and
- BCF = Bioconcentration factor (L/kg)
- The estimated CDI was used to determine the cancer and non-cancer risk as given in equations 5.2 to 5.5. The R/Ds and SFs for risk agents are point estimates and uncertainties in their estimates are not reported. The output of cancer risk and noncancer risk models were in the form of EDFs. The HI was summed up for all risk agents to obtain HL as a risk estimate for non-cancer hazard.

| D   | Tria  | Shape                             |                           |  |
|---|---|-----------------------------------|---------------------------|--|
| Parameters  | Minimum   | MLV                               | Maximum                   |  |
| FIR (mg/kg·day)   | 3,771   | 3,888                             | 4,113                     |  |
| BCF (Ľkg)   | Based on Table 5.10. Minimum and<br>maximum values are taken from available<br>data. The most likely value is defined based<br>on the regulatory recommended value. |                                   |                           |  |
| Lc (%)  | 0.4   | 5.0                               | value                     |  |
| ED (yr)   | 7.6   | 30                                | 55.3                      |  |
| EF (days/yr)  |   | Minimum Maximum                   |                           |  |
| AT (days)   |   |                                   |                           |  |
| C (mg/L)  |   |                                   |                           |  |
| 95 <sup>th</sup> percentile exp<br>predicted environm<br>(BF) and probabili | osure concentration<br>nental concentration<br>ity of exposure.   | n. The adjuste<br>on for bioavail | ed value of able fraction |  |

Table 5.11. Triangular distributions for human health risk assessment input parameters

The framework of human health risk assessment methodology for cancer and non-cancer risk agents is shown in Figure 5.4. This framework summarizes the steps that are discussed in the proposed methodology for human health risk assessment. The pollutant concentrations estimated from regression models were used as the exposure concentration for the commercial fish. This exposure concentration was used to estimate pollutant concentrations in fish tissues. The fish tissue concentration was used in estimating the chronic daily intake for cancer and non-cancer risk estimates.



Figure 5.4. A proposed framework for conducting human health risk assessment

The human health cancer and no-cancer risk estimates can be combined with the ecological risk (Chapter 4) to determine the total environmental risk for any given drilling waste discharge scenario. The methodology for combining risk estimates for environmental risk management is discussed in the next chapter.

# Chapter 6

# RISK MANAGEMENT

Risk management is an integral part of ecological and human health risk assessments as discussed in Chapters 4 and 5, respectively. Risk management explores and evaluates various alternatives and options which can optimise the risk posed by natural and anthropological activities at minimum cost and resources.

#### 6.1. INTRODUCTION

The major objective of risk management is to help in decision-making and selection of the best alternative available, which can reduce the risk at minimal cost. The application of risk management tools aid in the selection of prudent, technically feasible and scientifically justifiable actions that will protect the environment and human health in a cost-effective way. Risk management is a process of weighting alternatives and selecting the most appropriate action, integrating the results of risk assessment with engineering data and with social, economic, and political concerns to reach a decision.

Risk management integrates political, legal and engineering approaches to manage risks. Risk management is the action proposed based on the best alternative from the decisionmaker's point of view and individual/group priorities. Generally risk assessment is carried out objectively, whereas risk management involves preferences and attitudes, which have objective and subjective elements (Duah, 1993).

This chapter first describes various multiple criteria or multiple attribute decision-making (MCDM or MADM) approaches, which are commonly used in environmental risk management. These approaches are discussed with the help of fuzzy logic concepts. In addition to MCDM, various alternative ranking techniques and weighting schemes are discussed. The proposed MCDM approach - fuzzy composite programming involving the analytical hierarchy process (AHP) and weighting schemes are discussed in detail. Secondly the cost estimation for environmental damage and treatment technologies, and the status of various treatment options are described. Only those treatment technology options that are relevant to offshore disposal of drilling waste are discussed in this chapter. In the end, the research methodology developed in Chapters 2 to 6 is summarised. This overall research summary will help in applying the developed methodology for a case study in Chapter 7. Figure 6.1 illustrates and explains the organisation of this chapter.

#### 6.2. MULTIPLE CRITERIA DECISION-MAKING (MCDM)

Decision-making is an integral part of all management issues and is part of our daily lives. The main concern in decision-making is that decision problems are multiple and diverse in nature and usually have conflicting criteria. In the last 30 years, research related to multiple criteria decision-making (MCDM) or multiple attribute decision-

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making (MADM) has produced enormous literature in the fields of engineering, business and social sciences. Decision-making is broadly classified into MCDM and MODM (multiple objective decision making). The MCDM is associated with problems whose alternatives are predefined and the decision-maker is to select or rank various alternatives. The MODM designs the most promising alternative with respect to limited resources. Hwang and Yoon (1981) have critically reviewed methods and applications of MCDM/MODM for a single decision-maker. For more than one decision-maker, the problem becomes complex and the best solution is the one that will be accepted by all decision-makers. Hwang and Lin (1987) have also discussed group decision-making under multiple criteria.



Figure 6.1. Organization of chapter for developing a methodology for risk management

Fuzzy set applications in decision-making are the extension of many methods used in crisp sets of multiple criteria techniques. Chen and Hwang (1992) have summarised various MCDM methods and categorised them. Research attempts to incorporate uncertainty into decision-making of various attributes involve probability theory and/or fuzzy set theory. Fuzzy set theory is an important tool for modeling uncertainty or imprecision ansing from human perception. Human judgement is involved in decision making in different disciplines, therefore a rational approach toward decision making has to take into account human subjectivity.

A MCDM problem can be expressed in a matrix form

|      | Х,  | X .      |  | <i>X</i> . |       |
|------|-----|----------|--|------------|-------|
| A, ( | X   | $X_{l2}$ |  | Xin        |       |
| A    | X   | X .::    |  | X          | (6.1) |
|      | ×   |          |  |            | (0.1) |
|      |     |          |  |            |       |
| A_   | Xni | X        |  | X          |       |

where

A, = 1, 2...., m are possible courses of action or alternatives:

 $X_i = 1, 2, ..., n$  are attributes with reference to measured alternative performances; and

 $X_{\eta}$  = Performance or rating of alternative A, with respect to attribute or criteria X,

In practical problems, it is common that  $X_v$  are not assessed precisely due to unquantifiable, incomplete and non-obtainable information and partial ignorance. The unquantifiable information leads to subjective attributes, for example good, poor, high, low, etc. Examples of incomplete information are "about one million", " less than 10 miles per hour" etc. Sometimes crisp data are obtainable but need a lot of resources, but "approximation" is possible with less effort and time. Sometimes linguistic descriptions are used because of non-availability of information. Fuzziness is also attributed due to ignorance of factual conditions. These limitations in MCDM methods lead to fuzzy based approaches.

## 6.2.1. CLASSIFICATION OF FUZZY MCDM METHODS

A basic introduction to fuzzy numbers is given in Chapter 5. The applications of fuzzy set theory in various engineering disciplines, especially in decision-making, have gained a lot of popularity. To perform MCDM using fuzzy based methods, the following two steps are followed:

- The aggregation of the performance scores with respect to all the attributes for each alternative; and
- · The rank ordering of the alternative according to the aggregated scores

These two steps are referred to as "final rating" and ranking order", respectively. In crisp MCDM the final rating is expressed in real numbers, therefore rating order can easily be explained. Therefore, in traditional crisp MCDM only the first step is relevant. In a fuzzy MCDM problem, the performance scores of the alternatives are compared and the rank order of fuzzy numbers is determined, which is not a trivial task. Therefore, both phases are integral parts of fuzzy MCDM. The taxonomy of fuzzy MCDM methods is shown in Figure 6.2. Chen and Hwang (1992) have classified MCDM models based on the following four stages:

- 1. Problem size;
- 2. Data type:
- 3. MCDM methods; and
- 4. Techniques.

The size of the MCDM problem depends on the size of alternatives and attributes. Generally, fuzzy methods are suitable for alternatives and attributes less than 10. The data could be in the form of (1) all fuzzy, (2) all fuzzy singleton (single value and its corresponding membership function). (3) all crisp, and mixture of fuzzy and crisp. In stage 3, the concepts of fuzzy methods are derived from classical methods of MCDM: simple additive weighting (SAW), analytical hierarchy process (AHP), multiple attribute utility function (MAUF), etc. This final stage provides the techniques required for applying fuzzy MCDM methods that include the α-cut method, fuzzy arithmetic operations, possibility and necessity measures, eigen-vector method, etc. The details of references and sources of these approaches can be seen in Chen and Hwang (1992).



## 6.2.2. FUZZY COMPOSITE PROGRAMMING (CP) FOR DECISION-MAKING

Fuzzy composite and compromise programming techniques are not given in the taxonomy provided by Chen and Hwang (1992). Fuzzy composite programming is an extension of compromise programming (Bogardi and Bardossy. 1983). In this approach, the analytical hierarchy process and fuzzy arithmetic are used. Recently, this approach has gained popularity and many researchers have used it in environmental decisionmaking problems (e.g., James and Lee, 1992; Lee, 1992; Lee et al., 1991 and Stansbury et al., 1989 etc.). A detailed description of this technique is given in this section.

It is difficult to select an appropriate risk management strategy when the values of input variables, such as ecological and human health risks, cost, and qualitative terms like technical feasibility, are uncertain. Some multiple criteria decision-making optimization tools, including linear multi-objective programming, goal programming, compromise programming, composite programming, multi-attribute utility theory and ELECTRE programming are very popular for this type of problem. Stansbury (1991) has compared some of the decision-making models including MAUF, ELECTRE and composite programming (CP) and suggested that the choice of an appropriate MCDM method depends on the following factors:

- 1. Type of attribute values qualitative or quantitative.
- 2. Types of alternatives continuous or discrete options.
- 3. Robustness of results with respect to changes in attribute values.
- 4. Ease of computation, and

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5. Decision-maker - an individual or a group.

Compromise programming, composite programming, and multi attribute utility theory can effectively deal with discrete and continuous options. These MCDM tools are used to assist decision-makers in solving problems involving multiple attributes and multiple criteria. The uncertainties inherent in these problems are often treated separately from the decision process itself. The decision analysis is often carried out using point or crisp estimates, meaning no uncertainty within input variables. Fuzzy composite programming (CP) is used to assist decision-makers in solving problems of multiple attributes and conflicting objectives. Lee (1992) and Lee et al. (1991) incorporated such uncertainties in MCDM processes for the management of dredged material disposal and for developing management strategies for nitrate control in drinking water.

Stansbury et al. (1989) have developed a methodology that uses composite programming for studying the risk-cost tradeoffs of dredged materials in terms of ecological, human health and socioeconomic impacts. The environmental risk and cost due to excavation, transportation, and disposal of dredged material were evaluated for different available alternatives. The objective was to consider both the economics of dredging and humanhealth and ecological risks that are associated with dredging processes. Basic indicators such as carcinogenic and non-carcinogenic human health risk, ecological risks and costs were defined. Composite programming (CP) organizes the problem into the following format:

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- 1. Identification of management alternatives:
- 2. Definition of basic indicators:
- 3. Grouping them into more generalized indicators:
- 4. Defining weights, balancing factors, the best and worst values of the indicators: and
- 5. Estimation and ranking of the alternatives.

CP is a step by step procedure of regrouping a set of various basic indicators to form a single indicator (Bogardi and Bardossy, 1983). CP uses a composite structure of the basic indicators selected for risk management of drilling waste. In addition to cost and technical feasibility, risks to both human and non-human populations can also be taken as basic indicators. Carcinogenic and non-carcinogenic human health and ecological risks can also be considered.

The first step in CP is the normalization of the basic indicators. This is necessary because all basic indicators have different units and are difficult to compare in their respective units. At the first level, human cancer and non-cancer risks are determined for different contaminants, which are grouped into cancer and non-cancer human risk indices. Similarly, ecological risks for all species of commercial significance are determined for different contaminants to get ecological risk indices. At level 2, the human health cancer and non-cancer risks are grouped as a human health risk index. The same procedure is repeated for individual ecological risk indices to get a final ecological risk index. The Level-2 human and ecological risk indices are grouped to form the final environmental risk at Level 3. The same procedure is repeated for cost and technical feasibility of treatment operations. The grouping for different attributes is performed in steps, until we get the total risk index, total cost and total technical feasibility of operations to make a trade-off analysis among conflicting objectives. This trade-off analysis for different alternatives can be done at all hierarchy levels. The system index value at Level 4 represents the contribution of the final risk index, total cost and final technical feasibility of operations. Figure 6.3 shows a framework of composite programming in a step by step procedure for risk management of drilling waste discharges.





A fuzzy set U is defined as a set of objects or elements x as  $U = \{x\}$ . Therefore a fuzzy set A in U is characterised by a set of ordered pairs  $A = \{(x), \mu_A(x)\}, \forall U \ni x$ , where  $\mu_A(x)$  is grade or degree of membership of x in A (Zadeh, 1965). The membership function  $\mu_A(x)$  assumes its value [0, 1]. When  $\mu = 0$ , it does not belong to A and if  $\mu = 1$ , it indicates x is definitely an element in A. Values between 0 and 1 show the degree of belief.

The values of the basic indicators can be designated by fuzzy numbers to characterise their uncertainties. By defining  $Z_i(x)$  as a fuzzy number of the  $i^{th}$  basic indicator with a trapezoidal membership function of  $\mu/Z_i(x)$ , various management alternatives under uncertainty can be evaluated. The confidence level for an uncertain value can be determined using observed or measured variability. Since units of basic indicators are different, the actual value of each basic indicator should be transformed into an index,  $S_{i,h}(x)$ , using the best or the worst value of the indicator as shown in Figure 6.4 (Lee, 1992).



Figure 6.4. Transforming actual value  $Z_{i,h}(x)$  into normalized index value  $S_{i,h}(x)$ 

Using the index values of basic indicators, the Level 2 index values,  $L_{j,h}(x)$ , of composite indicators can be defined by

$$L_{j,k}(x) = \left\{\sum_{i=1}^{\infty} w_{i,j} \left[ S_{i,k,j}(x) \right]^{p_{i}} \right\}^{1/(p_{j})}$$
(6.2)

where,

n, = Number of elements in second-level group j:

- $S_{i,kj}(x) = Basic value for i<sup>th</sup> basic indicator in the second level group j of basic indicators$ with membership of h:
- $w_{ij}$  = Weight reflecting the importance of each basic indicator ( $\sum w_{ij} = 1$ ); and
- $p_j =$  The balancing factor for group j.

Further, the index values  $L_{4,k}(x)$ , of the third level composite indicators can be calculated by using the index values for the second level composite indicators. This procedure is repeated until the final step, which compares three index values of the 4<sup>th</sup> level indicators: environmental risk, cost and technical feasibility.

Selecting weights (w) and balancing factors (p) depends on a single or group of decisionmakers. The weights represent the relative importance of each indicator as viewed by a decision-maker, whereas the balancing factors account for maximum deviation of the indicators and limit the ability of one indicator to substitute for another. A weighting technique is used to group basic indicators into more general groups. The weights are allotted in each group based on their relative importance. The process, called an analytical hierarchy process (AHP), is used to determine the weight of each indicator in a group by a paired comparison of each of the indicators (Saaty, 1988; Lee, 1992). To determine the ranking for various management alternatives, assume L(x) as a fuzzy number, which is represented by a final composite indicator of alternative x. With the help of two index values,  $L_{hel}(x)$  and  $L_{hell}(x)$ , the membership function,  $\mu/L(x)/c$  of the fuzzy number can be calculated approximately. For m management alternatives there are m fuzzy numbers, [L(x), x = 1, 2, ..., m], to which any ranking method can be applied. The Chen ranking method (1985) determines the ranking of m fuzzy numbers by using maximising and minimising sets. This method has been extensively used in ranking alternative studies (Lee, 1992; Stansbury, 1989). The methodology of fuzzy composite programming allows the incorporation of complex social, ecological and economic information into the fuzzy decision making process. The details of weighting schemes and fuzzy ranking methods are given in later sections.

#### 6.2.3. CONVERTING LINGUISTIC TERMS TO FUZZY NUMBERS

In MCDM we generally deal with linguistic terms good, very poor, excellent etc. A numerical approximation is proposed to convert linguistic terms into corresponding fuzzy numbers. Chen and Hwang (1992) have defined eight scales to convert linguistic terms into fuzzy numbers. Two important scales are reported in Figure 6.5. The first scale has 3 levels, whereas the second scale contains five levels. The linguistic terms for the first scale are "low", medium" and "high". In the second scale, two additional degrees - "very low" and "very high" - are introduced.



Figure 6.5. Conversion of linguistic terms into numerical scores

The same linguistic terms contain different meaning at different scales. The "high" in first scale means [(0.6,0), (0.8,1.0), (1.0,1.0), (1.0,0)], i.e. the most likely interval (when the

membership function,  $\mu$  is 1) is in between 0.8 and 1 and the largest likely interval is in between 0.6 and 1 (when the membership function,  $\mu$  is 0). But in the second scale the "high" means [(0.6,0), (0.75,1.0), (0.9.0)]. This reflects the fact that the same linguistic terms may posses different meaning for different occasions.

The principle of this system is to pick a scale that contains all verbal terms given by the decision-maker and use the fuzzy numbers in that scale to represent the meaning of the verbal terms. Chen and Hwang (1992) have recommended that scales be used in the simplest form available. The determination of the number of conversion scales is intuitive. Miller (1965) has pointed out that " $7 \pm 2$ " represents the greatest amount of information that an observer can give about the objects based on absolute judgement. Therefore, the scales between 5 and 9 may contain maximum information about relative importance of options/alternatives, which are to be compared. In risk management of drilling waste some basic indicators are defined qualitatively. The scale shown in Figure 6.5 will help in defining the numerical scores for those basic indicators used in fuzzy composite programming.

## 6.2.4. FUZZY RANKING METHODS

In traditional MCDM methods, all  $X_{\mu}$ , and  $W_{\mu}$  (relative importance of attributes) values are taken as crisp values. A utility function  $U(x_{1}, x_{2},...,x_{m})$  is defined by a decisionmaker. For alternatives  $A_{\mu}$  the utility function aggregates its performance ratings  $X_{\mu}$  into a final rating,  $U_{\mu}$ . This final rating determines how well one alternative satisfies the decision-maker's utility. The alternatives with higher final ratings are preferred by a decision-maker. If the final ratings are real numbers, then it is straightforward to decide on the best alternative.

The alternative performance rating X<sub>u</sub> can be crisp. fuzzy, and/or linguistic. When fuzzy data are incorporated into MCDM problems the final ratings are no longer crisp numbers, rather they are fuzzy numbers. The fuzzy numbers are not straightforward to compare. In MCDM applications when the final ratings are fuzzy, different ranking methods can be used to compare these fuzzy utility values. Chen and Hwang (1992) have classified ranking methods into four major groups as shown in Figure 6.6.

These groups are preference relation. fuzzy mean and spread, fuzzy scoring and linguistic expression. Each class is further categorised based on the technique involved. Fuzzy scoring techniques are the most popular. In defining left/right scores. Chen's ranking method (1985) is extensively used for risk-cost trade-off analysis (e.g., Stansbury et al. (1989), Lee (1992) etc.). Another simple and popular method proposed by Yager (1980a, b) is based on centroid value. Details of Chen (1985) and Yager (1980a, b) methods are given in the next section.

## Chen Ranking Method (1985)

The first step in Chen's ranking method (1985) is to estimate the final composite L(x) of alternative  $A_i$ . The final index value of the composite system is in the form of fuzzy numbers. With the help of index values,  $L_{h=1}(x)$  and  $L_{h=0}(x)$ , the membership,  $\mu(x)$  can be estimated.

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If there are "m" alternatives, there are m fuzzy numbers  $[Lx_i, x = l, 2, ..., m]$ . The ranking of these "m" fuzzy numbers is done by maximising and minimising a set. The maximising set M is a fuzzy subset with membership function  $\mu_d(L)$  given as

$$\mu_{u}(L) = \begin{bmatrix} \frac{(L-L_{uu})}{(L_{uu}-L_{uu})}, & L_{uu} \leq L \leq L_{uu} \\ 0, & OTHERWISE \end{bmatrix}$$
(6.3)

where,  $L_{max} = min[minL_{h=0}(x)]$  and  $L_{max} = max[maxL_{h=0}(x)]$  for x = 1, ..., m. Then the right utility value,  $U_R(x)$ , for alternative A, is defined as

$$U_{g}(X) = max(min\{\mu_{M}(L), \mu[L(x)]\})$$
(6.4)

The minimising set G is a fuzzy subset with membership function  $\mu_{c}(L)$  given as

$$\mu_{c}(L) = \begin{bmatrix} \frac{(L - L_{wu})}{(L_{wu} - L_{wu})}, & L_{wu} \leq L \leq L_{wu} \\ 0, & OTHERWISE \end{bmatrix}$$
(6.5)

The left utility value,  $U_L(x)$ , for alternative A, is defined as

$$U_{L}(X) = max(min\{\mu_{G}(L), \mu[L(x)]\})$$
(6.6)

The total utility or ordering value for alternative A, is

$$U_{\tau}(x) = \frac{U_{R}(x) + I - U_{L}(x)}{2}$$
(6.7)

The alternative, which has the highest ordering value, is selected as the best alternative. Chen (1985) has generalised the results for trapezoidal fuzzy numbers as shown in Figure 6.7.

$$U_{T}(x) = \frac{1}{2} \left[ \frac{(d_{i} - L_{min})}{(L_{max} - L_{min}) - (b_{i} - d_{i})} + I - \frac{(L_{max} - c_{i})}{(L_{max} - L_{min}) + (a_{i} - c_{i})} \right]$$
(6.8)



Figure 6.7. Chen (1985) ranking method for trapezoidal fuzzy numbers

## Yager's Centroid Index Ranking Method (1980b)

In this category of ranking methodology, the centroid of the fuzzy number is determined. Each geometric centre corresponds to an x value on the horizontal axis. Yager (1980b) has proposed a ranking index as follows:

$$x_{o} = \frac{\int_{0}^{1} g(x) \mu_{i}(x) dx}{\int_{0}^{1} \mu_{i}(x) dx}$$
(6.9)

where g(x) is treated as a weight function measuring the importance of the value x. The denominator serves as a normalising factor whose value is equal to the area under the membership function  $\mu_i$ . When g(x) = x (linear weight), equation 6.9 gives the  $x_o$  (geometric centre). The value of  $x_o$  is the weighted mean value of fuzzy number L(x) and higher  $x_o$  values are considered better. Figure 6.8 shows a comparison of two fuzzy numbers and their centroids. The software Risk Calc (1995) estimates the centroid of a fuzzy number. The centroid is generally not the same as the mean except for scalars.



Figure 6.8. Yager's (1980b) centroid index method for ranking fuzzy numbers

Yager's estimate for ranking fuzzy numbers is not considered to be a very reliable method and Murakami et al. (1983) have modified this method. In this research, both Chen's (1985) and Yager's (1980b) methods were used to rank the alternatives.

## 6.2.5. WEIGHTING METHODS

MCDM methods require information about the relative importance of each attribute or criteria. The preference weights are estimated with the help of an importance matrix. They are normalised to sum to 1. In case of "n" criteria, a set of weights is

$$\underline{W}^{T} = (w_{1}, w_{2}, \dots, w_{n}, \dots, w_{n})$$
(6.10)

and

$$\sum_{j=1}^{n} w_{j} = l$$
 (6.11)

The MCDM problem becomes

$$D = \begin{pmatrix} X_{1} & X_{2} & . & . & X_{n} \\ A_{1} & X_{21} & X_{22} & . & . & X_{1n} \\ X_{22} & X_{22} & . & . & X_{2n} \\ . & . & . & . & . \\ A_{n} & X_{n2} & X_{n2} & . & . & X_{nn} \\ \end{pmatrix} \begin{pmatrix} W \\ W_{1} \\ W_{2} \\ . \\ . \\ W_{n} \end{pmatrix}$$
(6.12)
Hwang and Yoon (1981) described four different techniques for MCDM weighting schemes, which include: eigen-vector method, weighted least square method, entropy method and LINMAP. Saaty (1977) has introduced a method of scaling ratios using the principal eigen-vector of a positive comparison matrix. Weighting coefficients are assessed to reflect the relative importance of each attribute. Saaty (1988) suggested an analytical hierarchy procedure (AHP) to estimate the relative weight of each attribute in a group based on a paired comparison. To compare criterion "*t*" with "*y*" the decision-maker can assign value *a*<sub>u</sub> from Table 6.1.

| Intensity of<br>importance | Definition              |
|----------------------------|-------------------------|
| 1                          | Equal importance        |
| 3                          | Weak importance         |
| 5                          | Strong importance       |
| 7                          | Demonstrated importance |
| 9                          | Absolute importance     |
| 2, 4, 6, 8                 | Intermediate values     |

Table 6.1. Linguistic measures of importance (Saaty, 1988)

He proposed the following steps to assign the weights:

- a. If  $a_{ij} = \tau$ , then  $a_{ji} = (1/\tau)$ , where  $\tau \neq 0$  and  $i \neq j$ ;
- b. If i = j, then  $a_{ij} = a_{ji} = 1$ ; and
- c. Construct matrix  $A = (a_{ij} | i = 1, ..., n; j = 1, ..., n)$ .

Saaty (1988) has shown that the eigen-vector corresponding to the maximum eigen value of matrix A is a cardinal ratio scale for the criteria compared. The eigen value problem can be solved by

$$A \cdot W = \Phi_{max} \cdot W \tag{6.13}$$

where,  $\Phi_{max}$  is a scalar corresponding to the maximum eigen value and the unit vector "W" corresponding to  $\Phi_{max}$  gives the preference weights.

Lu and Hu (1999) have suggested a simple technique for calculating the weights in water quality analysis. They used a decision matrix for three different factors: chlorophyll-a (C), phosphorous (P) and Secchi depth (S) for determining eutrophication effects. The relative importance of these factors can be assigned using values given in Table 6.1.

$$P C S 
J = \begin{bmatrix} 1 & 1/2 & 2 \\ 2 & 1 & 3 \\ 1/2 & 1/3 & 1 \end{bmatrix} C (6.14)$$

A normalised matrix I is determined by normalising the matrix J column wise. The final weighted vector can be derived from summing the elements of each row of matrix I and normalising again for this vector.

$$I = \begin{bmatrix} 0.286 & 0.273 & 0.333 \\ 0.571 & 0.546 & 0.500 \\ 0.143 & 0.182 & 0.167 \end{bmatrix} \Rightarrow \begin{bmatrix} 0.892 \\ 1.617 \\ 0.4914 \end{bmatrix}$$
(6.15)

Therefore after normalising to 1, the above equation becomes

$$W^T = [P \quad C \quad S] = [0.297 \quad 0.539 \quad 0.164]$$
 (6.16)

A double weighting scheme is proposed in composite programming (equation 6.2). The other type of weight used in CP is balancing factor ( $p_i$ ). The balancing factor ( $p_j$ ) is assigned to groups of indicators to reflect the importance of the maximum deviations, which represent maximum difference, an indicator value, and the best value for that indicator. The larger the value, the greater the concern with respect to the maximum deviation. When  $p_j = 1$ , all deviations are equally weighted, for  $p_j = 2$ , each deviation receives its importance in proportion to its magnitude (Torno et al., 1988).

#### 6.2.6. CONVERTING STATISTICAL DATA INTO FUZZY NUMBERS

The concept of probability is usually related to the frequency of occurrence of events, obtained by repeated experiments. In contrast, fuzzy set theory could provide the appropriate framework to assess the possibility of events rather than their probability (Dubois and Prade, 1986). The most crucial step in the design of a problem that is to be solved by fuzzy set methods is the determination of membership functions ( $\mu$ ). There are many guidelines on developing membership functions for fuzzy sets. The fuzzy sets based on statistics are perhaps the most naturally fuzzy sets that can be used (Civanlar and Trussel, 1986).

The statistical data can be represented by histograms, which can be used for approximation of a probability density function (PDF). Consequently, the fuzzy sets using this histogram can follow the rule that, max  $[\mu(x)] = 1$ . A similar approach is used by Guyonnet et al. (2000) for comparing MC simulations and fuzzy sets as two methods of measuring uncertainties. Figure 6.9 shows that the highest frequency value (mode or MLV) is allotted a membership value of L Similarly the other values are also normalised proportionally.

Civanlar and Trussel (1986) have pointed out that from a heuristic point of view the elements that are most likely should have high membership values, however the set should be as selective as possible. These requirements are given below:

- 1. E  $[\mu(x)]x$  should be distributed according to underling probability density function (PDF);
- 0 ≤ µ ≤ 1. Because an infinite scale is not necessary to assign grade of membership function: and
- 3.  $\Sigma \mu^2$  should be minimised i.e. the fuzzy number should be as small as possibly.



Figure 6.9 Conversion of statistical data into a fuzzy number

Jooste (2001) developed a simple methodology for risk estimation of co-occurring stressors in an aquatic ecosystem based on fuzzy sets. He used the lowest 5<sup>th</sup>, median and the highest 95<sup>th</sup> percentiles defining a triangular fuzzy number. A similar approach was adopted in this research. The confidence intervals of lowest 1% and highest 99% were used to define the largest likely interval of the fuzzy number. The mode or most frequent value was used to define the most likely value of the fuzzy number with a membership function value of 1. Figure 6.10 shows this methodology for developing a triangular fuzzy number from statistical data. The models developed in this research (Chapters 3-5) are statistical in nature. The values were converted into triangular fuzzy numbers using this technique.



Figure 6.10. Methodology for developing a triangular fuzzy number

The next section focuses on the status of various types of treatment technology options available for offshore drilling operations. It helps in understanding the technical feasibility parameters of the treatment devices used for drilling waste risk management.

## 6.3. STATUS OF OFFSHORE TREATMENT TECHNOLOGY

The clean water act (CWA) requires the U.S. EPA to identify effluent reductions attainable through the application of a *Best Practicable Control Technology* (BPT). The U.S. EPA has estimated effluent levels based on the long-term average of the best existing treatment plants of various sizes and ages. The BPT is applied to conventional (BOD<sub>3</sub>, SS, pH and fecal coliform etc.), toxic, and non-conventional pollutants. The CWA has recommended the *Best Conventional Pollutant Control Technology* (BCT) as an additional level of control for discharges of conventional pollutants from existing industrial point sources. The CWA has also established the *Best Available Technology Economically Achievable* (BAT) as their main feature to control the discharge of toxic and non-conventional pollutants. The BAT effluent limitation guidelines represent the best existing economically achievable performance of direct discharging plants. The important factors are the cost of achieving BAT effluent reductions, the age of equipment and facilities involved, processes used and engineering aspects of control technology, and non-water quality impacts etc. The BAT economic achievability is determined on the basis of the total cost to the industrial subcategory and the overall effect of the rule on the industry's financial health.

The U.S. EPA (2000b) supports pollution prevention technology (P2) by encouraging the appropriate use of SBFs. The U.S. EPA intends to control the discharge of SBFs, either by using state of the art technology and/or by encouraging the replacement of OBFs. The OBFs are subject to zero discharge requirements and land based disposals are recommended. The U.S. EPA has estimated drilling fluid retention values for solid control equipment used in the U.S. offshore (baseline solids control system) and for add on solids control systems (used in the North Sea), which are capable of reducing retention values lower than the baseline solids control system.

#### 6.3.1. SOLID CONTROL SYSTEMS

The function of the solid control system is to separate drill cuttings from drilling fluid and at the same time maintain rheological properties of drilling fluids. The quality of drilling fluids deteriorates as the quantity of fine particles in the drilling fluid increases (Anon, 1999). The solid control system contributes to the fine particle fraction due to breakdown of larger particles. Therefore, the design and operation of the solid control system is important to limit the mechanical destruction of cuttings.

Separation of base drilling fluid is more difficult for WBFs than SBFs/OBFs. The solids have a tendency to disperse in the water phase. Therefore, different treatment techniques are used for WBFs and SBFs/OBFs. Additional treatments such as hydrocyclones and chemical flocculation units are used for WBFs. Typical solid control systems for SBFs drilling waste consist of primary and secondary shale shakers, a "drying" shale shaker or centrifuge which further removes the drilling fluid from the cuttings, a "high-G" shale shaker or centrifuge to remove fine solids, and a sand trap. Table 6.2 categorises various solid control devices. A brief description of various types of commonly used solid control devices is given in the following section.

| Unit Type                  | Category               | G-Force |
|----------------------------|------------------------|---------|
| Linear motion shale shaker | Secondary shale shaker | 6.25    |
| Mud Cleaner                | *FRU                   |         |
| Decanting centrifuge       | FRU                    |         |
| Linear motion shale shaker | Dryer                  | 8.0     |
| Vibrating centrifuge       | Dryer                  | 130     |
| Vertical centrifuge        | Dryer                  | 800     |
| Squeezing press            | Dryer                  |         |

Table 6.2. Drilling fluid recovery devices category

\*Fines removal unit

#### Shale Shakers

Shale shakers are vibrating screens used at primary and secondary levels of treatment. The primary or scalp shale shaker removes the larger size drill cuttings and protects downstream equipment from wear and tear. The primary shale shaker (PSS) recovers the drilling fluid from coarser size cuttings and sends the drilling fluid stream to a secondary shale shaker (SSS, mud cleaner). The cuttings from the primary shale shaker are sometimes sent to a new type of shale shaker, called a "drying" shaker or "cutting fluid (Anon. 1999).

In shale shaker design, screen cloth characteristics, motion type, position and size of screen and arrangement of multiple screens are important parameters. The factor that differentiates primary and secondary shale shakers is the size of solids removed. The primary shale shakers have coarser screens than the secondary and the drying shale shakers. Longer retention times on shale shakers increase the degradation of solids on the screens. Flat screens provide the least retention time and surface area in comparison to other designs. Recently, corrugated screens are in vogue, which provide more surface area and more retention time, but at the same time more capacity as well. Downward slope orientation provides faster conveyance and less retention time compared to upward slope. The U.S. EPA (2000b) has evaluated the operation of primary and secondary shale shakers for flat and corrugated screen deigns. Two tier shale shakers are very popular in the GOM (Gulf of Mexico). The U.S. EPA (2000b, 1999a) has observed that the downward sloping flat screens on the top tier minimise mechanical degradation of cuttings. The bottom tier is generally corrugated and slopes upward toward the discharge end. The cuttings from lower screens consist of smaller cuttings and mud type solids. These types of shale shakers require a force of 2 to 4G.

#### High-G Shale Shaker

The applied G force in this type of shale shakers ranges from 6 to 8G. The high-G shakers are used to remove fine particles from drilling fluid to maintain the viscosity of the drilling fluid stream. These shakers can also be used as "drying" shakers. The U.S. EPA (2000b, 1999a) has observed that high-G shale shakers are equipped with upward sloping corrugated screens. The solids discarded from high-G shale shakers are in the form of mud, but carry finer particles than the secondary shale shakers.

#### Centrifuges

Centrifuges are either a substitute for shale shakers or used as an add-on technology in addition to shale shakers. When centrifuges are used with shale shakers the efficiency of the solid control system increases by up to 30 or 40%. These units are installed in place of drying shale shakers. Recently, centrifuges have been used to recover SBFs from large cuttings; such centrifuges are very large in size and process all coarse and small drill cuttings. Mud 10 is a combination of shale shaker and centrifuge. It has an internal rotating cone that vibrates and achieves very low retention of SBFs on cuttings (U.S. EPA, 2000b). A schematic of an arrangement of solid control system in the North Sea and GOM is shown in Figure 6.11. This solid control arrangement is able to reduce SBFs retention value to 3.85%. The centrifuge dryer is different from Mud 10 and has a vertically oriented screen centrifuge and has a very high-G value. The U.S. EPA (2000b) has reported that retention values are as low as 3.72% in that case.

#### Squeeze Press

Squeeze presses are used to separate attached drilling fluid from the bulk cuttings waste stream prior to discharge. The squeeze press creates a brick like solid chunk with entrapped drilling fluid. This is not widely used for recovering of drilling fluid. It can reduce the retention level to 6.71% SBFs on cuttings, which is in between the efficiency of primary shale shakers (9.32%) and centrifuges (3.7 to 3.85%). The efficiency of a squeeze press is also better than a secondary shale shaker (13.8%) and mud cleaner (11.9%).

#### Fines Removal Units (FRU) or Mud Cleaner

An acceptable level of fines (< 5  $\mu$ m) is less than 5% in drilling muds. Higher levels lead to drilling problems due to changes in rheological properties of the drilling fluid. The U.S. EPA (2000b, 1999a) has suggested that where there are unfavourable formation characteristics, operators must be limited to diluting the fines in the active mud system through addition of fresh SBF and/or capturing a portion of fines in a container and sending the fines for disposal.



Figure 6.11. Configuration of Mud 10 solid control system (U.S. EPA, 2000b)

## Thermal Treatment Units

The U.S. EPA (2000b) has investigated four thermal distillations and oxidation processes for removing oil from drill cuttings as a part of their final rulemaking. These technologies appeared to be promising in achieving their goal in oil based types of waste. But the U.S. EPA (2000b) has rejected them for further investigation because of associated difficulties with these technologies, which include installation offshore, operation of equipment, handling of processes waste and their by products, etc. The U.S. EPA (2000b) has also mentioned that using thermal distillation technologies offshore is getting popular for SBF waste treatment and recovery. The U.S. EPA (2000b) did not recommend these technologies for defining BAT limitations, although they are more popular in land based treatment processes.

#### 6.3.2. PERFORMANCE OF SOLID CONTROL SYSTEMS

A primary shale shaker is generally a first treatment device in a solid control system. The secondary shale shaker receives drilling fluid/drill cuttings from primary shale shakers. The U.S. EPA (2000b) has estimated and reported long term average retention values of 10.6% by weight of synthetic base fluid on wet cuttings for a primary shale shaker and 15.0% for a secondary shale shaker. The U.S. EPA (2000b) has also provided retention data for a vibrating centrifuge, the add-on solid control technology that is currently in use in the North Sea. Based on available data. a long-term average of 5,14% by wet weight of synthetic base fluid can be achieved with a vibrating centrifuge. The U.S. EPA (2000a, b) have proposed effluent limitation guidelines for the control of pollutant discharges associated with the retention on cuttings (ROC) of SBFs. The U.S. EPA used the statistics of performance of various solid control devices for the following reasons:

- 1. Estimating the quantities of current baseline pollutant discharges:
- 2. Calculating the potential effluent limits; and
- 3. Evaluating regulatory options.

The U.S. EPA (2000b) has selected two final numeric limits for the retention of SBFs on cuttings. For base fluid (e.g., ester) environmental properties - toxicity, biodegradation, and bioaccumulation - the average ROC should not exceed 9.4% by wet weight. A technology including horizontal and vertical centrifuges, squeeze presses, and high G dryers can be used. For other types of base fluids, the average ROC should not exceed 6.9% by wet weight. The U.S. EPA (2000b) has summarised percent retention of SBFs on cuttings based on three different technology types. These technologies include simple shale shaker systems (e.g., primary and secondary shale shakers etc.), cuttings dryers (e.g., horizontal centrifuge and Mud 10), vertical centrifuge, squeeze press, high G dryer and fines removal units (FRU) (e.g., decanting centrifuges and mud cleaners). The reported data are expressed by weight of SBF/weight of wet cuttings. The summary of data is given in Table 6.3.

| Technology category/subcategory   | No. of<br>wells | Mean<br>(µ) | Std. dev.<br>( $\sigma$ ) | 95 <sup>th</sup><br>percentile |
|---|-----------------|-------------|---------------------------|--------------------------------|
| Primary shale shaker  | 32              | 9.32        | 3.05                      | 14.3                           |
| Secondary shale shaker  | 22              | 13.8        | 3.48                      | 19.5                           |
| Other shale shakers   | 22              | 8.96        | 1.78                      | 11.9                           |
| Horizontal centrifuge (Mud 10)  | 26              | 3.85        | 2.01                      | 7.16                           |
| Vertical centrifuge   | 8               | 3.72        | 1.54                      | 6.26                           |
| Squeeze press   | 5               | 6.71        | 1.39                      | 8.99                           |
| High-G dryer  | 6               | 9.40        | 2.17                      | 13.0                           |
| Cuttings dryer I<br>Horizontal centrifuge + Vertical centrifuge +<br>Squeeze press + High-G dryer | 45              | 4.89        | 2.72                      | 9.37                           |
| Cuttings dryer II<br>Horizontal centrifuge + Vertical centrifuge +<br>Squeeze press               | 39              | 4.19        | 2.06                      | 7.59                           |
| Cuttings dryer III<br>Horizontal centrifuge + Vertical centrifuge +<br>Squeeze press              | 34              | 3.82        | 1.89                      | 6.93                           |
| Decanting centrifuge  | 22              | 9.97        | 2.27                      | 13.7                           |
| Mud cleaner (FRU)   | 21              | 11.9        | 2.64                      | 16.2                           |
| Fines removal<br>Decanting centrifuge + Mud cleaner   | 39              | 10.8        | 2.51                      | 14.9                           |

Table 6.3. SBFs solid control system performance (modified after, U.S. EPA, 2000b)

The U.S. EPA (2000a, b) have compared the performance of different treatment technologies. For ester, the numeric limit is based on combining data from high-G dryer, squeeze press, horizontal centrifuge, and vertical centrifuge. The high-G dryer is important because it takes less space than other technologies. The U.S. EPA (2000b) has reported that in the development of a final numeric limit, 3 out of 6 high-G dryers, all 5 squeeze presses and 8 vertical centrifuges, and 25 horizontal centrifuges out of 26

complied with the numeric limit of 9.4%. The U.S. EPA (2000b) has also reported that all eight vertical centrifuges and 24 out of 26 horizontal centrifuges complied with a numeric limit of 6.9%.

The U.S. EPA (2000a, b) have suggested a baseline treatment train which includes primary shale shaker (PSS), secondary shale shaker (SSS), and fines removal units (FRU). The estimated volume contributions in a waste stream for the baseline control system are 78.5% (PSS), 18.5% (SSS) and 3% (FRU), respectively. Therefore, the synthetic base fluid percent attachment can be estimated as

Baseline treatment =  $(9.32 \times 0.785 + 13.8 \times 0.185 + 10.8 \times 0.03) = 10.20 \%$ 

The final cuttings waste stream retention values for BAT options 1 and 2 are given in Table 6.4. The U.S. EPA (2000b) has estimated the ROC values based on the cutting drver I and the fine removal unit (FRU) given in Table 6.3.

|         | Performance of system |      | Percent contributed (%) |     | Performance of system Percent contributed ( |  | Compliance level |
|---------|-----------------------|------|-------------------------|-----|---|--|------------------|
| Options | Cuttings<br>dryer     | FRU  | Cuttings dryer          | FRU | (% retention)                               |  |                  |
| BATI    |                       | 10.0 | 97                      | 3   | 4.03  |  |                  |
| BAT II  | 3.82                  | 10.8 | 100                     | 0   | 3.82  |  |                  |

Table 6.4. BAT weighted average compliance level solids retention

In the next section, the cost estimations are made for drilling fluid economic loss, treatment costs, human health and ecological damage costs etc.

## 6.4. COST ESTIMATION

In risk management, the optimisation of cost out of various feasible alternatives is one of the major objectives. Technically, various alternatives that can reduce the risk are possible to implement, but the decisive factor could be the cost to control that risk. The cost for various alternatives include the treatment cost, cost of loss of drilling fluid, economic loss due to lower production of commercially valuable ecological entities, human health damage cost, and possible non-water quality environmental damages. All cost estimates are reported in CAD 5 in the following sections.

#### 6.4.1. DRILLING FLUID COST

The U.S. EPA (2000b) has summarised the cost of SBFs. OBFs and WBFs drilling fluids. The U.S. EPA used \$70.7/bbl for WBFs for their compliance cost estimates. The OBFs and SBFs per barrel cost estimates are \$117.8 and \$314, respectively. The OBFs cost data estimates are in the range of \$109.9 to 141.3/bbl. The U.S. EPA (2000b) has suggested a mean value of \$124/barrel for OBFs. The U.S. EPA (2000b) has estimated that within next few years the SBFs cost would be within range of \$251.2 to 471/bbl. The U.S. EPA (2000b) has also suggested a mean value of \$347/bbl for SBFs. Table 6.5 summarises the range and mean values for cost estimates of various drilling fluids on a per barrel basis.

The costs of drilling fluid loss are based on the amount attached to the cuttings, which can be determined for various discharge options.

| Drilling Fluid | Mean | Range         |
|----------------|------|---------------|
| SBFs           | 347  | 251.2 - 471   |
| OBFs           | 124  | 109.9 - 141.3 |
| WBFs           | 70.7 |               |

#### Table 6.5. Drilling fluid per barrel cost (\$) estimates

## 6.4.2. SOLIDS CONTROL TREATMENT COST

The U.S. EPA (2000b) has reported three best available technology options for control and treatment of SBF drill cuttings, which include:

- 1. Controlled discharge BAT option 1:
  - (a) Limitations on stock synthetic base fluid (PAH content, biodegradation rate and sediment toxicity);
  - (b) Limitations on discharged SBF-cuttings (no free oil, formation oil contamination, sediment and aqueous toxicity, retention on SBFs cuttings) discharged;
  - (c) Limitations on Hg and Cd stock barite; and
  - (d) Prohibition of diesel oil discharge
- BAT discharge option 2: This is the same as the controlled discharge option except that retention of SBF on cuttings is based solely on the discharge from the cuttings dryer units, and does not include an allowance for the discharge of the fines removal units.

3. BAT option 3: Zero discharge option (Not considered in this research)

The U.S. EPA (2000a, b) have developed a methodology for calculating compliance cost for various options. They have summarised annual technology costs and pollutant loadings for drill cutting BAT options. The total treatment cost for year 1999 for various options is given in Table 6.6.

| Options  | Technology Cost<br>(\$/yr) |
|--|----------------------------|
| BAT option 1: Discharge with 4.03% retention of drilling fluid on cuttings | 70,000,000                 |
| BAT option 2: Discharge with 3.82% retention of drilling fluid on cuttings | 70,300,000                 |
| BAT option 3: Zero discharge   | 112,900,000                |

Table 6.6. Annual total technology cost for various BAT options in 1999

The U.S. EPA (2000a, b) have estimated per well waste volume using the number of drilling days required for different model wells from data provided by the American Petroleum Institute (API). Table 6.7 summarises the estimated drilling days for various model wells. Active drilling days comprise approximately 40% of the total time to drill, during which equipment is onsite. The actual days to drill can be calculated by multiplying by a factor of 2.5 to get active drilling days. For simplicity, one-month average time (30 days) was assumed to estimate the cost of solid control equipment rentals and for other expenditures.

| Model well                     | Active drilling days | Actual drilling days |
|--------------------------------|----------------------|----------------------|
| Shallow water development well | 5.2                  | 13.0                 |
| Shallow water exploratory well | 10.9                 | 27.3                 |
| Deep water development well    | 7.9                  | 19.8                 |
| Deep water exploratory well    | 17.5                 | 43.8                 |

Table 6.7. Drilling days for various model wells

The U.S. EPA (2000b) has also reported the cost associated with onsite treatment of drill cuttings for the baseline and BAT options. For calculating the incremental compliance costs, the baseline solids control consisting of primary shale shaker followed by a secondary shale shaker is considered. The secondary shale shaker receives drilling fluid from the primary shale shaker and discharges smaller particle sized drill solids than the primary shaker (Anon, 1999). The purpose of the primary shale shaker is to receive the drilling fluid and drill cuttings coming from the bore hole and make the first separation of cuttings from the drilling fluid, whereas the secondary shale shaker is used to separate the smaller solid particles from the drilling fluid. The long-term average of 10.2% base fluid retained on wet cuttings is reported (U.S. EPA, 2000b).

Both BAT control options 1 and 2 can reduce the solids retention below the baseline retention value. The difference between two BAT options is not based on the use of different treatment technologies, rather it is identical for both options. In BAT option II, the FRUs are excluded. The technologies considered under the category of "cuttings dryers" includes vibrating centrifuges (horizontal and vertical), squeeze press units and high-G linear shakers. The cost of add-on technology is estimated as \$ 3,768/day (US EPA, 2000b), The rental costs for various treatment devices are reported in Table 6.8.

| Device type                              | Cost (\$/day) |   |
|--|---------------|---|
| Shale shaker                             | 350           | - |
| (Operation and Maintenance)              | 80            |   |
| Mud 10 (vibrating horizontal centrifuge) | 3768          |   |
| Vertical centrifuge                      | 1884          |   |
| Mud cleaner (FRU)                        | 300           |   |

Table 6.8. Cost estimates for various treatment units (U.S. EPA, 2000b)

The cost estimates for various treatment options are summarised in Table 6.9. The treatment costs are estimated based on daily rentals cost for a 30 day drilling period, installation and downtime cost, and monitoring analysis, which includes crude oil contamination testing, retention of base fluid testing (retort analysis) and sediment toxicity testing. The total costs of tests are normalised to one-month time to estimate the daily cost.

Four treatment options are considered to develop a relationship between % base fluid attached and treatment cost/day. These options include a baseline solid control system option, BAT options 1 and 2, and an extra option assumed in which the treatment units are primary and secondary shale shakers and a vertical centrifuge as a tertiary treatment device. In this option it is assumed that 60%, 20% and 20% waste is coming from primary, secondary and vertical centrifuges, respectively. The % base fluid attached is calculated by the following relationship:

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| Treatment option | Item   | Cost (\$)       | Cost (\$/day) |
|------------------|--|-----------------|---------------|
|                  | <u>Treatment units</u><br>Shale shakers (2)<br>FRU | 00 705          | 860<br>300    |
| Baseline solid   | Installation and downtime cost                     | 88,705          | 2,957         |
| control system   | Monitoring analysis                                |                 | 100           |
| option (10.2%)   | Crude oil contamination test (@80/test)            | 80              |               |
|                  | Retention of base fluid (@80/test)                 | 2,040           |               |
|                  | SedTox monitoring test                             | 900             | -             |
|                  | Total cost   |                 | 4217          |
|                  | Treatment units                                    |                 |               |
|                  | Shale shakers (2)                                  |                 | 860           |
|                  | Vertical centrifuge                                |                 | 1,884         |
| Extra option     | Installation and downtime cost                     | 88,705          | 2,957         |
| (9.1%)           | Monitoring analysis                                |                 | 100           |
|                  | Crude oil contamination test (@80/test)            | 80              |               |
|                  | Retention of base fluid (@80/test)                 | 2040            |               |
|                  | SedTox monitoring test                             | 900             | 1.000         |
| a state of the   | Total cost   |                 | 5,801         |
|                  | Treatment units                                    |                 |               |
|                  | Shale shaker                                       |                 | 430           |
|                  | FRU  |                 | 300           |
|                  | BAT solid control system                           | 88,705          | 3,768         |
| BAT option I     | Installation and downtime cost                     |                 | 2,957         |
| (4.03%)          | Monitoring analysis                                | 1.201.000       | 100           |
|                  | Crude oil contamination test (@80/test)            | 80              |               |
|                  | Retention of base fluid (@80/test)                 | 2040            |               |
|                  | SedTox monitoring test                             | 900             |               |
|                  | Total cost   | a destant and a | 7,555         |
|                  | Treatment units                                    |                 |               |
|                  | Shale shaker                                       |                 | 430           |
|                  | BAT solid control system                           | 88,705          | 3,768         |
|                  | Installation and downtime cost                     |                 | 2,957         |
| BAT option II    | Monitoring analysis                                | 1.200           | 67            |
| (3.6270)         | Crude oil contamination test (@80/test)            | 80              |               |
|                  | Retention of base fluid (@80/test)                 | 1020            |               |
|                  | SedTox monitoring test                             | 900             |               |
|                  | Total cost   |                 | 7,222         |

# Table 6.9. A single well cost estimates for various options (modified after, U.S. EPA, 2000b)

Extra option =  $(9.32 \times 0.60 + 13.8 \times 0.20 + 3.72 \times 0.20) = 9.10 \%$ 

The costs given in Table 6.9 do not include the cost of drilling fluid lost during ocean disposal in each treatment option. Based on synthetic base fluid % retention and daily estimated treatment cost data, a simple exponential relationship is fitted which is given below:

$$Cost(\$/day) = A \cdot e^{b \cdot (\% base fluid)}$$

Table 6.10 provides the details of the statistics of the above equation. The R<sup>2</sup> estimate is approximately 0.86, which shows a reasonably good fit. To find out the confidence interval (*CI*) at upper and lower bounds of the cost estimates, mean ( $\mu$ ) and standard deviation (or standard error,  $\sigma$ ) can be used by performing Monte Carlo (MC) simulations. Figure 6.12 shows the fit of the regression equation.

Table 6.10. Regression statistics of treatment cost equation

| Predictor | μ      | σ     | р     | R <sup>2</sup> | $R^2_a$ |
|-----------|--------|-------|-------|----------------|---------|
| Ln (A)    | 9.20   | 0.158 | 0.000 | 85.6           | 50 F    |
| Ln (b)    | -0.074 | 0.022 | 0.075 |                | 78.5    |

(6.17)



Figure 6.12. Fitted model for treatment cost and base fluid % retention

#### 6.4.3. HUMAN HEALTH DAMAGE COST

Human health risk costs in a comparative risk project are costs associated with the incidence of environmentally induced illness in addition to regular illnesses in the absence of that particular environmental problem. Healthcare cost assessments in comparative risk projects have typically concentrated on medical costs and the cost of lost work time. Indirect costs stand for reduced productivity due to the lost work time and are estimated based on expected earnings during that lost time (U.S. EPA, 1993).

Table 6.11 summarises the daily health care costs associated with a variety of cancer and non-cancer illnesses. This cost includes direct and indirect costs. The values shown in Table 6.11 are derived from the U.S. EPA (1993) and are projected to the year 2000 using a Consumer Price Index (CPI). According to the data given in Statistics Canada (2001) \$100 in 1986 is valued at \$139 in 1997 in the health and personal care sector. The discount rate is calculated as 3.05% by using compound interest formulae. The discount rate (*i*) is assumed constant over the time period and is used to project the cost estimates for the year 2000.

|            | Health Effect           | Unit cost<br>(\$/d in 1990) | Unit cost<br>(\$/d in 2000) |
|------------|-------------------------|-----------------------------|-----------------------------|
|            | Non specific            | 279                         | 377                         |
|            | Respiratory             | 276                         | 373                         |
| Cancer     | Digestive               | 182                         | 246                         |
|            | Urinary                 | 170                         | 230                         |
|            | Nervous system          | 599                         | 809                         |
|            | Leukaemia               | 328                         | 443                         |
|            | Others                  | 196                         | 265                         |
|            | Asthma                  | 0.21                        | 0.28                        |
|            | Digestive problems      | 11.1                        | 15                          |
|            | Headache*               | 0.033                       | 0.052                       |
| Non-cancer | Non fatal heart attack* | 258                         | 405                         |
|            | Hyper tension*          | 0.95                        | 1.49                        |
|            | Non fatal stroke*       | 189.3                       | 297                         |

Table 6.11. Healthcare cost of cancer and non-cancer illnesses

\*Values in 1985

$$F = P \cdot (1+i)^n$$

(6.18)

where

F = Future cost;

P = Present cost: and

n = Number of years

The human health costs are provided as daily treatment estimates. The population cancer risk estimates can be multiplied with any particular cancer cost value to calculate the cancer risk. The non-cancer risk is estimated in terms of *HI* and it does not provide any information regarding probability of illness (a major deficiency of the use of this measure for risk assessment!). For non-cancer risk cost estimation, *HI*, can be related to qualitative estimates like low, medium and high as explained in section 6.2.1. These qualitative terms can be expressed by fuzzy numbers and can be added into other normalised cost estimates.

#### 6.4.4. ECOLOGICAL DAMAGE COST

Ecological damage due to disposal of toxic chemicals can affect commercial finfish and shellfish. The risk estimated to the whole ecosystem is assumed to be equal to the risk posed to the commercial fish. The ecological damage cost can be estimated by the following relationship:

Ecological damage cost (\$/day) = Risk × Total fish (kg/day) × Cost of fish (\$/kg) (6.20)

where risk is the probability of adverse effects. Incorporating health care costs and ecological damage costs into economic damage assessment may be perceived to be double counting. The reasoning might be that if economic damage is due to prohibition in commercial fishing, people are not exposed to contaminated fish. However, it is important to recognize the distinction between physically enduring an illness and paying for health care. The rationale behind considering health care costs with ecological damage is that the economic burden is one that is borne in addition to the pain and suffering of illness (U.S. EPA, 1993). For any particular discharge scenario all these environmental costs are accumulated to make a trade off between cost and risk.

#### 6.4.5. NON WATER QUALITY ENVIRONMENTAL DAMAGE COST

The elimination or reduction of water pollution may aggravate other environmental problems, which include air pollution (e.g., greenhouse gases, ozone depletion, acidification etc.), and solid waste disposal. The U.S. EPA (2000b) has evaluated the effects, which include air pollution, energy consumption, solid waste generation and management and affecting consumptive water use. The costs of these environmental effects are not included in this research.

#### 6.5. RISK MANAGEMENT - A PROPOSED METHODOLOGY

In the previous and current chapters, a step by step methodology is developed for estimating the fate of drilling waste discharges in the marine environment and its related ecological and human health risks. The estimated risks can be linked to various treatment technology options as well as economical damages and treatment costs. A framework for applying the various models is illustrated in Figure 6.15. This framework can be followed to make a risk-cost tradeoff analysis for various available alternatives.



Figure 6.15. Proposed research framework for risk management of drilling waste discharges

Various discharge scenarios based on synthetic base fluid attached to the drill cuttings can be defined to determine the pollutant loading rates for contaminants present in drilling waste. The contaminant fate models were used to estimate the predicted environmental concentrations of pollutants in water column and sediment pore water. The ecological and human health risk assessments were performed based on the exposure concentration. The ecological and human health risks were combined to determine the total environmental risk posed by the drilling waste discharges. The risk management methodology using fuzzy composite programming was used to determine the best drilling waste discharge scenario.

The research framework shown in Figure 6.15 is applied to a hypothetical case study on the East Coast of Canada in the next chapter. Various discharge scenarios based on base fluid percent attached to the drill cuttings were selected to investigate the best discharge option from minimum environmental risk, maximum cost saving and technical feasibility points of view.

## Chapter 7

## RISK MANAGEMENT OF DRILLING WASTE DISCHARGES IN THE MARINE ENVIRONMENT -A HYPOTHETICAL CASE STUDY

This chapter deals with a hypothetical case study of an oilfield on the East Coast of Canada. The models developed in the previous chapters are integrated to show the significance of adopting a holistic risk management methodology for the selection of the best management alternative or a discharge scenario for drilling waste disposal in the marine environment.

A risk-based methodology framework developed was used for a hypothetical case study in this chapter. Firstly, some basic information about the oilfield developments in the Grand Banks is presented. Various discharge scenarios representing management alternatives are selected based on regulatory discharge requirements and efficiency of the state-of-the-art solid control devices. Based on these selected discharge scenarios, the loading rates for contaminants present in the drilling waste stream are estimated. The fate models developed in Chapter 3 were employed in estimating the *PEC* in the water column and pore water. In the next step, the ecological and human health risk assessment models were used for risk characterization, which are based on estimated exposure concentrations (*EC*). In the end, a tradeoff analysis was performed to decide the best management alternative for the discharge of drilling waste from risk reduction, cost saving and technical feasibility viewpoints.

## 7.1. INTRODUCTION

The Grand Banks is one of the biggest offshore oil exploration and development sites in Canada. These fields contain huge quantities of natural gas reserves. More than 115 wells have been drilled in the Grand Banks up to year 2000 and they include crude oil reserves of more than 2-billion bbl capacity. The Canada-Newfoundland Offshore Petroleum Board (CNOPB) has estimated reserves of 2.1 billion bbl of crude oil, 1.640 billion bbl of natural gas. and 413 million bbl of natural gas liquid (NGL) on the Grand Banks (CNOPB, 2001).

The Grand Banks are situated to the southeast of Newfoundland covering 93,200 km<sup>2</sup>. The Grand Banks are the part of the North American continental shelf in the Atlantic Ocean (see Figure 7.1). It is an international fishing ground, which extends 560 km northto-south and 675 km east-to-west. The Grand Banks average water depth is 55 m, but at many places reaches to 180 m. The cold Labrador Current and the relatively warm Gulf Stream meet in the vicinity of the Grand Banks which often causes heavy fog. The mixing of the cold and warm waters produces favorable conditions for the growth of plankton, which is an essential element in the foodweb of commercial fish (CNOPB. 2001).

Hibernia was the first oilfield on the Grand Banks. It started production with a capacity of approximately 175.000 bbl/day in 1997. The Hibernia oilfield has estimated reserves of 884 million bbl crude oil, 247 billion bbl of natural gas and 145 million bbl of NGL. A gravity-based production system (GBS) is used at the Hibernia oilfield (CNOPB, 2001). The second important offshore oil project on the Grand Banks is Terra Nova. The Terra Nova oilfield has an estimated 406 million bbl of oil reserves. It will start oil production in 2002. The Terra Nova development project is using a floating production and offloading system (FPSO).



Figure 7.1. Grand Banks and the oilfields on the East Coast of Canada (not drawn to scale)

A new development on the Grand Banks is the White Rose field, which has estimated reserves of 250 million bbl. This project is in the beginning of the permitting process for development, with a production start-up target in year 2004. Other structures in the basin include Hebron-Ben Nevis and Riverhead, which are under exploration or appraisal as potential projects for future developments (CNOPB, 2001).

Other than oil exploration/production, the extensive use of the area by trawler fleets has led to overfishing which caused depletion in fish production. The Grand Banks fish resource includes cod, haddock, rosefish, various types of flatfish, herring, and mackerel etc. In 1977, Canada extended its fishing areas within 370 km of its shores, which includes most of the Grand Banks (Britannica, 2001).

#### 7.2. A HYPOTHETICAL OILFIELD ON THE EAST COAST OF CANADA

A hypothetical oilfield is considered on the Grand Banks of Newfoundland. It is approximately 35 km southeast of the Hibernia project and covers an area of 67 km<sup>2</sup>. The field is located in water approximately 95 m deep.

Dnilling of approximately 32 wells is associated with this oilfield, which results in substantial discharges of drilling wastes. Dnilling wastes consist primarily of the spent muds, rock and a variety of additives, which are generated during drilling. The elements of greatest concern in these wastes are base fluid, heavy metals and formation oil, although the cuttings themselves are also a concern because of their smothering effect on the benthic community. The environmental impact statement indicates that drilling wastes will be discharged over approximately 67 km<sup>2</sup> of the Project area. Even after the treatment of drilling wastes, over 1.400 m<sup>3</sup> of base oil will be released into the marine environment during the life of the project. The operators are committed to a minimum-release policy by using state-of-theart technology to control potentially harmful discharges. They have proposed the use of technology that reduces the oil discharged in dry cuttings to a 7 - 8% ROC as against the currently regulated 15% on the East Coast.

Major finfish and invertebrates stocks have changed drastically in the last 15 years in Newfoundland waters. Traditionally cod used to dominate catches, but in the last few years cod stocks have declined and other commercial species became important. By 1994 the catch statistics pertaining to these species have changed appreciably. The five most important species in the hypothetical study area, based on total landings at Newfoundland ports are: *stimpson surf clam, snow crab, lceland scallops, skate and redfish.* The Terra Nova environmental impact assessment report (1996) has also summarized the names of various commercial species found in different parts of the Grand Banks. The Newfoundland port landings of these species from the study area were *capelin, squid, herring* and *mackerel.* The total quantity of commercial landings to Newfoundland ports was estimated to be 2110 tons (Table 7.1).

#### 7.3. CONTAMINANT FATE MODELING

The risk-based decision methodology developed in this research was applied to the hypothetical case study of this oilfield. Some necessary assumptions and simplifications were made to fill in the missing information and streamline the complexity of a real world problem. Based on the available information, following necessary assumptions were made:

| Species<br>(Grand Bank) | Quantity<br>(tons) | Species<br>(Study area)        | Quantity<br>(tons) |
|-------------------------|--------------------|--------------------------------|--------------------|
| Clam                    | 11,092             | Squid                          | 95                 |
| Redfish                 | 1,495              | Herring                        | 973                |
| Skate                   | 1,940              | Mackerel                       | 5                  |
| Snow crab               | 4,742              | Capelin                        | 1,037              |
| Iceland scallop         | 4,033              | Total commercial               |                    |
| All other species       | 1,100              | fish landed at<br>Newfoundland | 2,110              |
| Total                   | 24,402             | ports                          |                    |

Table 7.1. Total landings of each species in Newfoundland (Terra Nova, 1996)

- 1. The hypothetical oilfield consists of approximately 32 wells, which encompass an affected area of 67 km<sup>2</sup>. The wells were assumed to be uniformly distributed over that area. The affected impact area ( $A_w$ ) for a single well was 2.09 km<sup>2</sup>. The radius of impact area (R) for a single well was approximately 816 m. The depth of water column was 95 m.
- The disposed drilling waste was assumed to be completely mixed in the impact area under steady state conditions.

- 3. The wells were assumed to be drilled only by SBFs. The esters are the most commonly used and environmentally benign synthetic based drilling fluids (U.S. EPA, 2000a). The physico-chemical characteristics of esters are more extensively reported in the literature than the other synthetic drilling fluids, and these were used in this case study (Chapters 2 and 3).
- Pollutant loading rates of heavy metals, base fluid (ester) and organic priority pollutants were calculated based on deep development model well data as suggested by U.S. EPA (2000a, b).
- For human health risk assessment, it was assumed that only St. John's and its suburban population (165.000) are exposed to all contaminated fish that is caught from the affected area.

## 7.3.1. ESTIMATION OF POLLUTANT LOADING RATES

To determine the pollutant concentration in the water column and pore water, the total pollutant loading rates for each discharge option were estimated. For this purpose, the U.S. EPA (2000a, b) assumed that the suspended solids (SS) concentration is attributed to drill cuttings in the waste stream. The density of dry cuttings is reported to be 2.57 kg/L in the literature (U.S. EPA, 2000a). The total volume of dry cuttings for a development well is approximately 140 m<sup>3</sup> (U.S. EPA, 1999a). This volume includes borehole volume and an additional 7.5% wash out from the walls of the wells.
Five discharge scenarios were selected as potential management alternatives for drilling waste disposal in a marine environment. These alternatives include 10% (current practice in Gulf of Mexico). 8.5% (obtainable with the current technology). 7.0%. 5.5% and 4.0% (BAT proposed). These alternatives represent the percentage of base fluid attached to the wet cuttings. Table 7.2 provides a detailed step-by-step approach for estimating the amounts of *SS* and base fluid expected to be released into the environment for the 10% discharge option.

The SBF consists of 47% base fluid (ester), 33% barte and 20% water. The density of drilling waste depends on the amount of base fluid attached to the drill cuttings. The major components of drilling wastes are dry drill cuttings, base fluid, barite and water. The weights and volumes of these components were calculated based on ester percentage attached to the cuttings. The volume and weight of dry drill cuttings were estimated using borehole volume and density of dry cuttings.

The quantities of each component were estimated to determine the weight and volume of total drilling waste for a given discharge option. The total volume of base fluid discharged for the 10% discharge option was estimated to be 57 m<sup>3</sup>. The amount of formation oil was estimated based on an assumption that it is 0.2% volume fraction of the SBF, which is 140 kg for the 10% discharge option. The weight of barite disposed under the 10% discharge option is approximately 32.000 kg. The barite discharged in the marine environment contributes trace heavy metals, which pose a threat to ecological entities.

| Items  | Water  | Base fluid                            | Barite                                | DC      | Form. oil |
|--|--|---------------------------------------|---------------------------------------|---------|-----------|
| % SBF  | 20   | 47                                    | 33                                    | N/A     | N/A       |
| Density (kg/m <sup>3</sup> )                             | 1000   | 792                                   | 4240                                  | 2575    | 832       |
| DC (includes 7.5% washout, m                             | (Are   | 1                                     | 37.2                                  |         |           |
| DC (kg) = $137.2 \text{ (m}^3) \cdot 2575 \text{ (m}^3)$ | (kg/m <sup>3</sup> )   |                                       |                                       | 35      | 3,150     |
| Items  | Formulation  |                                       |                                       | Sce     | nario 1   |
| Base fluid attached                                      | After treatment  | the % on wet cutting                  | zs                                    | 1       | 0.0%      |
| SBF fraction to the TW                                   | (Fraction base fl  | uid attached) · TW/                   | 0.47                                  | (       | .213      |
| TW (kg)  | DC/(1 - SBF fra  | ction to the TW)                      |                                       | 45      | 2,935     |
| Water (kg)   | 0.2 · (SBF fracti  | on to the TW) · TW                    |                                       | 1       | 9,100     |
| Water (m <sup>3</sup> )                                  | Water (kg)/1000  | 1                                     |                                       |         | 19.1      |
| Barite (kg)  | 0.33 · (SBF fract  | tion to the TW) · TW                  | V                                     | 3       | 1.835     |
| Barite (m <sup>3</sup> )                                 | Barite (kg)/4240   |                                       |                                       | 7.51    |           |
| Base fluid (kg)  | (Base fluid attac  | hed) · TW                             |                                       | 45,294  |           |
| Base fluid (m <sup>3</sup> )                             | Base fluid (kg)/7  | 192                                   |                                       | 57.2    |           |
| SBF (m <sup>3</sup> )                                    | ∑ (Water, Barite   | , Base fluid) 3 (m)                   |                                       | 84.0    |           |
| SBF (kg)   | $\Sigma$ (Water, Barite  | , Base fluid) (kg)                    |                                       | 96,230  |           |
| SBF (kg/m <sup>3</sup> )                                 | SBF (kg)/SBF (r  | n <sup>3</sup> )                      |                                       | 1,146   |           |
| Form. oil (m <sup>3</sup> )                              | 0.002 · SBF (m <sup>3</sup> )                                    | )                                     |                                       | 0       | .168      |
| Form. oil (kg)   | Form. oil (m <sup>3</sup> ) ·                                    | 832                                   |                                       |         | 140       |
| Total oil (m <sup>3</sup> )                              | Σ (Form. oil, Ba   | se fluid) <sup>3</sup> <sub>(m)</sub> |                                       | :       | 57.4      |
| Total oil (kg)   | $\Sigma$ (Form. oil, Ba  | se fluid) (kg)                        |                                       | 4       | 5,433     |
| Total drilling waste (m <sup>3</sup> )                   | Σ (DC, Water, B  | arite, Base fluid, Fo                 | orm. oil) <sup>3</sup> <sub>(m)</sub> | 2       | 21.3      |
| Total drilling waste (kg)                                | Σ (DC, Water, B  | arite, Base fluid, Fo                 | orm. oil) (kg)                        | 45      | 3,075     |
| Drilling waste density (kg/m3)                           | Total drilling waste (kg)/Total drilling waste (m <sup>3</sup> ) |                                       | 2                                     | 047     |           |
| Volume fraction of SBF                                   | SBF (m <sup>3</sup> )/Total of                                   | drilling waste (m <sup>3</sup> )      |                                       | 0       | .380      |
| Weight fraction of barite                                | Barite (kg)/Total  | drilling waste (kg)                   |                                       | 0.      | 0703      |
| Total suspended solids (kg)                              | $\Sigma$ (Barite, DC) a  | a)                                    |                                       | 384,830 |           |

Table 7.2. A methodology for estimating of drilling waste different constituents (10% attached base fluid).

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The density of drilling waste for the 10% discharge option was estimated to be 2.05 kg/L. The volume fraction of SBF and the weight fraction of barite in the total drilling waste were calculated for estimating pollutant loading rates (*E*). A similar procedure was repeated for other discharge options. The weights and volumes estimated for different components of drilling waste are summarized in Table 7.3.

The drilling waste density increases with a decrease in the percentage base fluid attached to the drill cuttings. The drilling waste densities estimated are 2.05, 2.12, 2.17, 2.25, and 2.33 kg/L for the 10%, 8.5%, 7%, 5.5% and 4% discharge options, respectively. The volumes of base fluid discharged with cuttings vary from 57 to 20 m<sup>3</sup> for the 10% to 4% discharge options, respectively. The amounts of barite discharged ranged from 32.000 to 11.000 kg, approximately. The total weight of drilling waste varies from 453.000 to 386.000 kg for 10% to 4% discharge options, respectively. There is not an appreciable difference in total quantity of drilling waste because the total weight of dry cuttings is the same for all discharge options.

The amounts of heavy metals depend on the quantity of barite discharged for each option. The effluent concentrations of heavy metals in drilling waste were estimated based on the weight fractions of metals in the barite, whereas organic pollutant concentrations depend on the volume fraction of formation oil present in the SBF. The volume fraction of SBF was calculated by dividing the volume of SBF with the total volume of drilling waste for a given discharge option. The estimated volume fractions of SBFs are 0.378, 0.331, 0.282, 0.229 and 0.173 for the 10%, 8.5%, 7%, 5.5% and 4.0% discharge options. respectively. Similarly, the banic weight fractions were estimated by dividing the weight of the banic by the total weight of drilling waste for a given discharge option. The estimated barite weight fractions are 0.0702. 0.0597, 0.0491, 0.0360 and 0.0281 for the 10%, 8.5%, 7%, 5.5% and 4.0% discharge options, respectively.

The SBF volume and bartie weight fractions in the drilling waste stream were used in estimating the organic pollutant and heavy metals concentrations ( $C_{e}$ ) in the drilling waste, respectively. Table 7.4 describes the mass balance equations used for estimating the  $C_{e}$ . A summary of SBF volume fraction and bartie weight fractions in drilling waste for different discharge options is given in Table 7.4.

The typical amounts of trace heavy metals (As, Cd, Cu, Cr, Hg, Pb, Ni and Zn) in mg per kg of barite are reported in Chapter 2 (U.S. EPA, 1999a). These values were used to estimate the concentrations of different heavy metals in drilling waste. Similarly, the naphthalene concentration in formation oil is required to estimate its effluent concentration ( $C_d$ ) in drilling waste. The effluent concentrations for different discharge options were estimated based on the formulation given in Table 7.4. The heavy metals in mg/kg of barite and naphthalene in mg/ml of formation oil were used in this formulation (Table 2.5). The pollutant effluent concentrations in the drilling waste stream were estimated for various discharge options (Table 7.5). The ester (base fluid) concentration in the drilling waste exas determined by dividing the total weight of ester discharged with the total volume of drilling waste estimated for that discharge option (see Table 7.3).

| Base fluid attached with cu | Base fluid attached with cuttings (% of TW) |         | 8.5     | 7.0     | 5.5     | 4.0     |
|-----------------------------|---|---------|---------|---------|---------|---------|
| SBFs fraction in total weig | ht (TW)                                     | 0.213   | 0.181   | 0.149   | 0.117   | 0.085   |
|                             | Weight (kg)                                 | 353,150 | 353,150 | 353,150 | 353,150 | 353,150 |
| Dry cuttings (DC)           | Volume (m <sup>3</sup> )                    | 137.2   | 137.2   | 137.2   | 137.2   | 137.2   |
| (including 7.5% washout)    | Density (kg/m <sup>3</sup> )                | 2,575   | 2,575   | 2,575   | 2,575   | 2,575   |
|                             | Weight (kg)                                 | 452,935 | 430,956 | 414,795 | 399,803 | 385,856 |
| Total weight (TW)           | Volume (m <sup>3</sup> )                    | 221.1   | 205.2   | 179.5   | 178.0   | 165.8   |
|                             | Density (kg/m <sup>3</sup> )                | 2,048   | 2,100   | 2,310   | 2,246   | 2,327   |
|                             | Weight (kg)                                 | 45,294  | 36,631  | 29,036  | 21,989  | 15,434  |
| Ester                       | Volume (m <sup>3</sup> )                    | 57.2    | 46.2    | 36.7    | 27.8    | 19.5    |
|                             | Density (kg/m <sup>3</sup> )                | 792     | 792     | 792     | 792     | 792     |
|                             | Weight (kg)                                 | 31,835  | 25,720  | 20,387  | 15,439  | 10,837  |
| Barite                      | Volume (m <sup>3</sup> )                    | 7.51    | 6.06    | 4.81    | 3.64    | 2.55    |
|                             | Density (kg/m <sup>3</sup> )                | 4,240   | 4,240   | 4,240   | 4,240   | 4,240   |
| and the second second       | Weight (kg)                                 | 19,100  | 15,588  | 12,356  | 9,357   | 6,568   |
| Water                       | Volume (m <sup>3</sup> )                    | 19.1    | 15.8    | 12.5    | 9.4     | 6.6     |
|                             | Density (kg/m <sup>3</sup> )                | 1000    | 1000    | 1000    | 1000    | 1000    |
|                             | Weight (kg)                                 | 139.7   | 113.2   | 89.7    | 68.0    | 47.7    |
| Form. Oil (0.2%)            | Volume (m <sup>3</sup> )                    | 0.17    | 0.14    | 0.11    | 0.08    | 0.06    |
|                             | Density (kg/m <sup>3</sup> )                | 832     | 832     | 832     | 832     | 832     |
| ODE                         | Weight (kg)                                 | 96,230  | 77,939  | 61,778  | 46,785  | 32,839  |
| SBFs                        | Volume (m <sup>3</sup> )                    | 84.0    | 68.1    | 53.9    | 40.9    | 28.7    |
| (Ester + barite + water)    | Density (kg/m <sup>3</sup> )                | 1,146   | 1,146   | 1,146   | 1,146   | 1,146   |
| Total oil                   | Weight (kg)                                 | 45,433  | 36,745  | 29,125  | 22,057  | 15,482  |
| (Ester + form. oil)         | Volume (m <sup>3</sup> )                    | 57.4    | 46.4    | 36.8    | 27.8    | 19.6    |
|                             | Density (kg/m <sup>3</sup> )                | 792     | 792     | 792     | 792     | 792     |
| T                           | Weight (kg)                                 | 453,075 | 431,069 | 414,885 | 399,871 | 385,904 |
| I otal drilling waste       | Volume (m <sup>3</sup> )                    | 221.4   | 205.3   | 191.2   | 178.0   | 165.9   |
| (including form. oil)       | Density (kg/m <sup>3</sup> )                | 2,047   | 2,100   | 2,170   | 2,246   | 2,327   |
|                             | Density (kg/L)                              | 2.05    | 2.10    | 2.17    | 2.25    | 2.33    |
|                             |   |         |         |         |         |         |

Table 7.3. Estimation of drilling waste constituents for different discharge scenarios.

Table 7.4. Estimation of pollutants effluent concentrations in drilling waste stream.

Organic pollutants:

Vol. fraction of form. oil = Vol. of form. oil/Vol. of SBFs = 0.002; and Vol. fraction of SBFs = Vol. of SBFs/Vol. of total drilling waste.

C<sub>E</sub>, Effluent conc. (mg/L) = (mg of pollutant/ml of form. oil) · (Vol. fraction of form. oil) · (Vol. fraction of SBFs) · (1000 ml/L)

Heavy metals:

Wt. fraction of barite = (Wt. of barite/Total drilling waste)

CE, Effluent conc. (mg/L) = (mg/kg of barite) · (Wt. fraction of barite) · (Drilling waste density, kg/L)

| Attached base fluid (%)          | 10.0   | 8.5    | 7.0    | 5.5    | 4.0    |
|----------------------------------|--------|--------|--------|--------|--------|
| Vol. fraction of SBFs            | 0.378  | 0.331  | 0.282  | 0.229  | 0.173  |
| Wt. fraction of barite           | 0.0702 | 0.0597 | 0.0491 | 0.0386 | 0.0281 |
| Density of drilling waste (kg/L) | 2.05   | 2.10   | 2.17   | 2.25   | 2.33   |

## 7.3.2. PREDICTED ENVIRONMENTAL CONCENTRATION (PEC)

The effluent concentrations ( $C_e$ ) of pollutants in drilling waste were now used for estimating the loading rate (E) for each discharge scenario. The regression models developed in Chapter 3 were used to determine the predicted environmental concentrations (*PEC*) in the water column and sediment pore water. The *PEC* represents the highest 95<sup>th</sup> percentile of water column and pore water concentration cumulative distributions.

First, the total quantities of pollutants were estimated by multiplying the total volume of the drilling waste with the  $C_E$  of each pollutant present in the drilling waste stream for a given option. The actual time of drilling may vary from 13 to 44 days for different types of wells (U.S. EPA, 2000a). An average time of one-month is assumed to estimate the loading rates for each pollutant. The loading rates for all pollutants are provided in mg/hr, except for ester, which is given in kg/hr. Table 7.6 provides a summary of pollutant loading rates estimated for different discharge scenarios.

| Pollutants  | BF    | $C_{E_1}$ Concentration of pollutants in drilling waste (mg/L) before disposal for various discharge scenarios |         |         |         |         |  |  |
|-------------|-------|--|---------|---------|---------|---------|--|--|
|             |       | 10.0%  | 8.5%    | 7.0%    | 5.5%    | 4.0%    |  |  |
| As          | 1.000 | 1.0133   | 0.8893  | 0.7571  | 0.6156  | 0.4639  |  |  |
| Cd          | 0.994 | 0.1570   | 0.1378  | 0.1173  | 0.0954  | 0.0719  |  |  |
| Cr          | 0.993 | 34.2510  | 30.0620 | 25.5908 | 20.8079 | 15.6796 |  |  |
| Cu          | 0.830 | 2.6687   | 2.3423  | 1.9940  | 1.6213  | 1.2217  |  |  |
| Hg          | 0.850 | 0.0143   | 0.0125  | 0.0107  | 0.0087  | 0.0065  |  |  |
| Ni          | 0.990 | 1.9266   | 1.6910  | 1.4395  | 1.1704  | 0.8820  |  |  |
| Pb          | 0.951 | 5.0092   | 4.3966  | 3.7427  | 3.0432  | 2.2931  |  |  |
| Zn          | 0.946 | 28.6139  | 25.1143 | 21.3790 | 17.3833 | 13.0990 |  |  |
| Naphthalene | 1.000 | 1.0800   | 0.9479  | 0.8069  | 0.6561  | 0.4944  |  |  |
| *Ester      | 1.000 | 0.2027   | 0.1785  | 0.1519  | 0.1236  | 0.0931  |  |  |

Table 7.5. Concentrations of pollutants in drilling waste for different discharge scenarios.

\*Values are in kg/L

The impact radius (*R*) of 816 m was used to estimate the *PEC* from regression models developed in Chapter 3. These regression models were used to estimate the *PEC* in the water column and pore water. Table 7.7 provides a summary of mean *PEC* values (of 95<sup>th</sup> percentile) in the water column. The background mean sea level concentrations are also provided for a comparison.

| Pollutants  | 10%     | 8.5%   | 7.0%   | 5.5%   | 4.0%   |
|-------------|---------|--------|--------|--------|--------|
| As          | 310.5   | 253.6  | 201.0  | 152.3  | 106.9  |
| Cd          | 48.1    | 39.3   | 31.2   | 23.6   | 16.6   |
| Cr          | 10495.1 | 8573.3 | 6795.6 | 5146.4 | 3612.3 |
| Cu          | 817.7   | 668.0  | 529.5  | 401.0  | 281.5  |
| Hg          | 4.4     | 3.6    | 2.8    | 2.1    | 1.5    |
| Ni          | 590.4   | 482.3  | 382.3  | 289.5  | 203.2  |
| Pb          | 1534.9  | 1253.8 | 993.9  | 752.7  | 528.3  |
| Zn          | 8767.8  | 7162.3 | 5677.1 | 4299.4 | 3017.8 |
| Naphthalene | 330.9   | 270.3  | 214.3  | 162.3  | 113.9  |
| *Ester      | 62.3    | 50.9   | 40.3   | 30.5   | 21.4   |

Table 7.6. Pollutant loading rates, E (mg/hr) for different discharge scenarios.

\*Ester (values are in kg/hr)

The estimated concentrations of pollutants in the water column are too low to cause any damage to the ecological community. The concentration of ester in the water column varies from  $2.56 \times 10^{-3}$  to  $8.79 \times 10^{-4} \mu g/L$  for the 10% to 4% discharge options, respectively. Similarly, for other pollutants, the *PECs* are lower than the corresponding background concentrations with a difference of orders of magnitudes. By comparing water column *PEC* with background level concentrations it can be concluded that water column exposure is not a real threat to the ecological entities. This conclusion is in harmony with the U.S. EPA findings (2000a, b).

| Pollutants  | Mean sea<br>concentration | 10%      | 8.5%     | 7.0%     | 5.5%     | 4.0%     |
|-------------|---------------------------|----------|----------|----------|----------|----------|
| As          | 3.70E-2                   | 1.62E-07 | 1.32E-07 | 1.04E-07 | 7.91E-08 | 5.54E-08 |
| Cd          | 4.40E-2                   | 1.68E-08 | 1.37E-08 | 1.08E-08 | 8.20E-09 | 5.75E-09 |
| Cr          | 7.90E-2                   | 6.13E-07 | 5.00E-07 | 3.96E-07 | 3.00E-07 | 2.10E-07 |
| Cu          | 7.20E-1                   | 1.10E-07 | 8.94E-08 | 7.08E-08 | 5.36E-08 | 3.76E-08 |
| Hg          | 5.40E-2                   | 1.23E-12 | 1.01E-12 | 7.98E-13 | 6.03E-13 | 4.25E-13 |
| Ni          | 4.00E-1                   | 2.15E-07 | 1.76E-07 | 1.40E-07 | 1.06E-07 | 7.43E-08 |
| Pb          | 4.80E-1                   | 2.40E-07 | 1.96E-07 | 1.55E-07 | 1.17E-07 | 8.24E-08 |
| Zn          |                           | 6.51E-07 | 5.32E-07 | 4.22E-07 | 3.19E-07 | 2.24E-07 |
| Naphthalene | 7.00E-1                   | 1.02E-14 | 8.31E-15 | 6.60E-15 | 5.00E-15 | 3.52E-15 |
| Ester       | -                         | 2.56E-03 | 2.09E-03 | 1.65E-03 | 1.25E-03 | 8.79E-04 |

Table 7.7. Mean values of PEC (µg/L) in the water column for different discharge scenarios.

The other exposure to marine organisms is the pore water concentration. The higher density of drilling waste and the affinity of pollutants to attach themselves to sediments are the major causes of higher concentrations in pore water. The mean values of *PEC* in pore water for ester are 24.6, 20.1, 16.0, 12.1 and 8.5 mg/L for the 10%, 8.5%, 7%, 5.5% and 4.0% discharge options, respectively (Table 7.8). The pore water *PECs* for naphthalene and Hg are the only two pollutants whose concentrations are below background level concentrations. For all other pollutants considered, the pore water *PEC* values are higher than background concentrations. The mean *PEC* values for As vary from 0.32 to 0.11 mg/L for 10% to 4% discharge options. The mean *PEC* values for other pollutants are summarized in Table 7.8.

| Pollutants   | Mean sea<br>concentration | 10%        | 8.5%       | 7.0%       | 5.5%       | 4.0%       |
|--------------|---------------------------|------------|------------|------------|------------|------------|
|              |                           | 3.20E+02   | 2.61E+02   | 2.07E+02   | 1.57E+02   | 1.10E+02   |
| As           | 3.70E-2                   | (2.03E+00) | (1.64E+00) | (1.25E+00) | (9.49E-01) | (6.53E-01) |
| ~            | 1.405.2                   | 2.32E+02   | 1.90E+02   | 1.50E+02   | 1.14E+02   | 8.00E+01   |
| Cd           | 4.40E-2                   | (4.58E+00) | (3.81E+00) | (2.99E+00) | (2.20E+00) | (1.49E+00) |
| -            | 5.00F.0                   | 2.87E+03   | 2.35E+03   | 1.86E+03   | 1.41E+03   | 9.89E+02   |
| Cr           | 7.90E-2                   | (4.03E+01) | (3.26E+01) | (2.58E+01) | (1.88E+01) | (1.33E+01) |
| 0            | 7.005.1                   | 1.70E+03   | 1.39E+03   | 1.10E+03   | 8.32E+02   | 5.84E+02   |
| Cu           | 7.20E-1                   | (3.39E+01) | (2.71E+01) | (2.09E+01) | (1.55E+01) | (1.02E+01) |
|              | 5 405 0                   | 5.28E-05   | 4.32E-05   | 3.42E-05   | 2.59E-05   | 1.83E-05   |
| Hg           | 5.40E-2                   | (2.51E-07) | (2.04E-07) | (1.62E-07) | (1.19E-07) | (8.52E-08) |
| NE           | 1005.1                    | 2.83E+03   | 2.32E+03   | 1.84E+03   | 1.39E+03   | 9.77E+02   |
| NI           | 4.00E-1                   | (7.00E+01) | (5.49E+01) | (4.36E+01) | (3.22E+01) | (2.24E+01) |
| DF           | 4 80E 1                   | 3.20E+03   | 2.61E+03   | 2.07E+03   | 1.57E+03   | 1.10E+03   |
| FD           | 4.60E-1                   | (5.76E+01) | (4.51E+01) | (3.43E+01) | (2.58E+01) | (1.75E+01) |
| 7-           |                           | 3.46E+03   | 2.82E+03   | 2.24E+03   | 1.70E+03   | 1.19E+03   |
| Zn           | 2                         | (4.74E+01) | (3.85E+01) | (3.05E+01) | (2.23E+01) | (1.58E+01) |
| Neghthelene  | 7.00F 1                   | 5.16E-07   | 4.21E-07   | 3.34E-07   | 2.53E-07   | 1.78E-07   |
| rvapninalene | 7.00E-1                   | (2.74E-09) | (2.16E-09) | (1.70E-09) | (1.26E-09) | (8.18E-10) |
| Entre        |                           | 2.46E+04   | 2.01E+04   | 1.60E+04   | 1.21E+04   | 8.48E+03   |
| Ester        |                           | (4.19E+01) | (3.39E+01) | (2.41E+01) | (1.89E+01) | (1.21E+01) |

Table 7.8: Means and standard deviations of pore water PEC (µg/L) for different pollutants in discharge scenarios.

\* The values in parenthesis show standard deviation

The standard deviations of *PEC* estimates for pore water are also reported in Table 7.8 for all contaminants. The values are reported in parenthesis. These values account for uncertainty in the pore water *PEC* estimates. The uncertainties in the coefficients of the

The estimated *PEC* values (Table 7.8) were multiplied with p and the corresponding *BF* to determine the actual exposure concentrations (*EC*). Table 7.9 summarizes the mean *EC* of pollutants for various discharge scenarios. The exposure concentrations calculated were used for ecological and human health risk assessments in the subsequent analyses.

| Pollutants  | 10%   | 8.5%  | 7.0%  | 5.5% | 4.0% |
|-------------|-------|-------|-------|------|------|
| As          | 0.23  | 0.19  | 0.15  | 0.11 | 0.08 |
| Cd          | 0.17  | 0.14  | 0.11  | 0.08 | 0.06 |
| Cr          | 2.05  | 1.68  | 1.33  | 1.01 | 0.71 |
| Cu          | 1.01  | 0.83  | 0.66  | 0.50 | 0.35 |
| Hg          | 0.00  | 0.00  | 0.00  | 0.00 | 0.00 |
| Ni          | 2.01  | 1.65  | 1.31  | 0.99 | 0.70 |
| Pb          | 2.19  | 1.78  | 1.42  | 1.07 | 0.75 |
| Zn          | 2.35  | 1.92  | 1.52  | 1.16 | 0.81 |
| Naphthalene | 0.00  | 0.00  | 0.00  | 0.00 | 0.00 |
| Ester       | 17.68 | 14.45 | 11.50 | 8.70 | 6.10 |

Table 7.9. Mean exposure concentration in pore water (µg/L) for different discharge scenarios.

# 7.4. ECOLOGICAL RISK ASSESSMENT

The risk characterization of ecological entities is based on dose response relationships. The estimated dose or exposure is compared with the response for each contaminant, which were established in Chapter 4. The following specific steps were performed in the ecological risk assessment: The estimated PEC values (Table 7.8) were multiplied with p and the corresponding BF to determine the actual exposure concentrations (EC). Table 7.9 summarizes the mean EC of pollutants for various discharge scenarios. The exposure concentrations calculated were used for ecological and human health risk assessments in the subsequent analyses.

| Pollutants  | 10%   | 8.5%  | 7.0%  | 5.5% | 4.0% |
|-------------|-------|-------|-------|------|------|
| As          | 0.23  | 0.19  | 0.15  | 0.11 | 0.08 |
| Cd          | 0.17  | 0.14  | 0.11  | 0.08 | 0.06 |
| Cr          | 2.05  | 1.68  | 1.33  | 1.01 | 0.71 |
| Cu          | 1.01  | 0.83  | 0.66  | 0.50 | 0.35 |
| Hg          | 0.00  | 0.00  | 0.00  | 0.00 | 0.00 |
| Ni          | 2.01  | 1.65  | 1.31  | 0.99 | 0.70 |
| Pb          | 2.19  | 1.78  | 1.42  | 1.07 | 0.75 |
| Zn          | 2.35  | 1.92  | 1.52  | 1.16 | 0.81 |
| Naphthalene | 0.00  | 0.00  | 0.00  | 0.00 | 0.00 |
| Ester       | 17.68 | 14.45 | 11.50 | 8.70 | 6.10 |

Table 7.9. Mean exposure concentration in pore water (µg/L) for different discharge scenarios.

# 7.4. ECOLOGICAL RISK ASSESSMENT

The risk characterization of ecological entities is based on dose response relationships. The estimated dose or exposure is compared with the response for each contaminant, which were established in Chapter 4. The following specific steps were performed in the ecological risk assessment:

- The pore water exposure concentration (EC) for each contaminant was used as a representative dose for ecological entities (Table 7.9).
- The response of the ecological system was determined from the lowest 10<sup>th</sup> percentile value on the *PNEC* empirical distribution function. The mean (μ<sub>id</sub>) and standard deviation (σ<sub>id</sub>) of response for each pollutant are provided in Chapter 4. The response values of each pollutant were used separately to determine ecological risk estimates.
- Hazard or risk quotient (HQ/RQ) was calculated by dividing the exposure concentration distribution with the representative response distribution. HQ values greater than 1 represent adverse effects. If the value is less than 1, it means that at least 95% of ecological community is safe for 90% of the time (Sadiq et al., 2001d: Lenwood et al., 1998).
- 4. To estimate the probability of adverse effects according to the above criteria, the estimated HQ for each contaminant was converted into a corresponding risk value as suggested by Karman et al. (1996) and Thatcher et al. (1999) by the following relationship:

$$Risk = \int_{y=0}^{Loc RQ_1} \left[ \frac{1}{S_{\pi} \sqrt{2\pi}} e^{\frac{(y-X_{\pi})^2}{2S_{\pi}^2}} \right]$$
(7.1)

 $X_n$  = Average of the lognormally transformed data = 2.85:

 $S_n$  = Standard deviation of the lognormally transformed data = 1.74; and

y = Variable to describe the normal probability curve = Ln (HQ)

 The risks estimated for each pollutant were grouped together as independent events according to the methodology discussed in Chapter 4, i.e.,

$$Risk (A + B) = Risk (A) + Risk (B) - Risk (A) \cdot Risk (B)$$

$$(7.2)$$

Similarly.

$$Risk(A + B + C) = Risk(A) + Risk(B) + Risk(C) - Risk(A) \cdot Risk(B) - Risk(B)$$

$$Risk(C) - Risk(A) \cdot Risk(C) + Risk(A) \cdot Risk(B) \cdot Risk(C)$$
 (7.3)

And so on. Thatcher et al. (1999) and Jooste (2000) have adopted similar methodologies in the case of multiple stressors.

6. The risk estimated from equation 7.1 is in the form of an empirical distribution function (EDF). Therefore composite ecological risks estimated from the series of equations (e.g. equations 7.2 and 7.3) are also in the form of an empirical distribution function. The composite ecological risk values are reported in the form of minimum, maximum likely (mode, MLV) and maximum values. These risk parameters, i.e., minimum, MLV and maximum values were used to develop a triangular fuzzy number. Table 7.10 summarizes the minimum, MLV and maximum values for each discharge scenario, where these values represent the lowest 1%, mode and the highest 99% of the EDF (Jooste, 2001). The ecological risk varies over a wide range from 0.18 to 0.63 for the 10% discharge option, with an MLV of 0.26. It reduces to 0.04 to 0.33 for the 4% discharge option with an MLV of 0.13. The ecological risks estimated for different discharge scenarios are also compared in Figure 7.2.

| Discharge<br>Scenario | Min.   | MLV    | Max.   |
|-----------------------|--------|--------|--------|
| 10.0%                 | 0.1775 | 0.2602 | 0.6276 |
| 8.5%                  | 0.1366 | 0.2750 | 0.5542 |
| 7.0%                  | 0.1175 | 0.2324 | 0.5035 |
| 5.5%                  | 0.0678 | 0.1778 | 0.4244 |
| 4.0%                  | 0.0448 | 0.1323 | 0.3302 |

Table 7.10: Composite ecological risks estimated for various discharge scenarios.

The base of the triangular fuzzy number represents the uncertainty in the estimates. The apex of the triangle represents the most likely value. The highest uncertainty in the ecological risk estimates can be observed for the 10% discharge option and the least uncertainty is observed for the 4.0% discharge scenario.



Figure 7.2. Ecological risk estimates for various discharge scenarios.

### 7.5. HUMAN HEALTH RISK ASSESSMENT

Human health is the other major components of environmental risk assessment. Human health risk assessment involves exposure through contaminated fish caught from the impacted site. Human health risk consists of cancer and non-cancer risk estimates. The chronic daily intakes (*CDI*) were calculated for each contaminant individually. For cancer risk, it was multiplied by the slope factor (*SF*) of arsenic (the only proven human carcinogen in this study) to estimate the unit risk over the life span of a person. Similarly, for estimation of non-cancer risks, reference doses (*RJD*) were divided by estimated *CDI* for each contaminant to determine *HI*. In the end, the *HIs* for all contaminants were

summed up to determine the composite hazard index  $(HI_r)$ . The following steps were performed to estimate the cancer and non-cancer human health risks:

- The pore water exposure concentrations (EC) of each contaminant were employed to estimate the fish tissue concentrations, which were further used as human exposure concentrations through consumption of contaminated fish. The CDI was calculated for each contaminant separately (Chapter 5).
- The cancer unit risk was estimated for arsenic only. The slope factor (SF) was multiplied by CDI to estimate the unit risk.
- The reference doses (RfD) were used to estimate the non-cancer risks. The CDI was divided by corresponding RfD to estimate the HI for that contaminant.
- The HIs for individual contaminants were added to get the composite hazard index (HI<sub>c</sub>).
- $\sum HI = HI_c \tag{7.4}$

The cancer and non-cancer risk estimates are reported in Table 7.11. The minimum, MLV and maximum values for various discharge scenarios are reported. The minimum values correspond to the lowest 1% and maximum values correspond to the highest 99<sup>th</sup> percentile value of the estimates, whereas the MLV is the highest frequency value, which corresponds to the mode value (Jooste, 2001).

| Items                 | Parameter | 10.0%                    | 8.5%                   | 7.0%                   | 5.5%                   | 4.0%                   |
|-----------------------|-----------|--------------------------|------------------------|------------------------|------------------------|------------------------|
|                       | Min.      | 0.0065                   | 0.0053                 | 0.0043                 | 0.0033                 | 0.0024                 |
| Non-cancer            | MLV       | 0.0163                   | 0.0144                 | 0.0135                 | 0.0090                 | 0.0048                 |
| $(HI_c)$              | Max.      | 0.0856                   | 0.0730                 | 0.0624                 | 0.0435                 | 0.0326                 |
|                       | Min.      | $1.24 \times 10^{-16}$   | $9.15 \times 10^{-17}$ | $8.26 \times 10^{-17}$ | $6.39 \times 10^{-17}$ | $4.53 \times 10^{-17}$ |
| Cancer<br>(Unit risk) | MLV       | $5.54 \times 10^{-16}$   | $4.31 \times 10^{-16}$ | $5.18 \times 10^{-16}$ | $1.73 \times 10^{-16}$ | $1.87 \times 10^{-16}$ |
|                       | Max.      | 3.49 × 10 <sup>-15</sup> | $2.95 \times 10^{-15}$ | $2.35 \times 10^{-15}$ | $1.64 \times 10^{-15}$ | $1.16 \times 10^{-15}$ |

Table 7.11. Human health risk assessment for various scenarios.

The highest value of  $HI_c$  for the 10% discharge scenario is approximately estimated to be 0.09. This value is approximately 12 times lower than the safety level of 1. Similarly, the unit risk values are very small, i.e., lower than *I* in a million, which is conventionally used as a regulatory measure for human health risk assessment. Although all conditions represent safe situations from a human health risk viewpoint, the purpose of this study was to make a comparative evaluation of various discharge scenarios based on risk estimates. Therefore these values are used in the risk management section for comparing the different alternatives.

### 7.6. COST ESTIMATION

The second important component of risk management is the cost required to implement any given management alternative. The cost components considered in this study are treatment cost, cost of drilling fluid lost during discharge, ecological and human health damage costs (Chapter 6). To compare management alternatives or discharge options, the cost estimates are normalized over one year and values are reported in \$/day. The summary of cost estimates for various discharge scenarios is given in Table 7.12.

| Items (\$/day)                | Parameter   | 10.0%                                       | 8.5%  | 7.0%                           | 5.5%                             | 4.0%                                  |
|-------------------------------|-------------|---|---|--------------------------------|----------------------------------|---------------------------------------|
|                               | Min.        | 201.28                                      | 235.13  | 296.93                         | 355.58                           | 413.2                                 |
| Treatment                     | MLV         | 352.79                                      | 437.25  | 452.06                         | 540.48                           | 586.68                                |
|                               | Max.        | 704.80                                      | 783.87  | 792.14                         | 875.65                           | 902.35                                |
| Ecological                    | Min.        | 26.71                                       | 20.56   | 17.68                          | 10.20                            | 6.74                                  |
|                               | MLV         | 39.16                                       | 41.38   | 34.97                          | 26.76                            | 19.91                                 |
| B                             | Max.        | 94.44                                       | 83.40   | 75.77                          | 63.86                            | 49.69                                 |
| Economic                      | Min.        | 245.35                                      | 198.41  | 157.33                         | 119.13                           | 83.62                                 |
| loss due to<br>drilling fluid | MLV         | 338.92                                      | 274.08  | 217.33                         | 164.56                           | 115.51                                |
| discharge                     | Max.        | 460.03                                      | 372.03  | 294.99                         | 223.37                           | 156.78                                |
| Human health                  | damage cost | Negligible<br>cancer risks<br>are less that | values of costs<br>costs are also<br>1 for all case | are estimated<br>neglected bec | for cancer risk<br>ause the comp | The non-<br>osites (HI <sub>c</sub> ) |

Table 7.12. Cost (\$/day) estimates for various discharge scenarios

The treatment cost was estimated from a regression model developed in Chapter 6. The values used for developing the treatment cost regression model were based on a onemonth drilling time. The values estimated from regression models were divided by 12 to normalize over a period of one-year. In this regression model, the treatment cost is a function of percentage of base fluid attached. The uncertainties in the regression model coefficients contribute to the uncertainty in the treatment cost estimates. The minimum, MLV and maximum values are reported in Table 7.12. The minimum and maximum values correspond to  $1^{st}$  and  $99^{th}$  percentile values, respectively. These values were obtained after performing MC simulations on the treatment cost regression model.

The ecological damage cost was estimated based on the composite risk estimates. The ecological risks estimated in the total impact area of the oilfield were used as the probability of adverse effects on commercial fish. The following formulation was used to estimate the ecological damage due to a single well.

$$Ecological \ damage \ cost (\$/day) = \frac{ER \cdot Fish (kg / yr) \cdot Cost (\$/kg)}{365 (days / yr) \cdot Wells}$$
(7.5)

where

ER = Composite ecological risk or probability of adverse effects

Fish caught = 2,110,000 kg/yr

Cost = 0.83 \$/kg (CEAA, 2001)

Total number of wells on the oilfield under study = 32

Conversion unit = 365 days/yr

The values estimated using the formulation above are summarized in Table 7.12. The other important component of cost estimation is the drilling fluid cost. The quantity of drilling fluid lost in each discharge scenario was converted into an economic loss by the following relationship.

Drilling fluid loss 
$$(S/day) = \frac{Unit cost(S/bbl) \cdot Drilling fluid volume(bbl)}{365(days)}$$
 (7.6)

where the unit cost of base fluid (ester) is reported in Chapter 6. The minimum, MLV and maximum values of ester unit cost are given in Table 6.5. The unit cost was defined by a triangular distribution T- (251, 347, 471). The volumes of drilling fluid loss have already been estimated for each discharge option in Table 7.3. The unit costs were multiplied by the volume of ester lost in the corresponding discharge option and normalized over oneyear. The economic loss for each discharge option was calculated and summarized in Table 7.12.

The last component of cost estimation is the human health damage cost. The human health cancer and non-cancer risks are used for estimating the cost of providing treatment facilities. The *HIc* values are below 1, therefore non-cancer risks can not be related to any treatment or hospitalization costs. Similarly the cancer risks values were too low to cause any significant expenditures for human health treatment. Therefore the values are not reported in Table 7.12.

The summary of total cost estimates in terms of minimum, MLV and maximum values are summarized in Table 7.13. The total cost values are also plotted in Figure 7.3 for the

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various discharge options. The total cost estimates vary over a wide range for the 10% discharge scenario. The daily cost estimated varies from 575 to 1230 \$/day for a 10% discharge option over a year. The major contributor to the cost is the cost of drilling fluid lost. Contrary to that, for the 4% discharge scenario the major contributor to the total cost is the treatment cost. The total cost estimates show that for the 7.0%, 5.5% and 4.0% discharge options the range of total cost is approximately the same but an MLV for the 7.0% discharge scenario is less than the other scenarios (Figure 7.3).

| Parameter | 10.0%  | 8.5%   | 7.0%   | 5.5%   | 4.0%   |
|-----------|--------|--------|--------|--------|--------|
| Min.      | 573.8  | 561.5  | 560.8  | 546.0  | 537.0  |
| MLV       | 828.6  | 740.2  | 668.1  | 802.9  | 732.2  |
| Max.      | 1230.6 | 1131.4 | 1075.5 | 1065.3 | 1079.0 |

Table 7.13. Total cost (\$/day) estimates for various discharge scenarios

#### 7.7. TECHNICAL FEASIBILITY

The third attribute for the risk management study under consideration is the technical feasibility of the treatment options or management alternatives. The technical feasibility of various alternatives was defined based on three basic indicators: ease of operation (EO), status of the technology (ST), and control measures required (CM). These are not necessarily the only indicators to compare various treatment alternatives, rather they cover the basic information about each treatment option. For example, the ease of operation may include the total number of hours of operation, capacity of the treatment unit, and exposure of workers to drilling fluid. Similarly, the status of technology can encompass the efficiency of the treatment device, availability of skilled labor, and space requirements. The control measures include maintainability of the treatment units, and safety of workers. All of these sub-indicators need a detailed study to incorporate information from vendors, regulators and literature.



Figure 7.3. Total cost estimates for various discharge scenarios.

The technical feasibility basic indicators were defined qualitatively. Miller's scale (1965), in which five comparative scales are defined from 1 to 5 was used for this purpose. The highest number refers to the best level and the lowest one refers to the poorest. The levels 2, 3 and 4 refer to satisfactory, good, and very good, respectively. The numerical scores are assigned from 0 to 1. The scales are assumed as fuzzy numbers to incorporate vagueness and fuzziness in human judgement. The MLV of each scale represents the membership function of 1. The base of the fuzzy number represents uncertainty in human judgement. Figure 7.4 illustrates these five qualitative levels to compare technical feasibility basic indicators.

Based on Figure 7.4, the ST, EO and CM were rated for various discharge scenarios. As described before, this qualitative scaling needs significant information before assigning any scale. The present study does not focus extensively on this aspect of risk management. The scales were assigned based on a simple comparative evaluation. The scales 1 and 5 were not assigned for any indicator because the scale 1 represents a poor level, which is not the case for any alternative. Even in the case of ST, the scale 1 could not be assigned to the 10% discharge option, because it represents a current discharge practice in the U.S. offshore. Similarly, the scale 5 represents an excellent level, which could be assigned to EO in the case of the 10% discharge option or for the 4% discharge option for ST. It is avoided because there are some other technologies like thermal treatment, which were not evaluated in this study but might have a better scale rating. Based on these considerations, the scales were assigned to technical feasibility attributes for various discharge options as given below (Table 7.14).



Figure 7.4. The qualitative scale used to rank technical feasibility indicators.

Table 7.14. The qualitative scales assigned to rank technical feasibility indicators.

| Items                              | 10.0% | 8.5% | 7.0% | 5.5% | 4.0% |
|------------------------------------|-------|------|------|------|------|
| Status of Technology (ST)          | 3     | 3    | 4    | 4    | 4    |
| Ease of Operation (EO)             | 4     | 4    | 4    | 3    | 3    |
| Control Measures Requirements (CM) | 4     | 4    | 4    | 3    | 2    |

# 7.8. RISK MANAGEMENT - FUZZY COMPOSITE PROGRAMMING

# 7.8.1 NORMALIZATION OF BASIC INDICATORS

Fuzzy composite programming is used for a tradeoff analysis among conflicting objectives for different alternatives. Chapter 6 has provided a detailed step-by-step methodology for conducting fuzzy composite programming. Figure 7.5 provides a framework for determining a system improvement index to compare different discharge scenarios.



#### Figure 7.5. Framework for fuzzy composite programming for conducting tradeoff analysis among various discharge scenarios/management alternatives

The above figure shows that at the first level, the human health cancer and non-cancer risks were grouped together to make a human health risk index. Similarly the three basic indicators - ease of operation, status of technology, and control measures - were grouped to get the technical feasibility index. At the second level, the human health risk and ecological risk were grouped together to make a general group of the environmental risk reduction index. At the final stage, environmental risk reduction, technical feasibility and total cost saving indices were grouped to obtain a system improvement index. The values of the basic indicators used in fuzzy composite programming are given in Table 7.15.

The higher the risk value the less desirable that alternative will be. Similarly if the total cost is more, that option will not be desirable. But contrary to that if some basic indicator has a higher value (e.g., ease of operation), it may be considered a better option. To avoid this confusion. all basic indicators were normalized based on two criteria, either WORST > BEST (e.g., risk, cost etc.) or BEST > WORST (e.g., technical feasibility). These two criteria were used to normalize the basic indicators into unitless numbers scaling from 0 to 1. The method is discussed in detail in Chapter 6. The BEST and WORST values are selected among the estimated values for different discharge options. Table 7.16 lists the BEST and WORST values for all basic indicators.

After defining the BEST and WORST criteria values for each basic indicator, the values reported in Table 7.15 were normalized. All normalized basic indicator values are unitless and are scaled from 0 to 1, in which a higher value means a better option. Now the risk estimates are risk reduction index and total cost is converted into a cost saving index. Table 7.17 summarizes the normalized values of risk reduction and cost saving indices. The technical feasibility indicators have already been scaled from 0 to 1 so they will remain the same. The estimated normalized values of all basic indicators are reported in unitless terms so that they can be grouped according to composite programming methodology.

| Indicators        | Values | 10.0%                    | 8.5%                     | 7.0%                      | 5.5%                     | 4.0%                     |
|-------------------|--------|--------------------------|--------------------------|---------------------------|--------------------------|--------------------------|
| HHR,              | Min.   | 1.24 × 10 <sup>-16</sup> | 9.15 × 10 <sup>-17</sup> | 8.26E × 10 <sup>-17</sup> | 6.39 × 10 <sup>-17</sup> | $4.53 \times 10^{-17}$   |
|                   | MLV    | 5.54 × 10 <sup>-16</sup> | 4.31 × 10 <sup>-16</sup> | 5.18 × 10 <sup>-16</sup>  | 1.73 × 10 <sup>-10</sup> | 1.87 × 10 <sup>-16</sup> |
|                   | Max.   | 3.49 × 10 <sup>-15</sup> | $2.95 \times 10^{-15}$   | $2.35 \times 10^{-15}$    | 1.64 × 10 <sup>-15</sup> | 1.16 × 10 <sup>-15</sup> |
|                   | Min.   | 0.0065                   | 0.0053                   | 0.0043                    | 0.0033                   | 0.0024                   |
| HHR <sub>sc</sub> | MLV    | 0.0163                   | 0.0144                   | 0.0135                    | 0.0090                   | 0.0048                   |
|                   | Max.   | 0.0856                   | 0.0730                   | 0.0624                    | 0.0435                   | 0.0326                   |
|                   | Min.   | 0.1775                   | 0.1366                   | 0.1175                    | 0.0678                   | 0.0448                   |
| ER                | MLV    | 0.2602                   | 0.275                    | 0.2324                    | 0.1778                   | 0.1323                   |
|                   | Max.   | 0.6276                   | 0.5542                   | 0.5035                    | 0.4244                   | 0.3302                   |
|                   | Min.   | 573.8                    | 561.5                    | 560.8                     | 546.0                    | 537.0                    |
| Cost              | MLV    | 828.6                    | 740.2                    | 668.1                     | 802.9                    | 732.2                    |
|                   | Max.   | 1230.6                   | 1131.4                   | 1075.5                    | 1065.3                   | 1079.0                   |
|                   | Min.   | 0.60                     | 0.60                     | 0.60                      | 0.30                     | 0.30                     |
| EO                | MLV    | 0.75                     | 0.75                     | 0.75                      | 0.50                     | 0.50                     |
|                   | Max.   | 0.90                     | 0.90                     | 0.90                      | 0.70                     | 0.70                     |
| ST                | Min.   | 0.30                     | 0.30                     | 0.60                      | 0.60                     | 0.60                     |
|                   | MLV    | 0.50                     | 0.50                     | 0.75                      | 0.75                     | 0.75                     |
|                   | Max.   | 0.70                     | 0.70                     | 0.90                      | 0.90                     | 0.90                     |
|                   | Min.   | 0.60                     | 0.60                     | 0.60                      | 0.30                     | 0.10                     |
| СМ                | MLV    | 0.75                     | 0.75                     | 0.75                      | 0.50                     | 0.25                     |
|                   | Max.   | 0.90                     | 0.90                     | 0.90                      | 0.70                     | 0.40                     |

Table 7.15. Values of the basic indicators used in fuzzy composite programming.

| Parameter                     | Worst (W)                | Best (B)                 | Criteria   |  |
|-------------------------------|--------------------------|--------------------------|--|--|
| Human Cancer Risk             | 3.49 × 10 <sup>-15</sup> | 4.53 × 10 <sup>-17</sup> | W>B  |  |
| Human Non-cancer Risk (HIc)   | $2.40 \times 10^{-3}$    | 8.56 × 10 <sup>-2</sup>  | W>B  |  |
| Ecological Risk (ER)          | 6.28 × 10 <sup>-1</sup>  | $4.48 \times 10^{-2}$    | W>B  |  |
| Status of Technology          | 0.00                     | 1.00                     | B > W  |  |
| Ease of Operation             | 0.00                     | 1.00                     | qualitative terms from                               |  |
| Control Measures Requirements | 0.00                     | 1.00                     | scales 1 to 5 and converted<br>into numerical scores |  |
| Total Cost (\$/day)           | $1.231 \times 10^{3}$    | $5.37 \times 10^{2}$     | W>B  |  |

Table 7.16. The criteria values used for normalizing basic indicators.

#### 7.8.2. WEIGHTING SCHEMES OF BASIC INDICATORS

After normalizing the basic indicators, the next step in fuzzy composite programming is to define the weights for different attributes. Double weighting schemes as described in Chapter 6 are used here. The first type of weighting (w) is based on one-to-one comparison of attributes. The AHP process is used to define the relative importance of each attribute and to group them into a generalized group. For example, human cancer risk and non-cancer risks were proportioned as 2:1 by importance, at the first level. Similarly, human health risk was given more priority than ecological risk by giving double weight to human health risk. At the highest level, the environmental risk reduction and cost saving were given three times more weight in comparison to the technical feasibility. The corresponding weights were calculated as

$$E C T$$

$$J = \begin{bmatrix} 1 & 1 & 3 \\ 1 & 1 & 3 \\ 1/3 & 1/3 & 1 \end{bmatrix} C$$

$$T$$

| Indicators                      | Values | 10.0%  | 8.5%   | 7.0%   | 5.5%   | 4.0%   |
|---------------------------------|--------|--------|--------|--------|--------|--------|
| HHR <sub>c</sub><br>(Reduction) | Min.   | 0.0000 | 0.1568 | 0.3309 | 0.5371 | 0.6764 |
|                                 | MLV    | 0.8523 | 0.8880 | 0.8628 | 0.9629 | 0.9589 |
|                                 | Max.   | 0.9772 | 0.9866 | 0.9892 | 0.9946 | 1.0000 |
|                                 | Min.   | 0.0000 | 0.1514 | 0.2788 | 0.5060 | 0.6370 |
| HHR <sub>ac</sub>               | MLV    | 0.8329 | 0.8558 | 0.8666 | 0.9207 | 0.9712 |
|                                 | Max.   | 0.9507 | 0.9651 | 0.9772 | 0.9892 | 1.0000 |
|                                 | Min.   | 0.0000 | 0.1259 | 0.2129 | 0.3487 | 0.5103 |
| ER<br>(Reduction)               | MLV    | 0.6304 | 0.6050 | 0.6781 | 0.7718 | 0.8499 |
|                                 | Max.   | 0.7723 | 0.8425 | 0.8753 | 0.9605 | 1.0000 |
|                                 | Min.   | 0.0000 | 0.1430 | 0.2236 | 0.2383 | 0.2186 |
| Cost<br>(Saving)                | MLV    | 0.5796 | 0.7070 | 0.8110 | 0.6166 | 0.7186 |
| (                               | Max.   | 0.9469 | 0.9647 | 0.9657 | 0.9870 | 1.0000 |
|                                 | Min.   | 0.6000 | 0.6000 | 0.6000 | 0.3000 | 0.3000 |
| EO                              | MLV    | 0.7500 | 0.7500 | 0.7500 | 0.5000 | 0.5000 |
|                                 | Max.   | 0.9000 | 0.9000 | 0.9000 | 0.7000 | 0.7000 |
| ST                              | Min.   | 0.3000 | 0.3000 | 0.6000 | 0.6000 | 0.6000 |
|                                 | MLV    | 0.5000 | 0.5000 | 0.7500 | 0.7500 | 0.7500 |
|                                 | Max.   | 0.7000 | 0.7000 | 0.9000 | 0.9000 | 0.9000 |
|                                 | Min.   | 0.6000 | 0.6000 | 0.6000 | 0.3000 | 0.1000 |
| СМ                              | MLV    | 0.7500 | 0.7500 | 0.7500 | 0.5000 | 0.2500 |
|                                 | Max.   | 0.9000 | 0.9000 | 0.9000 | 0.7000 | 0.4000 |

Table 7.17. Normalized values of the basic indicators

A normalised matrix I was determined by normalising the matrix J column wise. The final weighted vector can be derived from summing the elements of each row of matrix I and normalising again for this vector.

$$I = \begin{bmatrix} 0.429 & 0.429 & 0.429 \\ 0.429 & 0.429 & 0.429 \\ 0.142 & 0.142 & 0.142 \end{bmatrix} \Rightarrow \begin{bmatrix} 1.288 \\ 1.288 \\ 0.429 \end{bmatrix}$$
(7.8)

After normalising to 1, the above equation becomes

$$W^T = [E \quad C \quad T] = [0.43 \quad 0.43 \quad 0.14]$$
 (7.9)

where

E = Environmental risk (reduction):

C = Cost (saving); and

T = Technical feasibility

Table 7.18 summarises the calculated weights for all attributes at various levels. The second type of weight is the balancing factor (p). The balancing factor (p) was assigned to groups of indicators to show the importance of the maximal deviations. It represents the maximum difference of an indicator value from the best value for that indicator. The larger the value, the greater the concern with respect to the maximal deviation. When p = 1, all deviations are equally weighted, for p = 2, each deviation receives its importance in proportion to its magnitude (Torno et al., 1988).

#### 7.8.3. TRADEOFF ANALYSIS

After assigning weights to basic indicators and more generalized groups at higher levels, a tradeoff analysis is performed. The analysis is performed for each management alternative or discharge option. The advantage of using fuzzy composite programming is that any two attributes can be compared while grouping, e.g., the environmental risk reduction index can be compared to the cost saving index or technical feasibility index.

| Indicators                    | Importance value | w    | p |
|-------------------------------|------------------|------|---|
| Human Cancer Risk             | 2                | 0.67 |   |
| Human Non-cancer Risk         | 1                | 0.33 | L |
| Human Health Risk             | 2                | 0.67 |   |
| Ecological Risk               | 1                | 0.33 | 2 |
| Status of Technology          | 1                | 0.33 |   |
| Ease of Operation             | 1                | 0.33 | 2 |
| Control Measures Requirements | 1                | 0.33 |   |
| Technical Feasibility         | I                | 0.14 |   |
| Environmental Risk Reduction  | 3                | 0.43 | 2 |
| Cost Saving                   | 3                | 0.43 |   |

Table 7.18. Weights estimated for basic indicators and more generalized groups

Figure 7.6 shows a comparison of the environmental risk reduction and the cost saving indices for different discharge options. This plot shows the largest likely interval or range of the fuzzy number (base of the triangle, when membership function,  $\mu(h) = 0$ ) in two dimensions. The x-axis shows the largest likely interval or range of the environmental risk reduction index and the y-axis shows the largest likely interval/range of the cost saving index. For the 10% discharge option, the largest likely intervals are joined with the MLV. This can form a pyramid if it is shown in 3-D. The MLVs of two indices are also provided for each option given by the x and y components. The larger base of the pyramid shows higher uncertainty in the indices. The 10% discharge option has the largest base for the cost saving index. The 4% discharge option shows more uncertainty in cost saving index than the environmental risk reduction index. The MLV of cost saving and environmental risk reduction indices are at (0.93, 0.72). The ideal point in this type of comparison would be (1.0, 1.0) which is practically very difficult to achieve.



Figure 7.6. Comparison of environmental risk reduction and cost saving indices

The environmental risk reduction index can also be compared with the technical feasibility index. Figure 7.7 shows that the base (or largest likely interval) of the 5.5%

discharge option is larger than the other options, but the environmental risk reduction index uncertainty is maximum for the 10% discharge option. The 7.0% discharge option shows the best results from a technical feasibility viewpoint. If the system improvement index is based on only technical feasibility and environmental risk reduction viewpoints, then the 7.0% discharge option may be the best option. This can be observed, as its MLV is closest to the ideal point (1.0, 1.0). But this option shows higher uncertainty in the environmental risk reduction index than the 4.0% and 5.5% discharge options. The environmental risk reduction index was assigned three times more weight than the technical feasibility index, which reduces the chance of the 7.0% option to be selected as the best discharge option.





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Similarly, the cost saving index is also compared with the technical feasibility index. Figure 7.8 shows that the 7.0% option is the best option if the cost saving and the technical feasibility indices are the main criteria. The MLV of the 7.0% discharge option is closest to the ideal point (1.0, 1.0).



Figure 7.8. Comparison of cost saving and technical feasibility indices

After estimating the technical feasibility, cost saving and environmental risk reduction indices, they are grouped as a system improvement index. This final index is in the form of a fuzzy number. For all discharge options, the system improvement indices are compared in Figure 7.9. The ideal point of (1.0, 1.0) is also shown for comparison. The 7.0%, 5.5% and 4.0% discharge options look very close to each other. The MLV of the 7.0% and 4.0% discharge options are approximately the same, but the largest likely interval of the 7.0% discharge option is bigger than the 4.0% discharge option. The MLVs of the 7.0% and 4.0% discharge options are approximately 0.80, but the largest likely intervals of the 7.0% and 4.0% discharge options are in the range of 0.62 and 0.51, respectively. The larger base represents higher uncertainty in the estimates.



Figure 7.9. The system improvement index for comparison of various discharge options

### 7.8.4. RANKING ALTERNATIVES

Figure 7.9 shows that selection of the best alternative is not an easy task, because some alternatives are very close to each other. The Chen (1985) and Yager (1980b) centroidal index ranking methods were used to rank these discharge scenarios. In Chen's ranking
method (1985) the utility value  $U_d(x)$  was determined for each fuzzy number as described in Chapter 6. The highest utility value represents the best management alternative. Table 7.19 summarizes the system improvement index values for these discharge scenarios. The utility values and centroid were calculated for the system improvement index for each scenario. Similar conclusions can be drawn from both ranking methods that the 4.0% is the best management alternative and 7.0% is the next one and then followed by 5.5% discharge option.

| Alternatives | Min.   | MLV    | Max.   | Yager (1980b)<br>Centroid | Chen (1985)<br>U <sub>i</sub> (x) | Rank |
|--------------|--------|--------|--------|---------------------------|-----------------------------------|------|
| 10.0%        | 0.1935 | 0.6859 | 0.9154 | 0.6025                    | 0.3634                            | 5    |
| 8.5%         | 0.2354 | 0.7431 | 0.9353 | 0.6583                    | 0.4698                            | 4    |
| 7.0%         | 0.3259 | 0.8010 | 0.9496 | 0.7160                    | 0.6437                            | 2    |
| 5.5%         | 0.3829 | 0.7460 | 0.9573 | 0.7062                    | 0.6280                            | 3    |
| 4.0%         | 0.4534 | 0.7951 | 0.9630 | 0.7487                    | 0.7208                            | 1    |

Table 7.19. Ranking of various discharge scenarios

#### 7.8.5. SENSITIVITY ANALYSIS

The ranking of various discharge options was the last step in deciding which option is the best management alternative. The process of ranking alternatives involves assumptions and human judgments for assigning weights to various attributes. To confirm the ranking order achieved in the previous section, sensitivity analysis was performed in which various weighting schemes were employed and the entire fuzzy composite programming procedure was repeated. Table 7.20 summarizes the trials in which new weights and importance values were assigned to the last three groups. The first trial results have already been discussed in which risk reduction and cost saving indices were given three times more weight than the technical feasibility index. The second trial represents the case, in which technical feasibility is not considered, rather risk reduction and cost saving indices were assumed to be the selection criteria for the best management alternative. Similarly in the third trial, the environmental risk reduction index was given 1.5 times more weight than the cost saving index. This trial represents the pro-environment scenario. In the last trial, the weights of environmental risk reduction and cost saving indices are reversed, which is a pro-cost saving scenario.

| Trials                        | Weighting<br>schemes  | Environmental risk reduction index | Cost saving index  | Technical feasibility index |
|-------------------------------|---|------------------------------------|--|-----------------------------|
| Trial 1                       | Importance value  | 3                                  | 3  | 1                           |
| Risk and cost<br>having same  | w   | 0.43                               | 0.43   | 0.14                        |
| weights                       | р   |                                    | Cost saving<br>index<br>3<br>0.43<br>2<br>1<br>0.50<br>2<br>0.33<br>2<br>0.33<br>2<br>3<br>0.55<br>2 |                             |
| Trial 2                       | Importance value  | 1                                  | 1  | 0                           |
| Risk and cost                 | w   | 0.50                               | 0.50   | 0.00                        |
| weights                       | р   |                                    | index index feasib<br>3 0.43 0<br>2<br>1 0.50 0<br>2<br>2<br>2 0.33 0<br>2<br>3 0.55 0<br>2          |                             |
| Trial 3                       | Importance value  | 3                                  | 2  | 1                           |
| Risk is having<br>more weight | w   | 0.50                               | 0.33   | 0.17                        |
| than cost                     | 13 Importance value 3 2   having<br>veright<br>cost w 0.50 0.33 | 2                                  | LIGHT NO.  |                             |
| Trial 4                       | Importance value  | 2                                  | 3  | 1                           |
| Cost is having                | w   | 0.33                               | 0.55   | 0.17                        |
| than risk                     | р   |                                    | 2  |                             |

| Table 7.20 | . Different | weighting | schemes | for | sensitivity | anal | ysis. |
|------------|-------------|-----------|---------|-----|-------------|------|-------|
|------------|-------------|-----------|---------|-----|-------------|------|-------|

The second trial results are plotted in Figure 7.10. The 5.5% option improves its ranking from third to second position. The 7.0% option was at second position in the trial due to its better technical feasibility index value. The 4.0% discharge option is again shown to be the best discharge option among all alternatives. Both ranking methods have provided the robust ranking rating order.



Figure 7.10. The system improvement index for comparison of various discharge options for trial 2

The third trial represents the situation in which environmental reduction index is given more weight than cost saving index and represents a pro-environment scenario. The environmental risk reduction index is maximum for the 4.0% discharge option. Therefore a utility index of the 4.0% discharge option is improved from 0.72 to 0.74, approximately. The ranking orders show that the 4.0% discharge option is the best option and 5.5% is second, followed by the 7.0% discharge option, although the second and third ranks have no appreciable differences in their utility and centroidal values. The 10.0% discharge option is the least desirable option among all management alternatives. Figure 7.11 shows the system improvement indices for all management alternatives.



Figure 7.11. The system improvement index for comparison of various discharge options for trial 3

The final trial is a pro-cost saving scenario in which the cost saving index is given 1.5 times more weight than the risk reduction scenario. The results show that the 4.0% discharge option is still the best management alternative but the 7.0% has improved its ranking again from 3 to 2 in comparison to the 5.5% discharge scenario. The utility value of 7.0% discharge scenario is appreciably higher than the 5.5% discharge alternative. The 10.0% discharge option is still the least desirable option, then followed by the 8.5% discharge option. Figure 7.12 compares the system improvement indices for all management alternatives.



Figure 7.12. The system improvement index for comparison of various discharge options for trial 4

A summary of the ranking for various weighting schemes is provided in Table 7.21. Overall, the 4.0% discharge option is the best management alternative. The second best option lies somewhere between the 5.5% and 7.0% discharge options. The least desirable option is the current practice followed in the U.S. jurisdictions (Gulf of Mexico, California, and Alaska). The better management alternatives are those options in which treatment technology is sophisticated enough to substantially reduce the ecological damages without compromising the cost of treatment.

| Trials  | Method         | 10.0%  | 8.5%   | 7.0%   | 5.5%   | 4.0%   |
|---|----------------|--------|--------|--------|--------|--------|
| Trial 1<br>Risk and cost<br>having same<br>weights    | $U_t(x)$       | 0.3634 | 0.4698 | 0.6437 | 0.6280 | 0.7208 |
|   | Centroid       | 0.6025 | 0.6583 | 0.7160 | 0.7062 | 0.7487 |
|   | Rank           | 5      | 4      | 2      | 3      | 1      |
| Trial 2<br>Risk and cost<br>having same<br>weights    | $U_t(x)$       | 0.3789 | 0.5948 | 0.7192 | 0.7273 | 0.8074 |
|   | Centroid       | 0.5751 | 0.6503 | 0.7078 | 0.7225 | 0.7774 |
|   | Rank           | 5      | 4      | 3      | 2      | 1      |
| Trial 3<br>Risk is having<br>more weight<br>than cost | $U_t(x)$       | 0.3684 | 0.4680 | 0.6437 | 0.6515 | 0.7426 |
|   | Centroid       | 0.6137 | 0.6628 | 0.7183 | 0.7190 | 0.7596 |
|   | Rank           | 5      | 4      | 3      | 2      | 1      |
| Trial 4<br>Cost is having<br>more weight<br>than risk | $U_{\rm f}(x)$ | 0.3563 | 0.4562 | 0.6362 | 0.5755 | 0.6655 |
|   | Centroid       | 0.5992 | 0.6564 | 0.7169 | 0.6856 | 0.7246 |
|   | Rank           | 5      | 4      | 2      | 3      | 1      |

Table 7.21. Summary of sensitivity analysis results for ranking management alternatives.

On the East Coast of Canada, the regulations concerning drilling cutting discharges are currently being reviewed. A reduction from 15% attached base fluid to the neighborhood of say 7.0 to 8.0% of dry weight of cuttings corresponds to approximately 12.0% to 5 - 6% of wet cuttings. The results of this study shows that a discharge scenario of 5 - 6 % (wet cuttings) is one of the better management alternative, if not the best.

### Chapter 8

# CONCLUSIONS AND RECOMMENDATIONS

This chapter first presents some conclusions based on methodologies developed for contaminant fate modeling, ecological and human health risk assessments and risk management. It also summarises some specific conclusions based on a hypothetical case study of an oilfield on the East Coast of Canada. This chapter also provides recommendations and establishes the directions for future research. At the end, a statement of originality of this thesis is presented.

#### 8.1. SUMMARY AND CONCLUSIONS

The study was carried out by adopting a holistic approach for risk management of offshore petroleum drilling waste discharges in the marine environment. The research focused on the following objectives: (1) development of probabilistic contaminant fate models using fugacity and aquivalence based approaches for organic compounds and heavy metals, respectively; (2) development of an ecological risk assessment methodology using probabilistic concepts; (3) development of human health cancer and non-cancer risk assessment methodologies using probabilistic concepts; (4) development of a fuzzy composite framework for risk management by integrating environmental risk assessment with cost estimation and technical feasibility of various treatment options: and (5) application of a risk management framework for a hypothetical case study of drilling waste discharges on the East Coast of Canada.

Keeping these objectives in perspective, the following are the conclusions of this research:

1. A new methodology for contaminant fate modeling using a chemical specific approach was proposed in Chapter 3. Steady state non-equilibrium fugacity/aquivalence based equations were used to estimate the distributions of concentration in the water column ( $C_{\infty}$ ) and sediment pore water ( $C_{\infty}$ ). The uncertainty and variability in the model inputs were expressed by their statistical distributions. The Cw and Crw were estimated using Latin Hypercube Sampling (LHS) based Monte Carlo (MC) simulations. The fate model outputs, Cw and Crw followed the lognormal distribution due to the multiplicative nature of the model. The parameters of the lognormal distributions LN ~ ( $\mu$ ,  $\sigma$ ) were estimated for different discharge conditions, based on pollutant loading rates (E) and impact radii (R). The estimated parameters of the lognormal distribution ( $\mu$ ,  $\sigma$ ) for known discharge conditions were used for performing multiple regression analyses. The highest 95th percentile was used as the predicted environmental concentration (PEC). The uncertainties in the 95th percentile values (PEC) were estimated by the coefficients of mean ( $\mu$ ) and standard deviation ( $\sigma$ ) regression models. The methodology developed for estimating PEC through regression models is flexible and can be easily modified to any other desired safety level depending on the management goals and objectives. The strength of this methodology is that it incorporates natural variabilities and/or uncertainties in the model input parameters, which help in predicting the output concentrations and their associated uncertainties. This approach did not consider the model uncertainty, which is generally not high in well-tested models (CCME, 1997). The developed methodology is not only applicable to drilling waste discharges, rather it can be used for other industrial wastes having known physico-chemical characteristics.

2. A comprehensive methodology for probabilistic ecological risk assessment was developed in Chapter 4. The whole ecological community was defined as assessment endpoints. The toxicity data (LCw, ECw and NOEC) were converted into predicted no effect concentrations (PNEC). The lognormal was found to be the representative distribution for PNEC data. The lowest 10<sup>th</sup> percentile of the PNEC distribution was defined as the safety level or PNEC criteria value. Bootstrapping was performed on original PNEC data to determine uncertainty in the PNEC criteria values. The exposure assessment was based on contaminant fate modeling results. The estimated PEC values were converted into exposure concentrations (EC) by adjusting for bioavailability (BF) and probability of exposure (p). The hazard or risk quotients (HQ/RQ) were calculated by taking the ratio of EC and PNEC criteria values. The CHARM model's approach (Thatcher et al., 1999) was used to convert HQ/RQ into risk estimates for each contaminant. The composite ecological risk for drilling waste was determined by integrating the individual risk estimates assuming statistically

independent events. The ecological risk assessment methodology developed is unique and allows for the incorporation of more toxicity data (as it becomes available) to improve *PNEC* criteria values. This technique can be very useful in developing risk based water and/or sediment quality criteria. By fixing an acceptable risk posed by a particular pollutant in water and/or sediments, the pollutant load can be estimated to determine its allowable level in the industrial discharges. This risk based approach can help in developing pollutant effluent and emission standards.

3. A risk assessment methodology for human health was developed in Chapter 5. The human health risk methodology was based on the consumption of contaminated seafood. The chronic daily intake rate (CDI) was established based on fish ingestion rates, lipid content of contaminated fish, bioconcentration factors, exposure duration, exposure frequency, and averaging time. The input parameters were defined by triangular distributions based on the ranges and maximum likely values (MLV) reported in the literature. The human health risk assessment methodology for drilling waste discharges adopts a probabilistic approach based on an extensive literature search for input parameters. A comparison of the U.S. EPA (2000a) and the present study methodologies for human health risk assessment of drilling waste discharges was also presented in this chapter. The developed methodology is more comprehensive and extensive than the traditional U.S. EPA's (2000a) deterministic approach for drilling waste discharges.

- 4. A risk management methodology using fuzzy composite programming (CP) was developed in Chapter 6. The fuzzy composite programming involved identification of basic indicators, a methodology for grouping basic indicators, weighting schemes, converting linguistic terms and statistical data into fuzzy numbers, and ranking methodologies for management alternatives. To perform tradeoff analysis for the selection of the best management alternative or discharge scenario, cost estimates and technical feasibility parameters were studied in this chapter. A linear regression model was developed for the estimation of treatment cost as a function of percent base fluid attached to the wet drill cuttings. The basic indicators - risk estimates. technical feasibility, and cost estimates - were defined by triangular fuzzy numbers to incorporate their uncertainties. The final utility and centroidal values of the system improvement indices were calculated through Chen (1985) and Yager (1980b) ranking methods, respectively to determine the ranking order of the management alternatives. The CP methodology has successfully been used in other environmental engineering problems (e.g., Stansbury et al., 1989). In the present research, CP methodology is modified for probabilistic quantitative and fuzzy qualitative data. This approach has never been applied for management of drilling waste discharges. This modified CP methodology is also applicable to other environmental decisionmaking problems.
- 5. The framework of risk management developed in the first six chapters was applied to a hypothetical case study on the East Coast of Canada in Chapter 7. Five discharge scenarios were defined based on the percent base fluid attached to the wet drill

cuttings. These discharge scenarios or management alternatives were 10.0%, 8.5%, 7.0%, 5.5% and 4.0% attached base fluids. The regression models developed in Chapter 3 were used to estimate the PEC, which were converted into exposure concentrations using BF and p. The exposure concentrations were used in the human and ecological probabilistic risk assessment models to quantify the risk values. Similarly, cost estimates for treatment, drilling fluid lost, and ecological and human health damages for different discharge scenarios were made. The technical feasibility parameters - status of the technology, ease of operation and control measures required - were defined in qualitative terms. The qualitative terms were converted into numerical scores using Miller's scale (1965). The importance matrix of basic indicators was developed using an analytical hierarchy process (AHP). A double weighting scheme was employed in CP. All basic indicators were grouped into environmental risk reduction, cost saving and technical feasibility indices to perform a tradeoff analysis. The 4% attached base fluid option was the best management option for the first trial when risk and cost were given equal weight and technical feasibility was allotted one-third of the weight. The 7.0% discharge option was the second best management alternative and that was followed by 5.5%, 8.5% and 10.0%, respectively. The sensitivity analysis was performed using different weighting schemes to account for human subjectivity. In the second trial, the technical feasibility was not included in determining the system improvement index. The 4.0% discharge option remained the best management alternative, but the 5.5% discharge option improved its ranking from 3 to 2, and 7.0% discharge option moved to 3rd place. The 3rd trial was designed as a pro environment scenario in which risk reduction was given 1.5 times more weight than the cost saving index. The technical feasibility was given one-third importance in comparison to environmental risk. The overall ranking remained the same as that of the second trial. The last trial was a pro cost-saving scenario in which the relative importance of the 3<sup>rd</sup> trial was reversed for the cost and the risk indices. Now again, the 4.0% discharge option remained the best management alternative, but the 7.0% discharge option improved its ranking from 3 to 2, and the 5.5% discharge option ranking dropped from 2<sup>rd</sup> to 3<sup>rd</sup> place. The 10.0% option remained the least desirable discharge option. It was concluded that the 4.0% was the best management alternative for the discharge of drilling waste in the marine environment. The other better management alternatives or discharge options were the 7.0% and/or 5.5% attached base fluid to the wet cuttings. This case study illustrated the utility of the methodologies developed in first six chapters of this thesis. This case study provided a framework for a decision support system for the selection of the best drilling waste marine discharge option under any known regulatory and technical constraints.

#### 8.2. RECOMMENDATIONS

Recommendations are useful for setting the directions of future research. The following recommendations are presented based on the limitations and problems faced during this study:

- 1. The present study used MC simulations for the probabilistic analyses to incorporate uncertainty in the input parameters. To avoid the assumption of independent input variables, interval analysis and probability bound analysis may be used. Risk Calc (1999) at present is only capable of handling simple mathematical formulations. In the present research, effort was made to apply these methods in the beginning, but they did not work properly due to complexities in the contaminant fate modeling and ecological risk assessment models. In the future research, it may be very useful to develop a methodology for contaminant fate modeling and ecological risk assessment using fuzzy numbers to incorporate the uncertainty in the input variables and compare with the results of MC simulations.
- 2. The fugacity/aquivalence based model inputs are defined by statistical distributions. A probabilistic parametric approach was employed to perform the analysis. The inputs were assumed to be independent of each other due to the lack of reliable data to develop a correlation among input variables. For future studies, more data need to be collected to define the relationships among input variables to perform MC simulations.
- 3. The methodology of ecological risk assessment was based on defining the whole ecological community as the assessment endpoints. The criteria *PNEC* values or safety levels were developed based on available data and information from various sources. The criteria *PNEC* values will be changed if more comprehensive toxicity response information becomes available. There is an immediate need for developing a

database for toxicity response of organisms (e.g.,  $LC_{\infty}$ , NOEC) for different contaminants, which should be made available to the public domain.

- 4. The bioavailability of contaminants is a sensitive issue in ecological and human health risk assessment. The bioavailability (*BF*) was defined by leaching or conversion factors in this study (U.S. EPA, 2000a). More research and laboratory experiments are needed in this direction to estimate the actual bioavailable fraction of the contaminants.
- 5. The fugacity/aquivalence models developed in this study were based on an assumption of steady state conditions. It may be useful to modify the fugacity/aquivalence based models by introducing a time factor into the formulation for unsteady state solution.
- 6. A chemical specific approach was used in this study due to the lack of physicochemical and toxicity data of drilling waste as a whole (or a single entity). A whole toxicity approach can be used in future if enough information and data becomes available.
- 7. The exposure concentrations were a function of exposure probability (p), which was calculated by the ratio of impact area (A<sub>\*</sub>) to the total area under study. The migration rates of fish may play a very important role in estimating the actual exposure to the

contaminants. When more information is available in specific studies, it can be employed for more realistic estimates of exposure probability.

- 8. Human health risks were estimated based on the consumption of contaminated fish due to drilling waste discharges into the marine environment. The other source of human health exposure is the drilling platform, where workers are exposed to chemicals through inhalation, ingestion and dermal contact. That aspect of drilling waste was not considered in the present study and can be incorporated in future research for risk management of drilling waste.
- 9. The technical feasibility of various treatment options was defined by three parameters: status of the technology, ease of operation and control measures. These parameters were defined by five qualitative fuzzy scales that ranged from the best to the poorest. A more comprehensive study could be used for this aspect of the research.
- 10. The cost estimates were based on treatment, drilling fluid loss and human and ecological damage costs in the present study. The costs related to non-water quality impacts, including global warming impacts (GWI), ozone depletion potential (ODP) and risks associated with the safety issues at the platform can also be incorporated into the analysis.

11. The environmental impact component of this research was focussed on the risks associated with ecological and human health due to exposure through discharges of drilling waste into the marine environment. This study did not estimate the GWI, ODP and risks related to the health and safety (H&S) of the workers. A life cycle analysis (LCA) type of approach could be an alternative solution in which cradle-to-grave or cradle-to-gate type of methods can be used over the life span of drilling waste. The LCA could start from a factory where drilling fluids are manufactured to their final disposal location or a recycling facility.

#### 8.3. STATEMENT OF ORIGINALITY

Originality in the present research can be viewed from following perspectives:

Fugacity/aquivalence based models are used extensively in lake and river systems in the deterministic mode. In this research, a probabilistic framework for conducting contaminant fate modeling was presented for a marine environment. The output concentrations ( $C_{w}$ ,  $C_{rw}$ ) were lognormally distributed and the parameters of the distributions were converted into empirical regression models. The regression models were grouped together to estimate *PEC*, which was defined by the highest 95<sup>th</sup> percentile value. It can be defined at any level based on the management goals and objectives. These *PEC* equations are flexible and can easily be modified to any desired exposure level.

A new methodology for ecological risk assessment was developed for drilling waste discharges using the U.S. EPA (1998) and CCME (1999) frameworks. A new procedure for streamlining the  $LC_w$  and NOEC data into PNEC values was introduced. The PNEC data for different organisms were found to be lognormally distributed. The selection criteria of endpoints in the CHARM model (Thatcher et al., 1999) are based on only three types of organisms: fish, algae and crustaceans, whereas in the current study the whole ecological community was defined as endpoints. The criteria PNEC values or safety levels were estimated using bootstrapping to incorporate the uncertainties in the estimates. The safety level (HQ/RQ) of 1 or less was designated to correspond to the protection of 90% of the species 95% of the time.

Most of the studies related to drilling waste discharges in the marine environment (e.g., U.S. EPA. 2000a, 1999a; Meinhold, 1998) define the acceptable pollutant levels in a drilling waste stream based on water quality standards and guidelines proposed by regulatory agencies. Recently, the author and coworkers (Sadiq et al., 2001d) have proposed a methodology for evaluating various regulatory drilling waste discharge options based on the toxicity response of the whole ecological community using the approach developed in this thesis.

In this study, results of contaminant fate modeling were integrated with probabilistic human health risk assessment using LHS based MC simulations. The probabilistic human health risk assessment approach is not new but in the present study, an extensive

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literature search was conducted to estimate the parameters of the proposed triangular distributions of human health risk assessment through consumption of contaminated fish.

New simple techniques for estimating the costs of treatment, ecological and human health damages were introduced in this study. Probabilistic data were converted into triangular fuzzy numbers using confidence intervals and modes of the risk and cost distributions. The risk management methodology of fuzzy composite programming has been used previously in environmental engineering related problems, but has never been applied to drilling waste discharges. A risk management study encompassing risk minimization, cost saving and technical feasibility of discharge options, has never been studied for drilling waste management.

This study introduced a new concept of integrating probabilistic fate modeling with ecological and human health risk assessment methodologies within a risk management framework to determine the ranking order of management alternatives under these conflicting objectives. This study has provided a framework for a decision support system for the selection of the best drilling waste marine discharge option under any set of regulatory and technical constraints.

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## APPENDIX

## A1

## **Bootstrapping Macro**

GMACRO rehan.mac do k1=1:1000 sample 32 c1 c2: replace. let c3=log(c2) let c4(k1)=mean(c3) let c5(k1)=setdev(c3) let c5(k1)=setdev(c4) let c5(k1)=setdev(c4) let c5(k1)=setdev(c5) let c5(k1)=c5(k1) enddo





